

**Full Drainage Report
Stormwater Wetland Project
City of Arlington
Snohomish County, Washington**

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Prepared for

**City of Arlington
Arlington, Washington**

11 JAN '10



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EXECUTIVE SUMMARY

The City of Arlington (City), in Washington State, is proposing to construct a water quality treatment wetland (project) to treat City-derived stormwater prior to discharging it into the Stillaguamish River. The project is funded by a grant from the Washington State Department of Ecology's Stormwater Management Implementation Grant program. The City contracted Landau Associates to design the project. This report summarizes the design considerations that form the basis of the design (Table 1), including the methods and results for the stormwater modeling. The design plans are presented as Appendix B. This report also presents the methods and results of investigations related to soils, groundwater and flood hazards, and summarizes results of the critical area study (Landau Associates 2009). Key information and conclusions are listed below:

- The project is located on a portion of a 21.9-acre City-owned property, also known as the Hammer Property (property), at 201 West Cox Street (Parcel 31050200300200) in Arlington, Snohomish County, Washington (Figure 1).
- The design avoids impacts to critical areas, floodway, and historic sites.
- The stormwater wetland will receive year-round stormwater from the Old Town catchment area (171 acres in size), which was delineated and modeled by URS Corporation (URS 2008).
- The flows from the new stormwater wetland will exit the property at the same location as they do currently, via the Butler outfall pipe, which drains directly to the Stillaguamish River.
- The total size of the stormwater wetland project (grading area limits) is approximately 8.7 acres.
- The average retention time ranges from a few hours to several days, depending on storm events and their frequency.
- Between 9 and 99 percent of the stormwater will be infiltrated into soils depending on the size of the rainfall event. It is likely this water will return to the river hyporehiec zone.
- The project is expected to: improve the quality of the stormwater discharged to the Stillaguamish River; help achieve the goals and requirements of the Stillaguamish Total Maximum Daily Load Plan and National Pollutant Discharge Elimination System permit; offer habitat for aquatic-dependant and associated wildlife (fish not included); provide educational opportunities for residents, students, and visitors to learn how urban and natural areas can coexist in a balanced ecosystem; provide wildlife viewing opportunities; and reduce peak stormwater flows.

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LIST OF ABBREVIATIONS AND ACRONYMS

BGS	Below Ground Surface
cfs	Cubic Feet per Second
City	City of Arlington, Washington
DAHP	Washington State Department of Archaeology and Historic Preservation
DNR	Washington State Department of Natural Resources
DO	Dissolved Oxygen
Ecology	Washington State Department of Ecology
FAC	Facultative
FACU	Facultative Upland
FACW	Facultative Wetland
FEMA	Federal Emergency Management Agency
ft	Feet
ft ²	Square Feet
ft ³	Cubic Feet
GIS	Geographic Information System
HGM	Hydrogeomorphic
HMP	Habitat Management Plan
NAVD88	North American Vertical Datum of 1988
NGPA	Native Growth Protection Area
NGVD29	National Geodetic Vertical Datum of 1929
NOAA	National Oceanic and Atmospheric Administration
NPDES	National Pollutant Discharge Elimination System
NRCS	Natural Resources Conservation Service
OBL	Obligate
OHWM	Ordinary High Water Mark
PEM	Palustrine Emergent
PFO	Palustrine Forested
PHS	Priority Habitats and Species
PSS	Palustrine Scrub-Shrub
RMF	Redoxymorphic Feature
SMP	Shoreline Master Program
SR	State Route
TMDL	Total Maximum Daily Load
TP	Test Pit
UDC	Snohomish County Unified Development Code
UPL	Obligate Upland
USACE	U.S. Army Corps of Engineers
USDA	U.S. Department of Agriculture
USFWS	U.S. Fish and Wildlife Service
USGS	U.S. Geological Survey
UTM	Universal Transverse Mercator
WDFW	Washington Department of Fish and Wildlife
WDNR	Washington State Department of Natural Resources
WSDOT	Washington State Department of Transportation
WRF	Wastewater Reclamation Facility
WWTP	Wastewater Treatment Plant
yd ³	Cubic Yards

1.0 INTRODUCTION

In the late 1990s, the City of Arlington (City) became aware that there was a growing problem with water quality in the Stillaguamish River and that the City's stormwater discharge could be a contributing factor. As a result, the City is proposing to construct a stormwater wetland project (project) to treat City stormwater prior to discharging it into the Stillaguamish River. The project is funded by a grant from the Washington State Department of Ecology's (Ecology) Stormwater Management Implementation Grant program.

Landau Associates has been contracted by the City to assist with design and documentation related to the project. This report presents the methods and results of investigations related to soils (Appendix A); discusses groundwater and flood hazards; summarizes the results of the critical areas study (Landau Associates 2009); and summarizes the drainage considerations that form the basis of the design (Table 1; design sheets in Appendix B), including the methods and results for the stormwater modeling (model information is presented in Appendices C, D, and E). Specifically, this report includes information on the following project aspects:

- Design elements, including water input rates
- Design results, including grading plan and conveyance routes
- Results of hydrologic modeling, including weir and pipe sizing, and predicted changes in water retention time under various flow regimes.

Note that the river gages and Federal Emergency Management Agency (FEMA) Flood Insurance Study (FIS) and Flood Insurance Rate Map (FIRM) datum are related to National Geodetic Vertical Datum of 1929 (NGVD29). The site plans and the calculations in this report are based on North American Vertical Datum of 1988 (NAVD88). The conversion for the project location is to add 3.69 feet (ft) in elevation to the NGVD29 elevation to obtain the NAVD88 elevation. All discussions of elevations in this report reference the NAVD88 datum, unless otherwise noted.

1.1 PROJECT DESCRIPTION

The project is located in Water Resource Inventory Area 11, Stillaguamish (Hydrological Unit Code 17110008). The project site is located in the southeastern portion of a 21.9-acre City-owned parcel (Parcel 31050200300200; property), also known as the Hammer Property, at 201 West Cox Street in Section 2, Township 31 North, Range 5 East, Quarter SW, in Snohomish County, Washington (Figure 1). The project area is bordered to the east by State Route (SR) 9, to the south by SR 530, to the west by Dike Road, and to the north by the Stillaguamish River (Figure 2). A portion of the property legal description

is occupied by the Stillaguamish River, a shoreline of the state. New stormwater piping will be located within the Washington State Department of Transportation (WSDOT) right-of-way associated with SR 9 (see Figure 1). The upstream sub-basin, which will contribute stormwater to the wetland, is shown on Figure 3. The Wastewater Treatment Plant (WWTP) currently serving the City is located east of the property, on the east side of SR 9, at 108 West Haller in Arlington (see Figure 3).

Water quality in the Stillaguamish River downstream of the City has been impaired for multiple parameters, resulting in listings under Section 303(d) of the Clean Water Act (CWA) in 1996, 1998, and 2004 for dissolved oxygen (DO), temperature, and fecal coliform (Arlington, City of 2007). Currently, stormwater effluent from the downtown basin of Arlington is discharged to the Stillaguamish River untreated, with no flow control (Arlington, City of 2007). The proposed project is designed to reduce pathogens, to remove nutrients that are understood to result in lower instream DO, and to lower water temperature before reaching the river. The project is designed to reduce peak stormwater flows to the river. The project will also provide a wildlife benefit and opportunity for public education and community outreach.

A water reclamation facility (WRF) is planned with an anticipated start-up date as early as September 2010 (Arlington, City of 2007). The City will upgrade and expand its WWTP to a WRF using a membrane bioreactor facility with aerobic digestion, in order to meet the projected growth within the existing urban growth area and additional annexations, and to meet expected regulatory requirements through 2025 (Kennedy/Jenks 2007).

The stormwater wetland will receive year-round stormwater from the Old Town catchment area [identified as the Downtown Basin in the working draft of the City of Arlington Draft Stormwater Comprehensive Plan (Arlington, City of undated)] via gravity flow. Eventually, during the dry season (predicted to be between June 15 and September 15), the stormwater wetland may receive reclaimed water from the WRF via a controlled pump system, based on the results of real time continual monitoring of temperature and DO levels within the Stillaguamish River. This would be part of a separate project to be permitted in the future.

The total size of the stormwater wetland (area of site disturbance) is approximately 9.8 acres, including approximately 5.5 acres of created wetland area, consisting of four cells, separated by weirs (described in Section 2.3.1). Benefits of the project are expected to include the following:

- Improve the quality of stormwater discharged into the Stillaguamish River
- Meet the goals and requirements of the Stillaguamish Total Maximum Daily Load (TMDL) Plan and National Pollutant Discharge Elimination System (NPDES), Phase 2
- Offer habitat for aquatic dependant and associated wildlife

- Provide educational opportunities for residents in Arlington and Snohomish County, students, and visitors to learn how urban and natural areas can coexist in a balanced ecosystem
- Provide recreational leisure activities and wildlife viewing
- Reduce peak stormwater flows discharged to the Stillaguamish River from the catchment area.

1.2 DRAINAGE SUMMARY FORM

Project Name: **City of Arlington Stormwater Wetland Project**

Project Engineer: **Landau Associates**

Project Applicant: **City of Arlington**

Project Total Area: **21+ Acres**

Project Development Area: **8.7 Acres**

Number of Lots (if applies): **1**

**Table 1
Drainage Summary**

Drainage Basin Information		City of Arlington Downtown Basin
Sub-basin Area (acres)		180.7
Type of Storage Proposed		Wetland
Approximate Storage Volume [cubic ft (ft ³)]		275,000
Soil Type(s)		Puget Silty Clay Loam and Puyallup Fine Sandy Loam
Pre-Developed Runoff Rates		At Butler Outfall
Flow (Q) [ft ³ per second (cfs)]	Weekly	6.35
	6 mo.	33.93
	2 yr.	50.83
	10 yr.	75.06
	100 yr.	86.12
Post-Development Runoff Rates		
Q (cfs)	Weekly	0.12
	6 mo.	14.68
	2 yr.	28.63
	10 yr.	50.41
	100 yr.	68.08
Offsite Upstream Area		
Number of Acres		171
Offsite Downstream Flow		
Q (cfs)	100 yr.	N/A

1.3 SITE LAYOUT AND INFRASTRUCTURE

A site plan showing critical areas and other site features is presented on the drawings in Appendix B. The property is bound to the northwest by the Stillaguamish River; to the northeast by SR 9 (and the adjacent WSDOT bioswale) and smaller parcels that contain agricultural buildings and a residence, the “round house;” to the southeast by WSDOT Wetland 4 (WSDOT wetland) (see Figure 2); and to the southwest by Dike Road and undeveloped land. The property is currently developed as a farm

and is used for agricultural purposes (hay field). The site will continue to provide agricultural uses, including harvesting of cattail from the wetland. Existing structures are located on the parcel, including a home, barn, and sheds. This parcel is served by power (Snohomish County Public Utility District), water (City of Arlington) and an individual septic system.

A groundwater well is located on the property, which is connected to the irrigation system located on adjacent property to the north. A septic system is located east of the house, which requires a 100-foot setback from surface water per WAC 246-272A-0210. One historic site is documented as existing on the property, consisting of an old privy/outhouse. The protection area for this historic site is shown on the drawings in Appendix B. No excavation will occur within the protection area.

1.3.1 STORMWATER SYSTEM AND BASIN

A City-owned stormwater pipe bisects the northeastern corner of the site. Stormwater collected from the Downtown Basin portion of the City's municipal storm sewer system is conveyed to the Stillaguamish River via the Butler outfall, which is located north of the property (see the drawings in Appendix B). The outlet of this 36-inch concrete pipe is collapsing (see photo below) and will likely be repaired in the future as part of a separate project.



Photo 1: Butler outfall; note damage to pipe outlet.

The existing upstream basin and conveyance system is shown on Figure 2 of Appendix C, URS Calculation No. HM-1 (URS 2008). The calculation was a capacity analysis of the upstream basin piping. The upstream basin for the treatment wetland, referred to as the Downtown Basin, is composed of piping and catch basins which collect water from the entire basin and convey it to the project area before discharging to the Stillaguamish River via the Butler outfall. The basin is predominantly composed of a mixture of commercial and residential properties located on a river hillside/terrace.

1.4 SOILS INVESTIGATION

A soils investigation was conducted to help determine soil properties and infiltration rates which are pertinent to the project design and modeling tasks. An investigation was conducted by Landau Associates with assistance by City staff on May 27, 2008. Three backhoe test pits (TPs) were excavated within the proposed wetland area, and a description of the soils encountered is presented in Appendix A. A representative sample was collected from TP 2 at 3.5 ft below ground surface (BGS) and analyzed for grain size distribution and to determine the U.S. Department of Agriculture (USDA) textural classification. Infiltration rates used are based on rates stated in the USDA Natural Resource Conservation Service (NRCS) online database for the soil type present (NRCS website 2008).

Additional information on site soils was collected as part of a cultural resources assessment conducted for the project (Northwest Archaeological Associates, Inc. 2008) and from soil descriptive information acquired from the NRCS *Web Soil Survey* (NRCS website 2008).

The soils on the property are identified by the NRCS database as being either a Puget Silty Clay Loam (eastern one-fifth of the property) or a Puyallup Fine Sandy Loam (western four-fifths of the property). The cultural resources investigation excavated over 80 TPs and 10 investigational trenches on the property. The City of Arlington dug three additional TPs. Taken together, these investigations provide a detailed profile of the subsurface soils at the property. The alluvial soils in the wetland area are intermixed river deposits and vary in relatively short distances between sandy silt and silty sand. These soils will allow for infiltration of water from the saturated areas of the wetland. The infiltration rate for the sandy silt could be as low as 0.2 inches/hour and could range as high as 6 inches/hour for the silty sand. Cells 1 and 2 of the planned wetland are dominated by the sandy silt material (less than 20 percent of that area is silty sand). Cell 3 is also dominated by the sandy silt material but as much as 40 percent of that area is silty sand. For modeling purposes, the infiltration rate of Cells 1 and 2 was set at 0.25 inches/hour and the infiltration rate of Cell 3 was set at 0.5 inches/hour. These rates take into account long-term degradation of the infiltration rate expected due to fines accumulation in the wetland soils over time.

1.5 GROUNDWATER INVESTIGATION

City staff conducted a groundwater investigation in order to determine grading depths for the treatment wetland that would result in water storage and infiltration without interference with groundwater. Water levels were monitored within the City-owned well located on the project site (shown on Figure 2) in spring 2008 (date unavailable). River stage was also recorded by the City at the Haller Park boat ramp, located several hundred feet upstream of the site. Landau Associates compared the two data sets to determine the relationship between river stage and groundwater levels within the project area. In addition, three TPs were excavated by the City and Landau Associates in May 2008. Sacha Maxwell of Landau Associates and Bill Blake conducted a site reconnaissance on September 18, 2009 to confirm the location of an outlet from the offsite WSDOT wetland.

Groundwater was first investigated on the site in early March 2008, when water tables were assumed to be highest. Water levels in four backhoe excavation pits (see Figure 2) were below 6.8 ft BGS across the site. Specifically, groundwater was not encountered in three of the four excavation pits; however, groundwater was encountered at 6.8 ft BGS in TP 1, located in the eastern portion of the project area (the area for proposed treatment Cell 1) (City of Arlington 2008a). In late May 2008, groundwater was not encountered in TPs 1 through 3, which ranged from 7 to 8 ft BGS (see Figure 2). Ten backhoe trenches and 88 shovel TPs were excavated as part of the cultural resources investigation in March 2008 (Northwest Archaeological Associates, Inc. 2008; these are not shown on Figure 2). Six of the trenches were located within proposed wetland areas. No groundwater was encountered to the depth of the trench (approximately 5 ft BGS) and all TPs were dry with the exception of those located near the WSDOT bioswale/location of proposed Cell 1, which contained saturated soil at approximately 3 to 4 ft BGS (Northwest Archaeological Associates, Inc. 2008).

Results of the water level monitoring at the City well located within the project area show that water levels were near 10 to 10.8 ft BGS in June and July 2008, while river stage was near 54.2 ft NAVD88 (this generally represents low flow conditions). Groundwater elevation is understood to fluctuate with river stage; however, a complete evaluation of this relationship is not available.

Analysis of alluvial aquifer groundwater elevations for the areas of exposed river alluvium associated with the Stillaguamish River show the contours decrease from 60 ft elevation (NAVD88) in the western portion of the property in a west/northwest direction toward the river to approximately 55 ft (extrapolated from 10-ft contour intervals as mapped) near the western portion of the property (Pacific Groundwater Group 2007).

Over time, City staff has observed the property during and after flood events. In each case, water drained from the property within 24 hours of the ending of flooding. This draining occurs, even though there are low points of the property that retain 1-2 ft of water. During flooding events, it is likely that the

flood water completely charges the relatively shallow aquifer with river water to the ground surface. The fact that the site soils can drain that water away so rapidly is indicative of the aquifer's ability to discharge groundwater rapidly to the river as the river level drops. This also means that it is likely that, as the river level rises, the water table also rises fairly quickly on the property.

1.6 CULTURAL RESOURCES

The cultural resources assessment report, conducted by Northwest Archaeological Associates, Inc. (Northwest Archaeological Associates, Inc. 2008), was reviewed to help estimate soils data applicable to project design and permitting.

1.7 CRITICAL AREAS INVESTIGATION

A Critical Areas Report (Landau Associates 2009) is being submitted separately to Snohomish County and the City for the project, and that report should be referenced for a summary of the critical areas investigation results.

1.7.1 RIVER, FLOODPLAIN/FLOODWAY, AND SHORELINE

The property is floodplain, with approximately the northwestern third of the property mapped as floodway, and the remaining two-thirds of the property mapped as floodway fringe (FEMA website 2009; Snohomish County 2008a; see Figure 2). The river is diked by a continuous levee (referred to as the Dike Road Dike) on both banks upstream and downstream of the property. The portion of the dike along the northern extent of the property is referred to as the Johnson levee. The Johnson levee dike was breached in 2002 (R2 Resource Consultants 2005) and during winter 2008-2009 flooding events, and has since been repaired.

Adjacent to the property, the river flood stage is 65.4 ft and bankfull stage is 64.2 ft (NAVD88; NOAA website 2009). The top of dike elevation is approximately 68 to 69 ft and the river level during a 100-year event is approximately 71 ft (NAVD88; NOAA website 2009), such that the river overtops the dike during a 100-year event (NOAA website 2009).

Based on observations by the City during these and other flooding events, and modeling and analysis conducted by R2 Resource Consultants to assess avulsion risk in the Stillaguamish River within the vicinity of the project area, floodwaters on the site flow southwest, bypassing the treatment wetland project site, and overtop a dike associated with Dike Road to continue southwest in a "split flow channel" (R2 Resource Consultants 2005). Dike Road Dike overtops during flood events that exceed approximately 19 ft at Arlington (flood stage is 14 ft at Arlington).

According to the R2 study:

Of the six dike locations, the Johnson Dike location...was considered to be of lowest relative risk of erosion leading to avulsion in the vicinity of a breach. There is an existing breach at Location 1 that is approximately 3 feet deep and 10 feet wide, but the invert of the breach appears to be around 2 feet or so higher than the adjacent floodplain, which is relatively level. In addition, Dike Road forms a downstream grade control (elevation ~61.1 ft, NAVD88) that may restrict the flow rate through the existing breach which should prevent the breach from becoming significantly larger. The Dike Road surface is not substantially elevated above the floodplain and the downstream side slope of the road prism is gentle. These topographic factors contribute to a relatively low risk of avulsion in the vicinity of the dike because they limit the erosive forces locally in the vicinity of the dike and the road.

During the 2-, 5-, and 100-year flood events, the simulated water depths range from 0 ft (near the levee) to approximately 8 ft deep at the southern property boundary and reach up to approximately 10 ft within the WSDOT wetland (R2 Resource Consultants 2005). Flow velocity on the majority of the property, including the project site, is modeled as being in the range of 0 to 1 cfs (R2 Resource Consultants 2005).

1.8 WATER QUALITY REGULATORY REQUIREMENTS

The City has required new development and redevelopment to comply with Ecology Stormwater Management Manual for Western Washington guidelines since the mid-1990s. However, the largest contiguous storm drainage catchment in the City, 286 acres of Old Town Arlington, predates the manual's technical requirements, and most of the stormwater runoff from this area discharges untreated to the Stillaguamish River at the Butler outfall.

TMDL Plans and Arlington's NPDES permit require the City to implement actions to improve water quality. Wastewater discharge limits are defined in the City's NPDES permit. The WWTP currently discharges treated municipal wastewater into the Stillaguamish River, in accordance with NPDES Permit No. WA-002256-0; modification #1, October 13, 2006. The discharge is released through an outfall whose placement is permitted in Washington State Department of Natural Resources (WDNR) Aquatic Lands Outfall Easement No. 51-070281, October 2002, renewal in 2008.

The City is required under its NPDES Phase II stormwater permit to fully implement the requirements of the Stillaguamish River TMDL Plan. Essential elements of the Plan include: 1) treatment of stormwater from Old Town; and 2) eventual treatment of reclaimed water from the future WRF via the proposed treatment wetland. The timing of diversion of reclaimed water through the treatment wetland will depend on water quality parameters within the Stillaguamish River. TMDL parameters for the Stillaguamish River are summarized in Table 2 below.

**Table 2
Stillaguamish River TMDL Parameters**

TMDL Target in Stillaguamish River			Parameter			
			Temperature (°C)	DO	Conductivity (µmhos/cm)	Fecal Coliform (Col/100 ml)
Aquatic	Reference Condition	Recreation	(Maximum)	(Minimum)	(Maximum)	(Maximum)
Class A	Higher	Class A	18	8.0	200	100
Non-core	NA	Secondary	17.5	8.0	100	200
Core	NA	Primary	16	9.5	75	100
Char	Lower	Extraordinary	12	9.5	50	50

Source: Ecology 2005a.
 µmhos/cm = microhos per centimeter.
 Col = Colonies.
 N/A = Not applicable.

2.0 METHODS FOR TREATMENT WETLAND DESIGN

The design of the treatment wetland began with developing goals and objectives (see Table 3 below). Landau Associates and the City gathered information regarding critical areas and cultural resources present in the vicinity of the project area in order to avoid and/or minimize impacts to these areas. We gathered geotechnical information on soils and groundwater, and conducted conceptual hydrologic modeling of the wetland to determine potential residence time and discharges for stormwater and reclaimed water. We then reviewed water quality parameters to assess treatment efficacy of the planned constructed stormwater wetland, and conducted water quality analysis to determine the potential efficacy of the treatment wetland for treating pollutants of concern.

**Table 3
Project Goals, Objectives, and Design Aspects**

Goal 1: Meet NPDES permit requirements for water quality in effluent at the Butler outfall and provide additional treatment	
Objectives:	A) Reduce water temperatures in street runoff, stormwater, and/or reclaimed water effluent
	B) Increase DO content in stormwater and effluent
	C) Reduce nutrient concentration and biological oxygen demand in stormwater
Design Aspects:	A) Provide settling area for suspended solids and associated nutrients and toxicants
	B) Provide forested canopy cover to shade water and lower temperature
	C) Provide aeration of effluent
	D) Provide increased retention time and interaction with soils and vegetation for nutrient cycling
	E) Provide sink for trace metals (copper, zinc, etc.) via chemisorption with organic matter
	F) Reduce bacterial loads through deposition of sediments and infiltration
Goal 2: Reduce peak flows at outfall from stormwater and wastewater sources.	
Objectives:	A) Provide water storage to reduce peak flows to the river
	B) Provide alternative discharge location for reclaimed wastewater from City WWTP
Design Aspects:	A) Provide large area for water storage
	B) Provide long, sinuous route for water travel during low flows
	C) Provide inlets for stormwater and for reclaimed water, including bypass mechanisms
	D) Allow for periodic river flooding during storm events
Goal 3: Provide wildlife habitat within wetland and buffer area.	
Objective:	A) Provide multiple habitat types
	B) Provide connectivity to other habitats (shoreline corridor)
Design Aspects:	A) Created areas with varying water depths
	B) Install habitat features, such as bird nesting boxes, snags, rock piles and amphibian refuge
	C) Provide contiguous buffer corridor between wetland habitats and river
	D) Maintain water movement to prevent mosquito breeding
Goal 4: Provide recreational and education opportunities.	
Objectives:	A) Provide shoreline access, recreation and wildlife viewing opportunities
	B) Provide educational opportunities
Design Aspects:	A) Create public access trail along the river and through the wetland
	B) Install informational signs and viewing areas
	C) Designing functional elements like fountain and waterfall to be aesthetically pleasing
	D) Provide structure for volunteers to collect donations / sell harvested cattail bouquets
	E) Retain open space and existing buildings for future public use

2.1 SOURCES OF INFRASTRUCTURE INFORMATION

Existing infrastructure on the property and surrounding area (as shown on the drawings in Appendix B) was surveyed and documented. The following primary sources of information were used:

- AutoCAD survey data files created for the project (Huey Surveying and Land Consulting 2008)
- City Geographic Information System (GIS) maps showing storm and wastewater conveyance infrastructure (Arlington, City of 2008b)
- As-Built Plans and Designs related to SR 9, bioswales, and wetlands (WSDOT 1997)
- Snohomish County Permit Planning and Zoning Map (Snohomish County 2008a).

2.2 UPSTREAM AND DOWNSTREAM ANALYSIS

The upstream basin for the new stormwater wetland is the Old Town catchment area (Downtown Basin) in Arlington and the downstream area for the wetland is the Stillaguamish River.

2.2.1 UPSTREAM BASIN ANALYSIS

The upstream basin is the Downtown Basin, as identified in the City's Stormwater Comprehensive Plan (Arlington, City of undated). The Downtown Basin was delineated and modeled in Calculation No. HM-1 by URS in March of 2008 (URS 2008; see Appendix C). The basin's physical parameters were derived directly from the URS report to create the upstream basin model used in the calculations in this report. The pipe names and other identifiers for the two models are the same. The Downtown Basin is approximately 171 acres in size and is shown on Figure 1 of Calculation No. HM-1 in Appendix C. This area includes about 12.2 acres along West Avenue where stormwater is infiltrated and does not contribute to the Butler outfall (sub-basins W-07, W-10, and W-13). Basin BD-13, which is mentioned in the URS report, was representative of a different piping layout, and although listed in the URS calculation, is not actually used in the model (Talich, C., 2008, personal communication).

Because the URS calculation used a different modeling program (XP-SWMM) than the software used for the calculations in this report (Stormshed 3G), the flow rates at selected locations were checked against the URS modeled flow rates for identical storm events after the upstream model was created. The flow rates of the two models generally agreed, with differences within the range of approximately 5 to 10 percent.

Both models tend to over-predict the observed flow rates at identified flooding locations. Observed flooding is generally less severe than that predicted by the models. Through discussions with

URS and the City, it was generally agreed that the likely reason for this discrepancy is that the existing pipe system contains areas which are older and in a state of disrepair. These pipe segments pass through soils which have a relatively high permeability, and it is likely that significant water loss occurs from leaky pipes in the existing system.

No attempt to correct the model for this water loss was made, because the City plans to repair some of these pipes, and future water loss may be much lower than present. The stormwater wetland project was designed for peak flows from the upstream basin without any adjustment for water loss from leaky pipes.

2.2.2 DOWNSTREAM ANALYSIS

The flows from the new stormwater wetland will exit the property at the same location as they do currently, via the Butler outfall pipe, which drains directly to the Stillaguamish River. As a result, the project is exempt from flow control requirements. The creation of the wetland will result in peak flows to the river being diminished. The flows are *de minimis* to the overall river flows and no downstream impacts are expected.

2.3 DESIGN ELEMENTS

The stormwater wetland is designed to meet accepted removal efficiencies for the common parameters affecting the TMDLs, as found in the regional literature using best available science and all known, available, and reasonable methods of prevention, control, and treatment (AKART) procedures for constructed wetlands (Arlington, City of 2007). Design of the wetland was completed based on the project goals and objectives (see Table 3), critical areas, site infrastructure, and TMDLs (see Table 4 below). Grading plans were created (see Appendix B) and were used to create the hydrologic model (described in Section 3.0).

**Table 4
Pollutants, Total Maximum Daily Load Parameters, Sources, and Mechanisms for Treatment**

Pollutant(s)	TMDL Parameter	Source(s)	Treatment Mechanisms
Heat	Temperature	Stormwater, Reclaimed Water	Shading with trees and woody shrubs; infiltration through soil
Phosphorus (P)	DO	Stormwater, Reclaimed Water	Particulate P - sediment deposition in Cell 1 (settling pond); dissolved P - uptake by plants
Nitrogen	DO	Stormwater, Reclaimed Water	Cycling in wetlands – uptake by plants and denitrification in seasonally saturated areas
Pathogens	Fecal coliform (FC)	Stormwater	Sediment deposition; wetland discharge to groundwater, with secondary discharge via ephemeral channel
Oil and grease	None; can affect DO	Stormwater	Retention of free and dispersed floatable oil in baffled stilling basin (oil/water separator); deposition of sediments as for TSS
TSS	DO and FC	Stormwater	Sediment deposition in baffled stilling basin, bioswales, and wet pond cell
Metals	None	Stormwater	Settling associated with TSS and/or uptake by emergent vegetation (removal via harvest)
Gross solids	None	Stormwater	Filtration in influent bioswales and baffled stilling basin

Source: City of Arlington 2007.

2.3.1 STORMWATER WETLAND DESIGN FEATURES

Based on the project goals, objectives, and site constraints (critical areas, protection areas, topography, and existing infrastructure), a number of key elements are included in the design of the treatment wetland. The grading plan and profile and depths of cut and fill are shown on the drawings in Appendix B.

The total size of the stormwater wetland project (grading area limits) is about 8.7 acres. The wetland consists of four cells, separated by weirs, as shown on the drawings in Appendix B and as summarized on Table 5 below. Proposed habitat and public access features are also illustrated on the drawings in Appendix B. Important design features include:

- The design avoids the historic area; the historic area may be used for viewing or interpretive signage and/or for sale of harvested cattail.
- The design avoids critical area impacts, except for trail and/or planting.
- An underground pipe from Cell 4 to the Butler outfall will be installed to avoid the septic system protection area.
- A truck access road (15 ft wide) will be constructed to connect to the existing site access road (under SR 9), will span the entire length of Cell 1 and continue to the existing groundwater well (and turnaround location), and will also allow access to the wetland piping and structures (catch basins).

- The total size of the treatment wetland (area of site disturbance) is approximately 8.7 acres, including approximately 6.0 acres of created wetland area, consisting of four cells, separated by weirs (Appendix B).
- Approximately 900 cubic yards (yd³) of imported gravel material will be required to construct the road and trails.
- The grading excavation volume is approximately 29,600 yd³.
 - 14,400 yd³ of native soils will be exported from the site, and the net cut is 14,400 yd³ (8.9 acre-ft), which will provide 8.9 acre-ft of additional storage for surface water.
 - 4,500 yd³ of the remaining volume will consist of structural material (free of organics) which will be used for berms and for underneath the roadway.
 - 9,800 yd³ of the remaining volume will consist of the upper sod layer (upper 1 foot) and will be used as replacement of topsoil (after over-excavation) in order to provide a planting medium in the wetland cells and adjacent upland areas.
- A trail will be constructed around and through the wetland that is approximately 6 ft wide at the top elevation and will include viewing points.
- The design incorporates connectivity to future public access space, the proposed dike trail, the existing round house, and parking areas.
- Prefabricated footbridges are to be installed over the weirs to increase trail connectivity and access.
- The wetland will function at multiple water inflow rates while maintaining a relatively long residence time.
- The design includes multiple habitat types and public access and educational features.

**Table 5
Treatment Wetland Design Characteristics by Cell**

Characteristic	Cell			
	1	2	3	4
Primary Purpose	Settling	Storage and Treatment	Storage and Treatment	Conveyance
Water Storage Volume (Approximate)	35,000 ft ³ (to elevation 64 ft)	116,000 ft ³ (to elevation 63 ft)	123,000 ft ³ (to elevation 62 ft)	None
Bottom Elevation	59 ft	60 ft	60.25 ft	Exit at elevation 57.5 ft; Butler outfall invert at elevation 51.7 ft
Weir Information	Weir notch at elevation 63.0 ft; notch 10 ft wide and 2.5 ft deep; top of weir at elevation 65.5 ft	Two weir notches: Lower notch is at elevation 61.0 ft and is 0.5 ft deep and 2 ft wide; the upper notch is at elevation 61.5 ft and is 11 ft wide and 2 ft deep; top of weir at elevation 63.5 ft	Weir notch at elevation 60.25 ft; notch is 8 ft wide and 2.75 ft deep; top of weir at elevation 63.0 ft	N/A
Water Drop Over Weir	Water drop into Cell 2 is 1.5 ft.	Water drop from the lower notch into Cell 3 is 1 ft.	Water drop into Cell is 1.05 ft.	N/A
Primary Water Quality Treatment Parameters	TSS, Garbage, Oils and Grease	Nutrients and Toxicants	Temperature; Nutrients and Toxicants	DO
Key Water Quality Characteristic	Can be Dredged	Emergent, Shrub Vegetation, Infiltration	Tree Canopy Cover, Infiltration	Waterfall and Rough Channel Surface
Weekly (Typical) Depth of Storage	4.2 ft	1.21 ft	0.17 ft (2 inches)	None
Maximum (100-year Storm Event) Depth of Storage	5.92 ft	2.87 ft*	2.16 ft*	5.75 ft**
Other Key Features	Accessible by Truck and Foot	Slower Infiltration Rate (0.25 inch/hr)	Increased Infiltration Rate (0.5 inch/hr)	Rounded Cobble Lined Channel
Dominant Habitat Types	PEM	PEM, PSS	PEM, PSS, PFO (some UPL)	Unconsolidated Bottom/PSS

* = Controlled by lowest point in berm (overflow).

** = Occurs when there is backwater from river during high river stage and/or storm events.

TSS = Total Suspended Solids.

PEM = Palustrine Emergent.

PFO = Palustrine Forested.

PSS = Palustrine Scrub-Shrub.

UPL = Upland.

3.0 HYDRAULIC MODELING

The primary purposes of the hydraulic modeling are to:

- Determine the necessary capacity for piping into and out of the wetland to adequately convey the maximum design flow.
- Determine weir sizing.
- Predict performance of the wetland for reducing peak flow rates to the river, and estimate residence time and discharge/infiltration under various storm events.

The following primary resources were reviewed:

- Stormwater Modeling Data: Calculation No. HM-1 for the City of Arlington Stormwater Comprehensive Plan (URS 2008) (see Appendix C).
- City Wastewater Treatment Plant Engineering Report (Kennedy/Jenks 2007).

The conveyance capacity of the Downtown Basin stormwater system was modeled by URS, and the results of the calculations were provided to the City as Calculation No. HM-1. A Landau Associates civil/stormwater engineer created a streamlined version of the stormwater system model using Stormshed 3G, Version 10.0.3603.18924. The type of storm event assumed and modeled was a standard Soil Conservation Service (SCS) 24-hour Type 1A storm hyetograph, which is a storm event of peak intensity. The model was based on the existing upstream model data provided by URS (URS 2008), and the grading plan created in AutoCAD, including proposed pipes, weirs, channels, and wetland cells developed by Landau Associates based on a cooperative effort with the City.

For the stormwater source, the following five rainfall events were modeled, which are similar to those used in the URS calculations:

- Weekly event – 0.38 inches
- 6-month event – 1.26 inches
- 2-year event – 1.8 inches
- 10-year event – 2.75 inches
- 100-year event – 3.75 inches.

The weekly event was derived from rainfall data from the NOAA Hourly Precipitation Data for March 2008 (NOAA 2008). During that month, the largest event for each week was selected from the Everett data (NOAA 2008). The exception was a 1.2-inch rainfall during the fourth week, which was identified as a rainfall event nearly equal to a 6 month event and was therefore excluded. The next largest

rainfall that week (a 0.7-inch event) was selected instead. The four events selected were 0.3 inches, 0.2 inches, 0.3 inches and 0.7 inches, which averaged to 0.38 inches for an average weekly rainfall event.

3.1 ASSUMPTIONS & LIMITATIONS

The following key assumptions were used in the modeling effort:

- The data presented in URS Calculation No. HM-1 is assumed to be correct and accurate.
- Using SCS curve numbers for pervious land cover (68) and impervious land cover (98) is assumed to generate rainfall amounts that are representative of stormwater flows from the Downtown Basin after the planned stormwater-related improvements are implemented. The SCS curve numbers selected were taken from Table 2.2 in Volume III of the *Stormwater Management Manual for Western Washington* (Ecology 2005b).
- The Stormshed model developed assumes that the storm drain piping is completely repaired and does not leak. Actual storm runoff flow rates currently measured for known rainfall events appear to be lower than the flow rates given by the model, and it is assumed that the difference is due to leaks in the City storm drain piping.
- Modeling groups of existing pipes by their average physical characteristics (e.g., pipe diameter, pipe material) is assumed to provide similar results to modeling each pipe individually.
- Infiltration rates for Cell 1 and Cell 2 are assumed (based on field investigation and accounting for long-term clogging of the soils by fine sediment) to be 0.25 inches per hour and the infiltration rate for Cell 3 is assumed to be 0.5 inches per hour.

3.1.1 QUALITY CONTROL

Because the URS calculation used a different modeling program (XP-SWMM) than the software used in the calculations contained herein (Stormshed 3G), the flow rates at selected locations were checked against the URS modeled flow rates for identical storm events after the upstream model was created (see Calculation No. HM-1 in Appendix C). The input data used in the model and the model results are presented in Appendices D and E. The flow rates of the two models were generally in agreement, and the differences were within the range of approximately 5 to 10 percent. Both models tend to over-predict the observed flow rates at identified flooding locations. Through discussions with URS and the City, it was generally agreed that the likely reason for this discrepancy is that the existing pipe system contains areas which are older and in a state of disrepair. These pipe segments pass through soils with relatively high permeability. It is likely that significant water loss occurs from leaky pipes in the existing system. No attempt to correct the model for this water loss was made because the City plans to repair some of these pipes. Future water loss may be much lower than present.

3.2 BASIS OF DESIGN

There are several primary factors in determining the effectiveness of a wetland to improve water quality on an overall basis; however, the most important is retention time. In terms of bacteria reduction, two factors that are of key importance are light and temperature (EPA 2006). All three of these factors were considered in the design of the constructed wetland. In general, more light is better for bacteria reduction; ultraviolet radiation (which is present even in indirect light) helps reduce the amount of bacteria. More shade is beneficial for temperature control during spring and summer months as warmer temperatures tend to encourage bacteria growth. Retention time is of primary importance for sedimentation and influences many of the other beneficial treatment aspects of the wetland. Increased retention time means more contact time with soils for better cation exchange (e.g., dissolved metals removal), more light exposure for bacteria reduction, more time in the shade for temperature reduction, and more time for predation of bacteria within the wetland.

The one area of concern for the Stillaguamish River which is not impacted by additional retention time is DO. The wetland has several features designed to improve the DO level in the stormwater. The stormwater entering the wetland system will pass over a rock spillway which will act to increase the DO of the water as it splashes around and over the rock. Each of the weirs has a vertical elevation drop of 0.9 to 1.5 ft for water to splash/mix into the next cell. At each of these locations, the splashing action will tend to raise the DO level of the stormwater.

The above factors were all considered as the wetland was designed. In order to provide for some temperature control, trees are to be planted in portions of the wetland area. However, other portions of the wetland will remain exposed to sunlight to encourage bacteria reduction by ultraviolet radiation exposure. Sedimentation is being encouraged in two ways. The wetland is being built as large as possible within the available area to encourage as much retention time as possible. An additional benefit of maximizing the wetland area is that infiltration into the native soils is also maximized. As water infiltrates into the soil within the wetland, a significant portion of the sediment will be filtered out and deposited. The amount deposited will vary by storm event; however, in general, the smaller the storm event, the greater the percentage of the sediment load that will be deposited and retained within the wetland. The amount of water infiltrated for each storm event is summarized in Table 6 below (also see Model Results in Appendices D and E).

Table 6
Anticipated Stormwater Infiltration and Event Volume

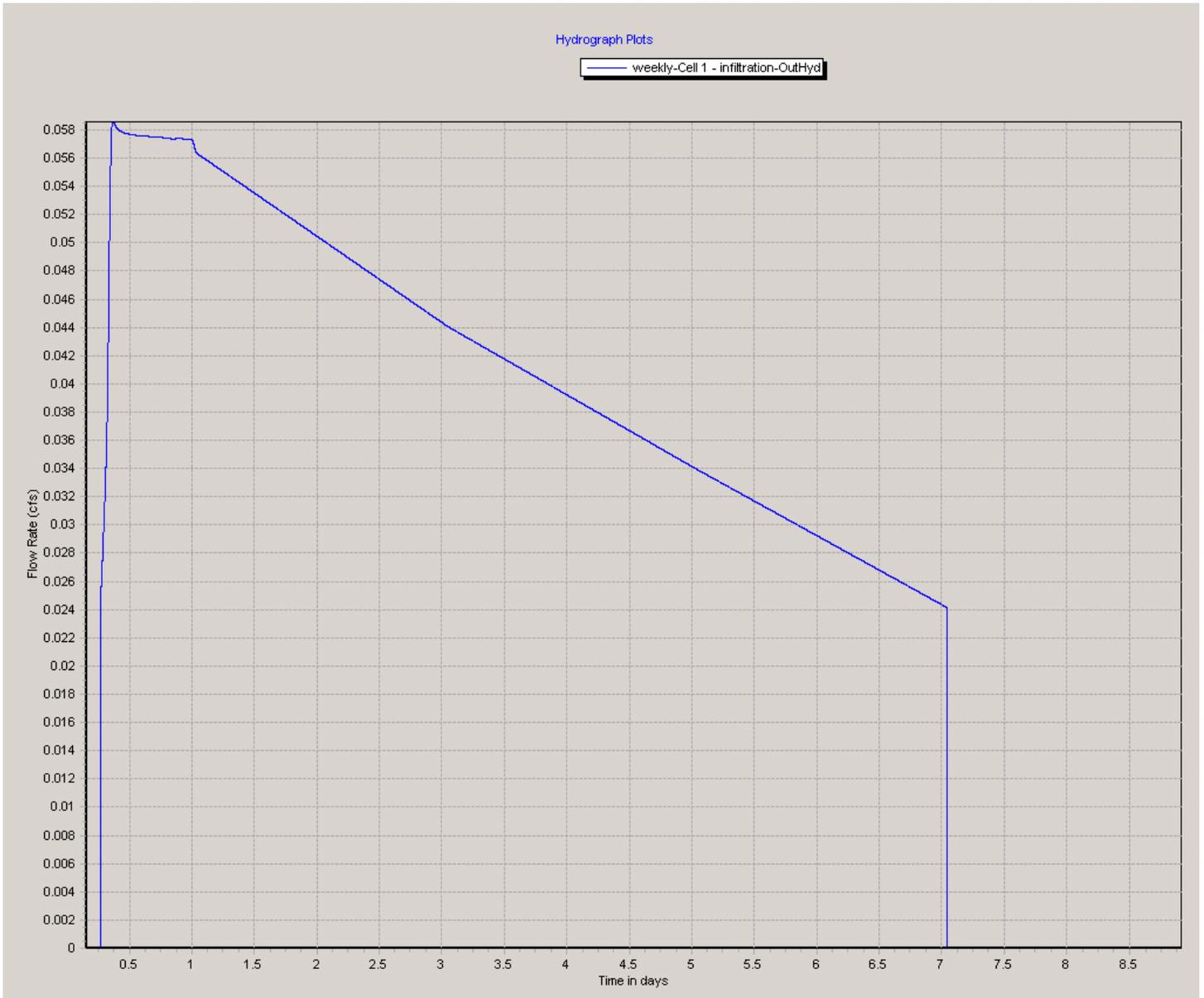
Storm Event	Infiltration Volume - Cell 1 (ft³)	Infiltration Volume - Cell 2 (ft³)	Infiltration Volume - Cell 3 (ft³)	Discharge Volume to River (ft³)	Total Volume (ft³)	Percent Infiltrated
Weekly	24,145	56,149	25,888	177	106,359	99%
6 Month	25,196	82,370	65,940	376,076	549,582	32%
2 Year	25,503	86,783	71,934	690,148	874,368	21%
10 Year	25,863	91,768	79,633	1,320,003	1,517,267	13%
100 Year	26,138	95,175	85,703	2,007,003	2,214,019	9%

3.3 TREATMENT TIME MODEL RESULTS

3.3.1 WEEKLY EVENT

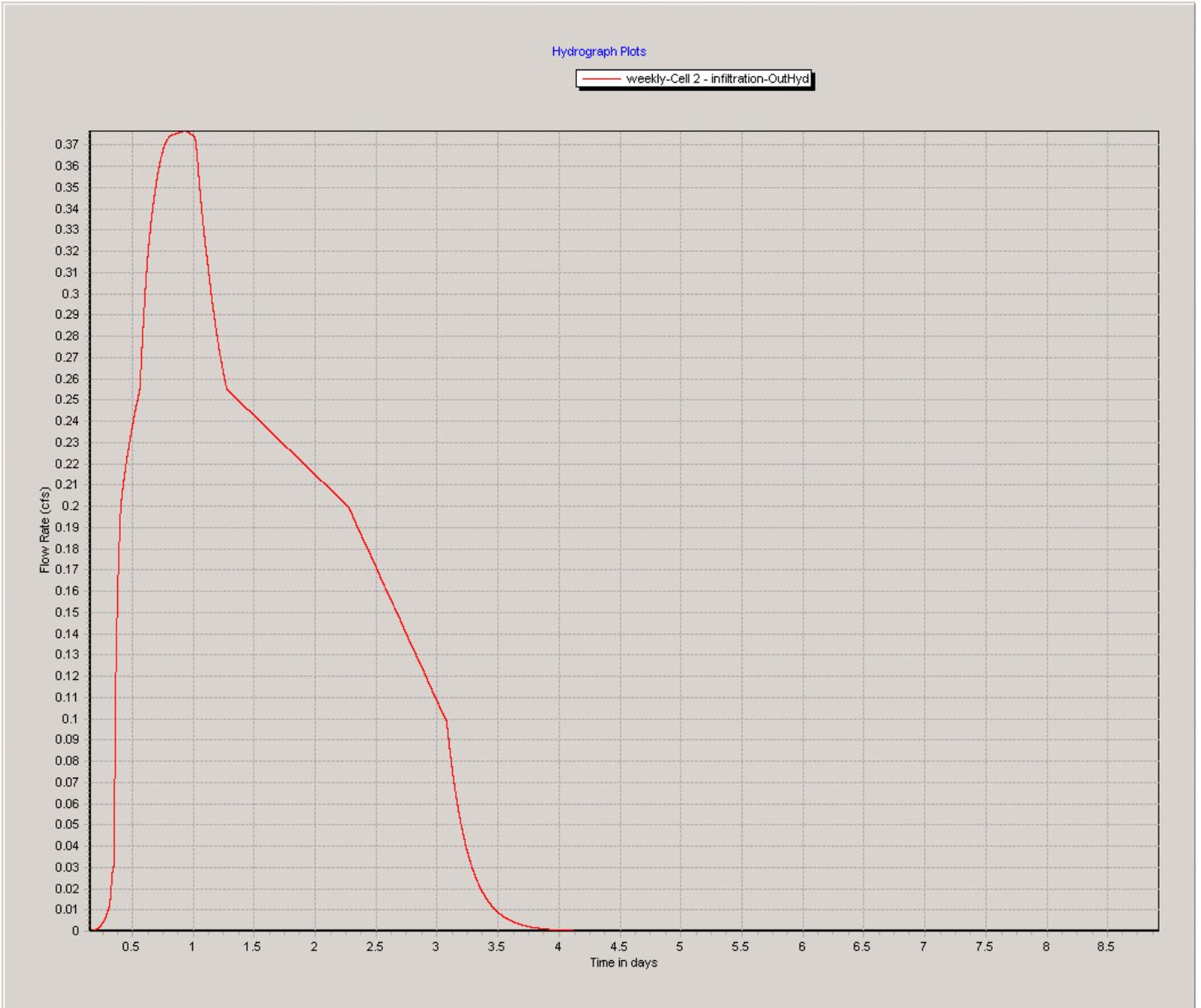
The weekly and smaller storm events represent the majority (over 90 percent) of all anticipated rainfall events. Water storage within the wetland under various size storm events (examples used consist of the weekly and 10-year storm events) are illustrated on Figures 4, 5, and 6. For the weekly event (0.38 inches of rainfall), almost all of the water which previously flowed from the Downtown Basin into the Stillaguamish River will typically be infiltrated in the wetland cells. The treatment time for this event is largely based on the amount of time it takes water to infiltrate in Cells 2 and 3. Cell 2 has a longer treatment time and stores/infiltrates approximately 50 percent of the stormwater during the smaller storm events. In addition, almost all of the water that enters Cell 3 is passed through Cell 2. The treatment time for each cell is defined primarily by the amount of time water stays in the wetland cells before it is infiltrated. Charts indicating the modeled infiltration rate over time in each of the wetland cells for a weekly event (0.38 inches of rain in 24 hours) are presented below.

Chart 1: Cell 1 Weekly Event



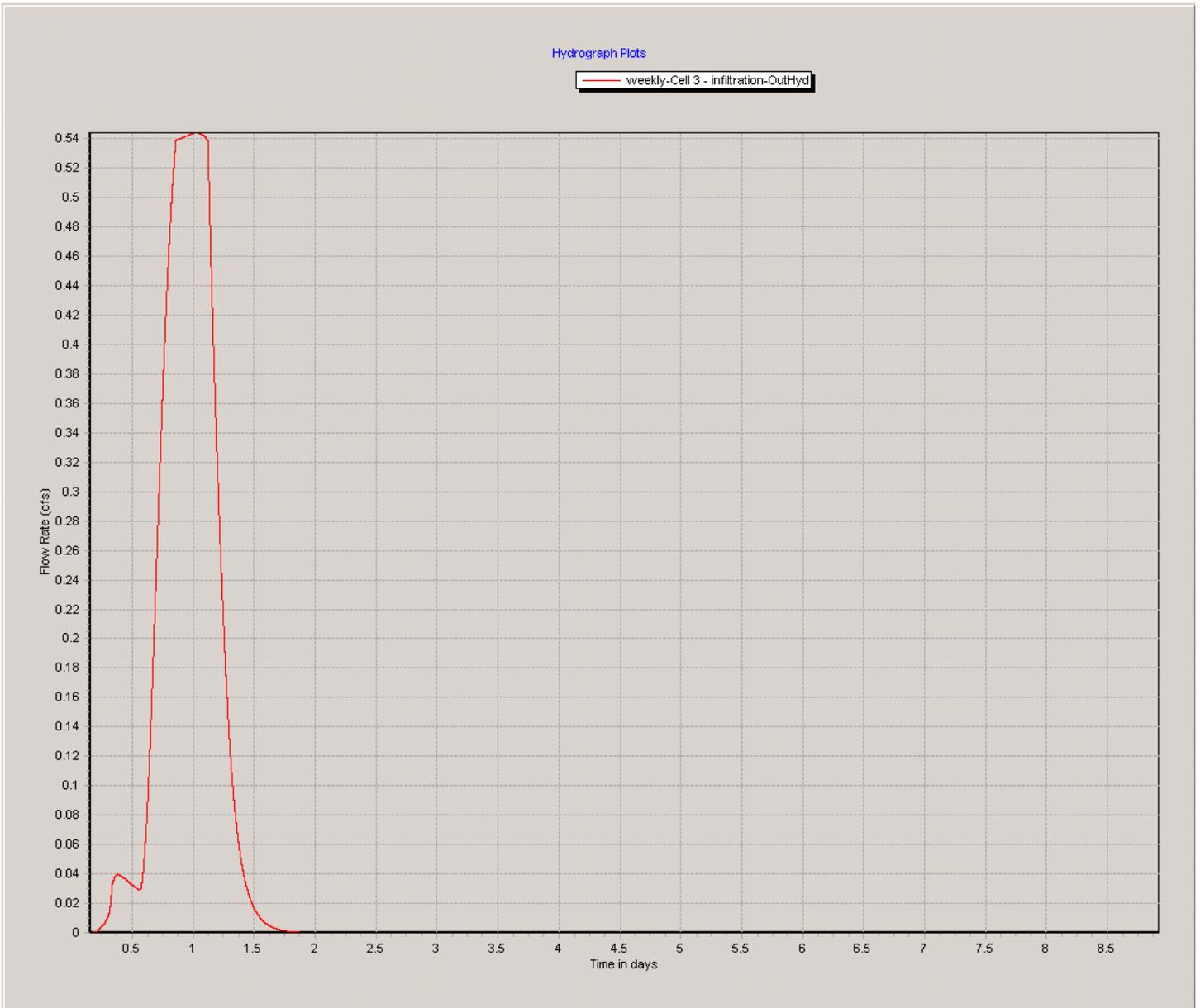
Cell 1 fills fairly rapidly with several feet of water, and then maintains a relatively steady infiltration rate (based on the relatively steady water elevation) until the stormwater runoff into Cell 1 and the flow over the Cell 1 weir stops; after it stops, the retained water is slowly infiltrated over time. The infiltration drop-off shown on day 7 is when the Stormshed model stopped calculating the infiltration and is not a true representation of infiltration conditions. During the winter, Cell 1 likely refills with water from a follow-on storm event before it completely drains. About 25 percent of the water flow entering Cell 1 never leaves Cell 1 from a weekly event when Cell 1 begins the event empty. If Cell 1 starts the event partially saturated, then more water will be passed into Cells 2 and 3.

Chart 2: Cell 2 Weekly Event



For the weekly storm event, water is anticipated to be stored in Cell 2 for almost 4 days. Some of the water in Cell 2 is infiltrated immediately, but the average treatment time for the majority of the water is likely to be approximately 1 to 2 days.

Chart 3: Cell 3 Weekly Event



For the weekly storm event, Cell 3 stores water for only about a half of a day after the rain event stops, and thus, the average treatment time for this portion of the water flow (about 25 percent) is approximately a half of a day.

3.3.1.1 Comparison to Existing Conditions

For the weekly storm event, all of the stormwater from the Downtown Basin is currently routed without treatment through the Butler outfall to the Stillaguamish River. After project completion, all of

the stormwater from the weekly event will be routed to the constructed wetland where it will be treated and infiltrated adjacent to the Stillaguamish River.

3.3.1.2 Treatment Summary

For the weekly storm event, the wetland stores water in each of the three cells after the rain event ends for periods ranging from approximately one-half day to several days. This treatment time will serve to provide initial treatment for fecal coliform and temperature. DO will be increased by the flows over the spillway and weir drops.

Infiltrated water will have some additional treatment time in the soil before reaching the existing shallow aquifer, which will provide additional treatment for fecal coliform. It is also expected that the temperature of the infiltrated water will change to match the surrounding soil temperature at the same time.

During the winter months, rain typically falls every few days; thus, it is expected that the bottom of the wetland will typically be saturated with stormwater during the winter months. During the late spring to early fall, rainfall is much less frequent, and it is expected that the bottom of the wetland will not typically be saturated during this period. Overall treatment effectiveness for smaller storm events is expected to be similar regardless of the time of year.

3.3.2 6-MONTH AND LARGER EVENTS

The larger storm events occur much less frequently than the smaller storm events and the primary treatment differences between the smaller events and the larger events are the percentage of water infiltrated and the amount of water which flows through the wetland (see Table 6 and Figures 5 and 6).

Even for the 6-month event, which is the most common of the larger storm events, the site will infiltrate only about a third of the stormwater. The remaining stormwater is retained in the wetland for a varying period of time. The average retention time could be as short as a few hours, or as long as a few days. In general, larger storm events will have shorter average retention times, as once the wetland fills with water (about 400,000 ft³ of water), the rest of the flow is passed through the wetland. Retention time will be increased when backwater effects from the river are occurring (see Section 3.7).

3.3.2.1 Comparison to Existing Conditions

For the larger storm events, all of the stormwater from the Downtown Basin is currently routed without treatment through the Butler outfall to the Stillaguamish River. Some of the stormwater is

flooded during larger storm events either onto the property/WSDOT wetland or out catch basins on the east side of SR 9.

After project completion, all of the water from the larger storm events will be routed to the constructed wetland, where it will be treated, and between about 9 and 32 percent of the stormwater will be infiltrated adjacent to the Stillaguamish River. Most of the treated stormwater will be released through the Butler outfall to the Stillaguamish River. Some of the stormwater will be flooded during larger storm events either onto the property/WSDOT wetland and/or out catch basins on the east side of SR 9.

3.3.2.2 Treatment Summary

For the larger storm events, the wetland typically stores a portion of the runoff from the event for several days after the rain event ends. This treatment time will serve to provide initial treatment for fecal coliform and temperature. DO in the runoff will be increased by the flows over the spillway and weir drops.

Infiltrated water will have some additional treatment time in the soil before reaching the existing shallow aquifer, which will provide additional treatment for fecal coliform. It is also expected that the temperature of the infiltrated water will change to match the surrounding soil temperature at the same time.

3.4 WEIR SIZING

The design basis for the weirs was avoidance of downstream interference of the functionality of the weirs during the design storm events (weekly to year storm events). Therefore, each weir was sized so that, for the weekly and year storm event, the peak water storage level behind a weir would be less than the weir elevation upstream of it, or, in the case of Weir 1, the elevation of the water flowing into the settling pond behind Weir 1. The weirs are free of interference up to the 2-year storm event, with the exception of Weir 3, which is interfered with slightly by the inlet capacity of the downstream culvert. During the larger storm events, the weirs are intended to allow backwater to occur back through the wetland so that flooding can mimic pre-existing conditions (see Section 3.7).

The weir widths were interactively changed in the model until the maximum desired storage elevation was achieved behind each weir for the design storm events. The weir widths, bottom elevations, and top elevations selected and modeled are presented in Table 7 below.

**Table 7
Weir Summary**

Weir	Notch Width	Notch Elevation	Top Elevation	Peak Water Elevation				
				Weekly Storm	6 Mo. Storm	2 Yr. Storm	10 Yr. Storm	100 Yr. Storm
Weir 1	10	63.0	65.5	63.21	64.02	64.34	64.75	64.92
Weir 2 Lower	2	61.0	63.5	61.21	62.00	62.29	62.65	62.87
Weir 2 Upper	11	61.5	63.5	N/A	62.00	62.29	62.65	62.87
Weir 3	8	60.25	62.5	60.18	60.94	61.51	61.81	62.16

The functionality of Weir 3 is interfered with by backwater effects from the downstream culvert entrance in all events larger than the 2-year storm event. The Stormshed software shows that a weir width of 8 ft is sufficient to pass the peak flow through this weir at a depth less than the maximum depth of the weir. The same is true for Weir 2. Weir 2 is similarly interfered with by backwater effects by 10-year and larger storm events. See Section 3.7 for a more complete discussion of backwater effects from the pipe system and high river levels.

3.5 SPILLWAY SIZING

There are two spillways associated with the project. One is at the west end of the bore pipe under SR 9 as the stormwater flow enters into the wetland. The other is a spillway from Cell 2 into the WSDOT wetland.

3.5.1 BORE PIPE SPILLWAY

The bore pipe spillway is designed based on the 100-year flows into the wetland from the 48-inch diameter pipe to be installed under SR 9. The model calculates the peak 100-year flow out of this pipe to be approximately 86.12 cfs, flowing at a velocity of 9.5 ft per second.

The proposed design is for a riprap-lined spillway that is 2 ft deep, including 0.5 ft of freeboard. The spillway is designed to be a minimum of 10 ft wide at the base and have a horizontal to vertical ratio (H:V) of 5H:1V interior side slopes. With these constraints, the spillway will pass the 100-year flow, as shown by the following equation.

$$Q_{100} = 3.21(LH^{3/2} + 4H^{5/2}) \text{ For a discharge coefficient } C= 0.6 \text{ and } 5H:1V \text{ side slopes.}$$

Using a length (L) of 10 ft and a flow depth (H) depth of 1.5 ft, the flow capacity of the spillway is:

$$Q_{100} = 3.21((10)(1.5)^{3/2} + 4(1.5)^{5/2})$$

$Q_{100} = 94.35$ cfs, which is more than the calculated 100-year flow rate of 86.12 cfs (see Appendix D, Figure D-6); therefore, a spillway with 5H:1V side slopes, 1.5 ft of flow depth, and a 10-ft-wide channel bottom is sufficient.

3.5.1.1 Spillway Rock Sizing

Because portions of the bore pipe spillway channel are within the WSDOT right-of-way, guidance from the WSDOT *Highway Runoff Manual* was used to size the rock (WSDOT 2008). From Table 5.4.3, Rock Protection at Outfalls, the rock thickness must be 2 ft thick with a geotextile fabric between the rock and the native soils in the spillway channel (WSDOT 2008). The rock will be riprap, which is reasonably well graded, with a gradation as follows:

Maximum stone size:	24 inches (nominal diameter)
Median stone size:	16 inches
Minimum stone size:	4 inches

3.5.2 WSDOT WETLAND SPILLWAY

The culvert beneath SR 530 that drains the WSDOT wetland has not been surveyed; however, based on observation by Kelley Wrigg, Landau Associates, the culvert appears to be a 24-inch-diameter corrugated metal pipe. The invert of the outlet on the north side of SR 530 appears to be at an elevation close to 59 ft.

This culvert does not appear to have the capacity to be the primary relief flow direction when these properties are flooded. It likely does not have the capacity to pass the flow volumes which can pass over the top of the dike during river flooding events. Instead, flood elevations on the property are controlled by the height water builds up as it flows over Dike Road to the west. No modeling has been performed to attempt to predict these flow depths. During flood events in January 2009, water filled to an elevation of about 66 ft on the property and the WSDOT property. This would appear to be typical of small flood events from when the river elevation exceeds the river dike elevation. At this water surface elevation, water is flowing approximately 4 ft deep over the top of Dike Road at the west side of the property. The water is about 7 ft deep (about 3.5 culvert diameters) at the 24-inch culvert under SR 530.

A nomograph, shown on Figure 7, is adapted from Figure 4.3.1.C of the King County *Surface Water Design Manual* (King County 2005). From the nomograph, for the projecting section pipe inlet, the flow that the 24-inch pipe inlet would support is about 33 cfs.

In order to mimic pre-existing conditions to the extent possible, the spillway from Cell 2 to the WSDOT wetland has been sized to provide as much flow as the culvert has been estimated to be able to pass (33 cfs). The spillway bottom elevation is set at an elevation of 62.5 ft, and the flow depth (H) will be only 0.5 ft. The spillway side slopes will be at 5H:1V.

The length (L) of the spillway channel required to pass a flow (Q) of 33 cfs was determined using the following equation:

$$L = \left[\frac{Q}{3.21H^{3/2}} \right] - 4H, \text{ or}$$

$$L = \left[\frac{33}{3.21(0.5)^{3/2}} \right] - 4(0.5), \text{ or}$$

L = 27.1 ft. Therefore, the WSDOT wetland spillway width has been set at 28 ft, as shown on the drawings in Appendix B.

3.5.2.1 Spillway Rock Sizing

The spillway rock will be quarry spalls with a geotextile fabric between the rock and the Cell 2 berm soils. The rock thickness will be 1 ft, with a gradation of the quarry spalls, as follows:

Passing 8-inch-square sieve:	100%
Passing 3-inch-square sieve:	40% to 60% maximum
Passing 0.75-inch-square sieve:	0 to 10%

3.6 PIPE SIZING

The basis for the wetland project pipe sizing was that the pipes be able to pass the 25-year storm event, and also be analyzed for the 100-year storm event. As noted earlier, the storm flows modeled in this report and by URS appear to generate flows which are larger than those actually observed. This is believed to be the result of water loss in the system through leaking conveyance pipes. The pipe calculations reflect no system water loss and represent a maximum flow rate analysis.

The capacities of all the new pipes in the system were modeled for the 100-year storm event as the pipe segments are proposed to be installed. The model of the upstream system is a representative model and each reach (pipe link) often represents several pipes, where the physical properties have either been totaled (length), averaged (slope), or assumed worst case (manning's number or pipe diameter). It is not the purpose of the model to identify upstream deficiencies, but rather to create a representative flow to the new wetland system in order to model the wetland as accurately as possible. There are some upstream deficiencies, which have been noted by previous studies and which this model also identifies. No recommendations for these pipes are made as part of this drainage report because the City has already identified these issues and is working on solutions.

All of the new proposed pipes have the capacity to pass not only the 25-year storm event, but the 100-year storm event as well. The pipe with the lowest remaining capacity is the 48-inch pipe DM5A to Cell 1 to be installed beneath SR 9. This pipe is at 80 percent capacity during the modeled 100-year storm event peak flow (see Model Results in Appendices D and E).

3.7 FLOODING, BACKWATER, FLOOD HAZARD, AND ZERO RISE ANALYSIS

Note that the river gages and FEMA FIS and FIRM datum are related to NGVD29. The site plans and the calculations in this report are based on NAVD88. The conversion for the project location is to add 3.69 ft in elevation to the NGVD29 elevation to obtain the NAVD88 elevation. All discussions of elevations in this report reference the NAVD88 datum, unless otherwise noted.

Elevation references:

- The river stage varies between 58 ft (0 flow) and 71 ft (100-year flow).
- The dike along the river is between 68 ft and 72 ft.
- The Butler outfall invert is at an elevation of 51.7 ft (URS 2008).
- The WSDOT bioswale edge has an elevation of about 65 ft.
- To convert the river gage at SR9 to project datum (NAVD88), add 51.7 ft to the river gage reading.

3.7.1 FLOODING AND BACKWATER

Currently, when the river elevation rises to close to 66 ft, water flows back up the Butler outfall pipe and bubbles up in CB#6 at the east end of the Butler outfall (see photos below). In these cases, the flow begins to fill the WSDOT bioswale when the river elevation rises above 61 ft, and then spills onto

the rest of the property from the bioswale when the river elevation rises above 65 ft. Water can also flood onto the property when flows in the river exceed the height of the river dike along the property's northern side. Both of these scenarios occurred during the January 2009 flooding event. There was a breach in the dike during this flood event which has since been repaired. The small flooding events which occur when river water is flowing back up the pipe system (but these flows do not exceed the existing dike height) can be prevented by the installation of a backwater prevention device (such as a flapper gate) on the Butler outfall. Because the Butler outfall pipe is currently in need of repair, it is recommended that the installation of a backwater prevention device happen concurrently with future pipe repair activities.

After the wetland is constructed, during times in which flood water passes over the dike and onto the property, the wetland will provide little or no treatment of stormwater. However, once floodwaters subside enough so that flows can exit the property through the outfall, the wetland will again start to provide some level of treatment of not only stormwater, but the retained floodwaters as well. The deposition of silt during flooding events is not anticipated to adversely impact the wetland, as the soils which are typically deposited by flooding events are the same types of soils that will be present in the wetland. Routine maintenance of the treatment wetland will include removal of accumulated sediment in Cell 1 (the sediment basin) in order to restore original treatment wetland design elevations.

3.7.2 FLOOD HAZARD AND ZERO RISE

The creation of the wetland will result in about 8.9 acre-ft of soil material being removed from the site (see the grading plans in Appendix B) or the creation of about 8.9 acre-ft of additional flood storage. This additional storage will either be used by stormwater from the City before it enters the river or by the river itself when it exceeds the height of the existing dike. In either case, the additional storage provided by the wetland excavation results in lower flood elevations on the property and downstream properties than would occur under existing conditions. Thus, the proposed project provides a net loss of flood elevation, which exceeds the zero rise requirement.



Photo 2: Water back-flowing from river outfall and bubbling up through catch basin lid.

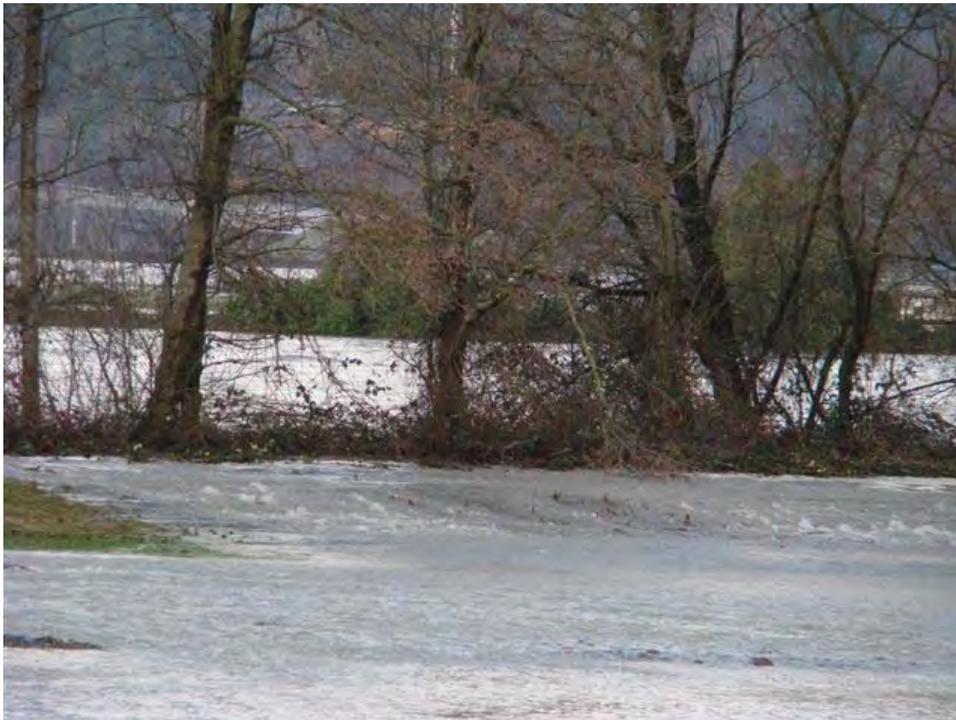


Photo 3: Previous breach in dike near western end of property.



Photo 4: Looking east across property from Dike Road during January 2009 flooding event



Photo 5: Looking at northeastern portion of property from SR 9.

3.7.3 POST-CONSTRUCTION FLOODING

The wetland has been designed with the intent to preserve existing flooding patterns to the extent possible. Data on exactly how much water flows, and in what directions, during flood events is not available. However, pictures of previous flood events, and elevation data gained through survey of the property and adjacent areas, help make a likely determination of flood flows possible.

3.7.3.1 Existing Condition Comparison

Once flood waters flow over the dike onto the property, they start to fill the property with water and the flood flows can exit the property in two directions. The two existing exit points are at about the same elevation. One pathway is to the southeast to the WSDOT wetland property, and the other is to the south across the low point on Dike Road. Because the WSDOT wetland property is constrained by fill on two of its three sides and by the property on the other, there is only one exit for flood waters from the WSDOT wetland. That exit is through an approximately 24-inch culvert, which passes under SR 530 to the south. Surface water flowing south to north is what currently appears to help sustain the WSDOT wetland. However, during flood events, water can flow through the culvert to the south. The amount of water which can flow through the culvert is constrained by the size of the pipe (approximately 24-inches) and the head (height) of the flood flows that build up can force water through the pipe. Once the flood waters on the WSDOT property equalize with the flood waters on the property, the flow onto the WSDOT property is constrained by what flow can pass through the 24-inch pipe. Indications are that flood water depths are often less than elevation 66 ft, but could be higher during a major flood event. The 24-inch culvert inlet with 5 diameters of head (10 ft) can only pass about 40 cfs of flow, and 40 cfs is a small fraction of the amount of flow that can pass over Dike Road at the same flood elevation (about 68 ft).

To achieve this level of flow, the water elevation would have to be 68 ft over both the WSDOT and proposed wetlands (well above the wetland berm) and the impact of the overflow spillway would be negligible. For this reason, the lower headwater depth was used in the calculations of the spillway from Cell 2 to the WSDOT wetland.

3.7.3.2 Wetland Filling During a Flood Event

It is assumed for the purposes of the discussion in this section that a flapper gate has been installed on the Butler outfall to prevent backwater from the river and that it rains during the river flood event.

As the river level begins to rise, the Butler outfall's ability to sustain drainage flow will diminish as the tail water in the river increases. As the river level rises, the flapper gate will be forced closed. The

wetland will initially fill with water from the storm drainage system draining to the wetland. Cell 4 will fill to an elevation of 60.25 ft, and once the water level exceeds that elevation, then the operation of Weir 3 will be diminished by tail water in Cell 4 until the two cells equalize in elevation from water continuing to flow into the system. Once they equalize, the two cells will continue to fill, until the water in Cells 3 and 4 reach an elevation of 61.0 ft. At this point, the performance of the lower Weir 2 notch will begin to diminish flows from Cell 2 into Cell 3. At an elevation of 61.5 ft, the flows through the upper Weir 2 notch will begin to diminish, and eventually, at a water elevation above 61.5 ft, Cells 2, 3, and 4 will equalize in elevation and begin to rise together. The water in these cells will continue to rise until the water elevation reaches 62.5 ft. At this elevation, water will begin to flow through the overflow spillway into the WSDOT wetland. This spillway has been sized to support a flow of up to 33 cfs. If the flow into the project wetland exceeds 20 cfs for a long enough time, then the WSDOT wetland will fill to an elevation of 63 ft. At that elevation, water in the project wetland will also reach an elevation of 63 ft, and water will flow out of the project wetland on the north side of the wetland and eventually cross Dike Road.

If the river elevation exceeds the existing dike elevation, then river water will rapidly fill the project wetland and the WSDOT wetland by backwatering through the system, as described above. In the case of river flooding, the property will flood, as it flooded before the project, but with about 8.9 acre-ft of additional flood storage on the property, which will reduce flood impacts to downstream properties.

4.0 RECLAIMED WATER

In the future, it is the City’s goal to redirect some of the reclaimed water from the Wastewater Reclamation Facility (WRF; under construction) to provide hydrologic support to the wetland during the summer months. Flows from the WRF will result in a different hydroperiod than stormwater flows in that, at a given WRF input, the wetland will fill to a maximum level (which varies depending on the pumping rate) and will remain at that level until the WRF pumps are turned off, as summarized in Table 8 below.

**Table 8
Hydroperiod of Stormwater and Reclaimed Water**

Water Source	Variable	
	Timing	Hydroperiod
Stormwater	Year-round; primarily winter	Rises to peak and then immediately starts to diminish until the rainstorm event subsides
Reclaimed water	Primarily summer	Starts at a maximum flow rate and continues at that rate until the flow is turned off

It is proposed that the WRF discharge reclaimed water to the wetland at least once a week in a volume equal to that predicted for the weekly storm event (about 2.4 acre-ft) to help sustain the wetland during the dry summer months. However, the WRF discharge could be limited to those weeks in which any of the monitored TMDLs for the Stillaguamish River is exceeded. The volumes of water predicted to enter the wetland from each of the design storm events is listed in Table 9 below.

**Table 9
Flow Volume to Wetland by Storm Event**

Storm Event	Storm Event				
	Weekly	6 month	2 year	10 year	100 year
Flow Volume to Wetland	2.4 acre-ft	12.1 acre-ft	19.2 acre-ft	33.3 acre-ft	64.0 acre-ft

Although the wetland will fill differently from steady state pumping than from a storm event, if the volume of water pumped is similar, the wetland should fill to approximately the same depths. The addition of the reclaimed water during the dry summer months would help sustain the wetland. The amount of water pumped should be varied based on the City’s monitoring of the wetland status (more or less water based on the wetland’s response to the reclaimed water). Reclaimed water should not be pumped if substantial precipitation is predicted in the next 24 hours.

5.0 CONCLUSIONS

The proposed treatment wetland design is expected to reduce peak stormwater flows to the Stillaguamish River, decrease the overall volume of urban stormwater runoff discharged to the river by increasing the amount of infiltration, increase the detention time of stormwater runoff prior to its discharge to the river, offer additional summer month treatment of City wastewater discharge, and thereby reduce the overall pollutant load to the river from both stormwater and wastewater. The result will be an overall improvement in the water quality of the Stillaguamish River, including, but not limited to, TMDL parameters.

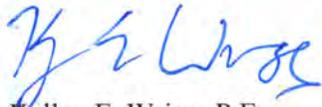
The weir system for the treatment wetland was designed to hydraulically handle the 100-year 24-hour storm event, while causing less flooding than occurs in the existing conditions. The design is such that typical and floodwater hydrologic patterns will be similar under current and proposed conditions, except that there will be increased storage capacity and desynchronization of flows as a result of the proposed project. The project will also increase public access to shoreline areas by creating trails and viewing areas, and will improve habitat functions for wildlife by creating multiple wetland communities. The existing groundwater well and the historic area will be preserved and protected (see Critical Areas Report; Landau Associates 2009).

6.0 USE OF THIS REPORT

This report was prepared for use by the City of Arlington and applicable regulatory agencies. No other party is entitled to rely on the information, conclusions, and recommendations included in this document without the express written consent of Landau Associates. Further, the reuse of information, conclusions, and recommendations provided herein for extensions of the project, or for any other project, without review and authorization by Landau Associates, shall be at the user's sole risk.

This document has been prepared under the supervision and direction of the following key staff.

LANDAU ASSOCIATES, INC.

A handwritten signature in blue ink, appearing to read "Kelley E. Wrigg".

Kelley E. Wrigg, P.E.
Associate Engineer

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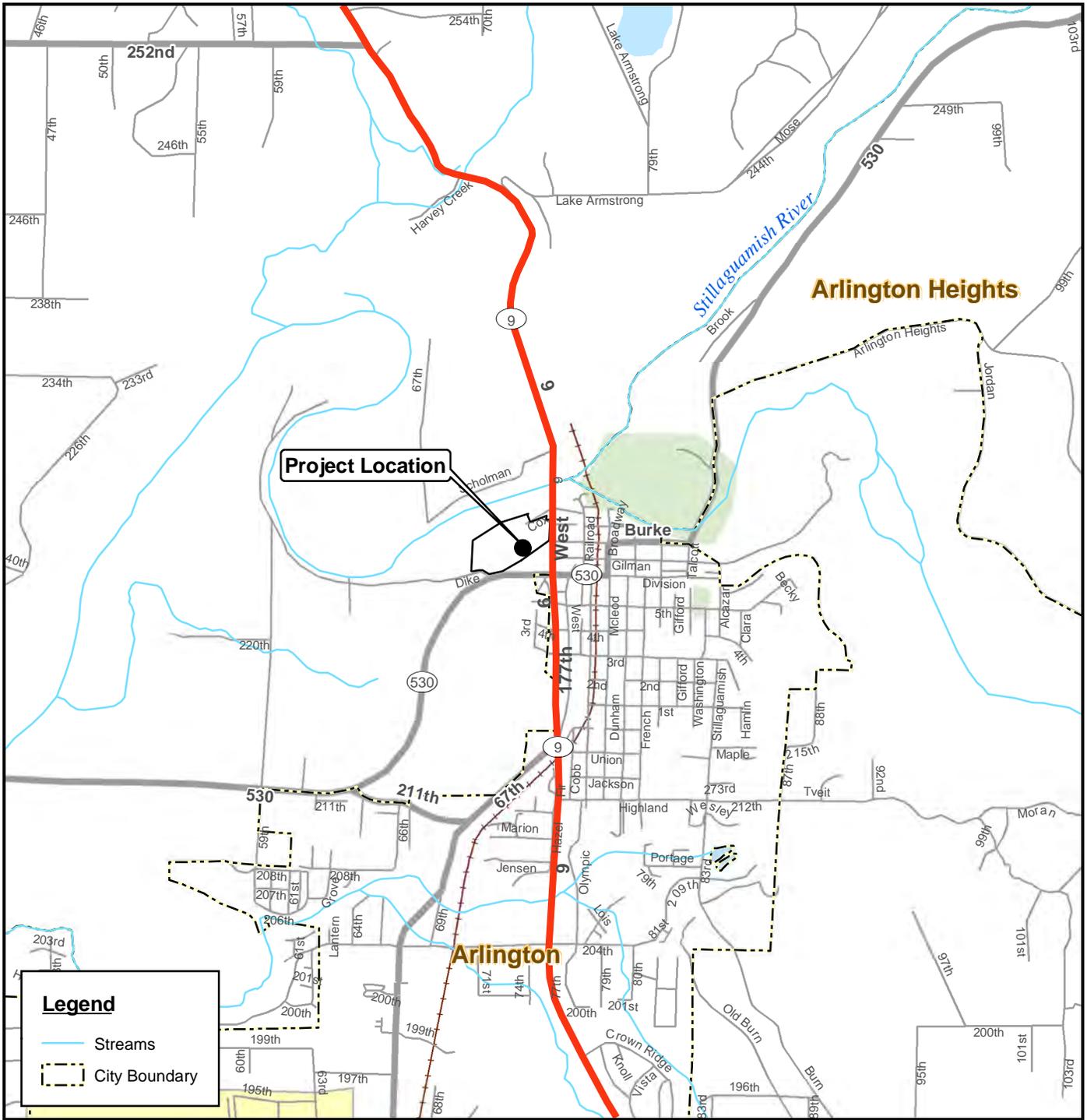
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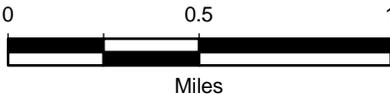
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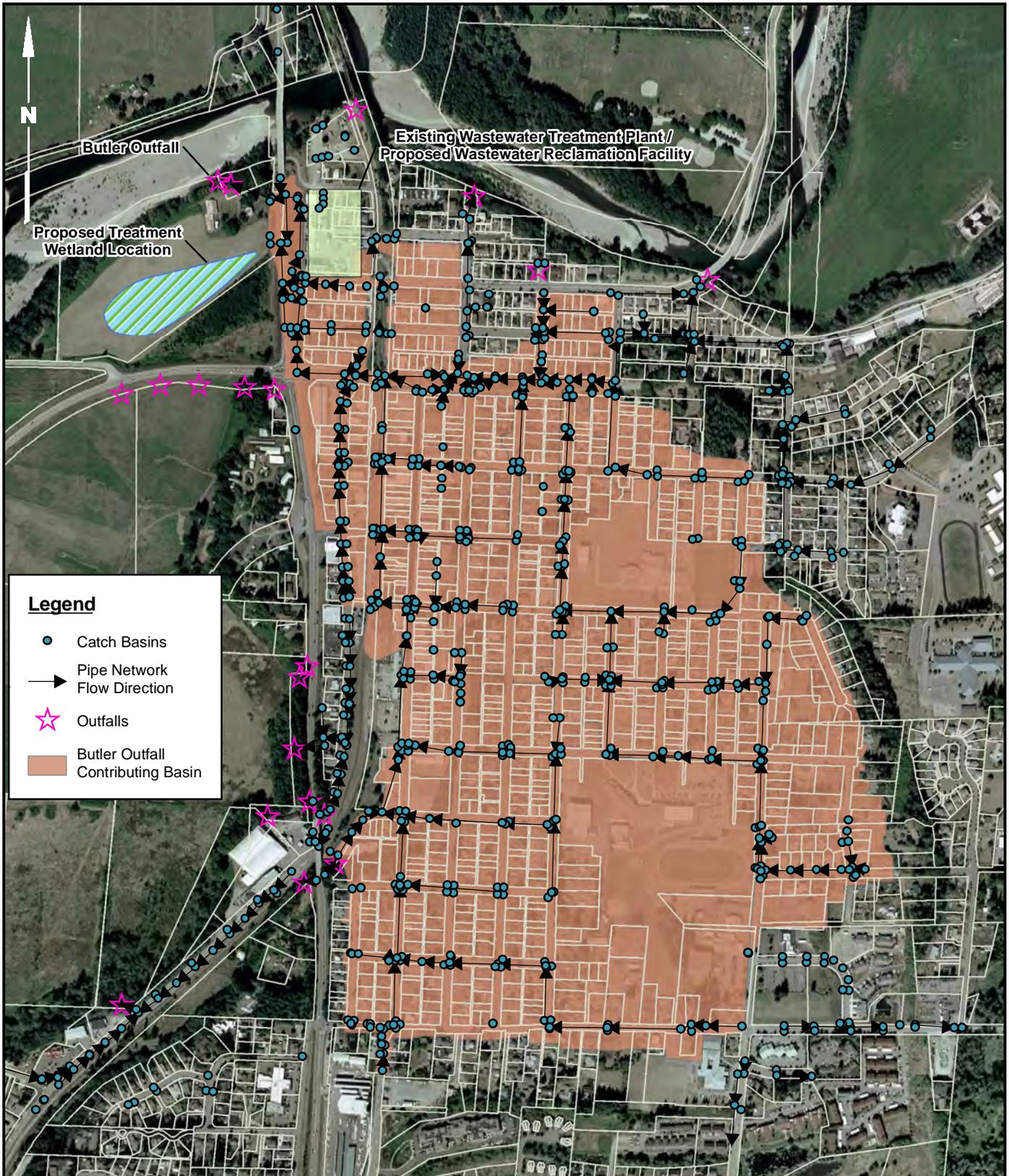
Data Source: ESRI 2006



City of Arlington
Stormwater Wetland Project
Snohomish County, Washington

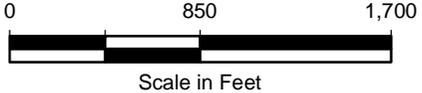
Vicinity Map

Figure
1



Legend

- Catch Basins
- Pipe Network Flow Direction
- ★ Outfalls
- Butler Outfall Contributing Basin



Data Source: City of Arlington; ESRI Image Server

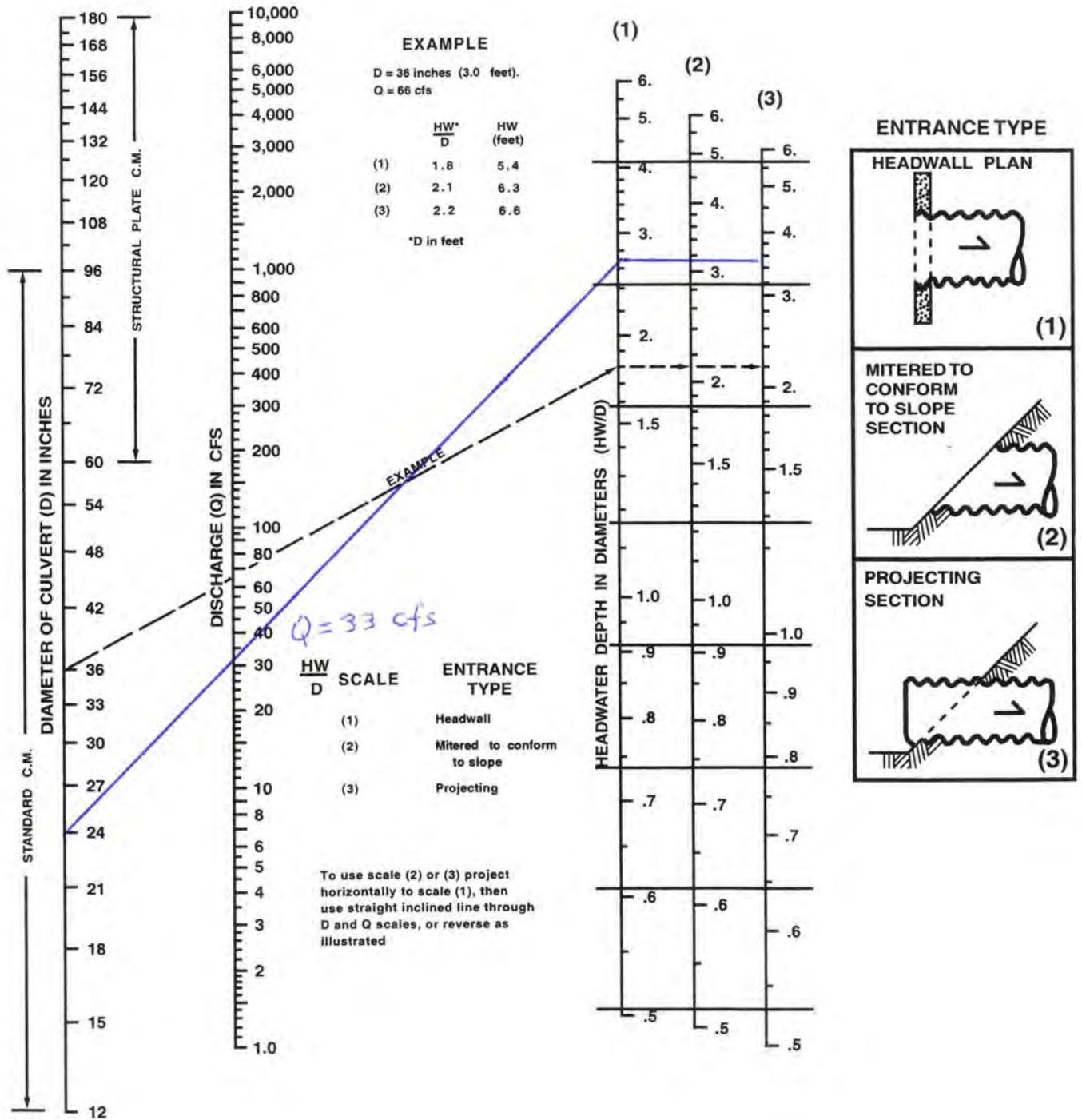
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City of Arlington
 Stormwater Wetland Project
 Snohomish County, Washington

Stormwater Basin

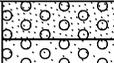
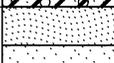
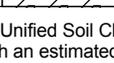
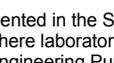
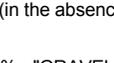
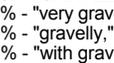
Figure
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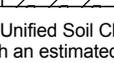
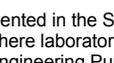


Adapted from: Figure 4.3.1.C of King County 2005 Surface Water Design Manual

**Soil Logs and Laboratory Analysis and Soil
Descriptions from the Natural Resources
Conservation Service**

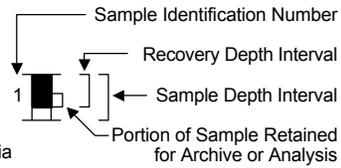
Soil Classification System

	MAJOR DIVISIONS	USCS GRAPHIC SYMBOL	USCS LETTER SYMBOL ⁽¹⁾	TYPICAL DESCRIPTIONS ⁽²⁾⁽³⁾
COARSE-GRAINED SOIL (More than 50% of material is larger than No. 200 sieve size)	GRAVEL AND GRAVELLY SOIL (More than 50% of coarse fraction retained on No. 4 sieve)	CLEAN GRAVEL (Little or no fines)	 GW	Well-graded gravel; gravel/sand mixture(s); little or no fines
		GRAVEL WITH FINES (Appreciable amount of fines)	 GP	Poorly graded gravel; gravel/sand mixture(s); little or no fines
	SAND AND SANDY SOIL (More than 50% of coarse fraction passed through No. 4 sieve)	CLEAN SAND (Little or no fines)	 GM	Silty gravel; gravel/sand/silt mixture(s)
		SAND WITH FINES (Appreciable amount of fines)	 GC	Clayey gravel; gravel/sand/clay mixture(s)
		CLEAN SAND (Little or no fines)	 SW	Well-graded sand; gravelly sand; little or no fines
		SAND WITH FINES (Appreciable amount of fines)	 SP	Poorly graded sand; gravelly sand; little or no fines
FINE-GRAINED SOIL (More than 50% of material is smaller than No. 200 sieve size)	SILT AND CLAY (Liquid limit less than 50)	 SM	Silty sand; sand/silt mixture(s)	
		 SC	Clayey sand; sand/clay mixture(s)	
		 ML	Inorganic silt and very fine sand; rock flour; silty or clayey fine sand or clayey silt with slight plasticity	
	SILT AND CLAY (Liquid limit greater than 50)	 CL	Inorganic clay of low to medium plasticity; gravelly clay; sandy clay; silty clay; lean clay	
		 OL	Organic silt; organic, silty clay of low plasticity	
		 MH	Inorganic silt; micaceous or diatomaceous fine sand	
	 CH	Inorganic clay of high plasticity; fat clay		
 OH	Organic clay of medium to high plasticity; organic silt			
 PT	Peat; humus; swamp soil with high organic content			

OTHER MATERIALS	USCS GRAPHIC SYMBOL	USCS LETTER SYMBOL	TYPICAL DESCRIPTIONS
PAVEMENT		AC or PC	Asphalt concrete pavement or Portland cement pavement
ROCK		RK	Rock (See Rock Classification)
WOOD		WD	Wood, lumber, wood chips
DEBRIS		DB	Construction debris, garbage

- Notes:
- USCS letter symbols correspond to symbols used by the Unified Soil Classification System and ASTM classification methods. Dual letter symbols (e.g., SP-SM for sand or gravel) indicate soil with an estimated 5-15% fines. Multiple letter symbols (e.g., ML/CL) indicate borderline or multiple soil classifications.
 - Soil descriptions are based on the general approach presented in the Standard Practice for Description and Identification of Soils (Visual-Manual Procedure), outlined in ASTM D 2488. Where laboratory index testing has been conducted, soil classifications are based on the Standard Test Method for Classification of Soils for Engineering Purposes, as outlined in ASTM D 2487.
 - Soil description terminology is based on visual estimates (in the absence of laboratory test data) of the percentages of each soil type and is defined as follows:
 - Primary Constituent: > 50% - "GRAVEL," "SAND," "SILT," "CLAY," etc.
 - Secondary Constituents: > 30% and ≤ 50% - "very gravelly," "very sandy," "very silty," etc.
 - > 15% and ≤ 30% - "gravelly," "sandy," "silty," etc.
 - Additional Constituents: > 5% and ≤ 15% - "with gravel," "with sand," "with silt," etc.
 - ≤ 5% - "with trace gravel," "with trace sand," "with trace silt," etc., or not noted.
 - Soil density or consistency descriptions are based on judgement using a combination of sampler penetration blow counts, drilling or excavating conditions, field tests, and laboratory tests, as appropriate.

Drilling and Sampling Key		Field and Lab Test Data	
SAMPLER TYPE	SAMPLE NUMBER & INTERVAL	Code	Description
Code	Description	Code	Description
a	3.25-inch O.D., 2.42-inch I.D. Split Spoon	PP = 1.0	Pocket Penetrometer, tsf
b	2.00-inch O.D., 1.50-inch I.D. Split Spoon	TV = 0.5	Torvane, tsf
c	Shelby Tube	PID = 100	Photoionization Detector VOC screening, ppm
d	Grab Sample	W = 10	Moisture Content, %
e	Single-Tube Core Barrel	D = 120	Dry Density, pcf
f	Double-Tube Core Barrel	-200 = 60	Material smaller than No. 200 sieve, %
g	2.50-inch O.D., 2.00-inch I.D. WSDOT	GS	Grain Size - See separate figure for data
h	3.00-inch O.D., 2.375-inch I.D. Mod. California	AL	Atterberg Limits - See separate figure for data
i	Other - See text if applicable	GT	Other Geotechnical Testing
1	300-lb Hammer, 30-inch Drop	CA	Chemical Analysis
2	140-lb Hammer, 30-inch Drop		
3	Pushed		
4	Vibrocore (Rotasonic/Geoprobe)		
5	Other - See text if applicable		



Groundwater	
	Approximate water level at time of drilling (ATD)
	Approximate water level at time other than ATD

10/09 \\EDM\DATA\GINT\GINT\PROJECTS\1097001.010.GPJ SOIL CLASS SHEET

TP-1

SAMPLE DATA				SOIL PROFILE			GROUNDWATER
Depth (ft)	Elevation (ft)	Sample Number & Interval	Sampler Type	Test Data	Graphic Symbol	USCS Symbol	
0							Excavation Method: <u>Rubber-tired Backhoe</u> Ground Elevation (ft): _____ Excavated By: <u>None</u> Logged By: <u>MWB</u>
1		S-1				ML	Dark brown (2.5 y 4/3), SILT with trace roots (stiff, moist)
2		S-2				ML	ALLUVIUM Light brown to orange (2.5 y 5/3), fine sandy SILT with trace roots (stiff, moist) -A-horizon ends at 1.5 feet -Redox at 2.5 feet
4						ML	Light brown to orange (2.5 y 5/3), fine sandy SILT (stiff, moist to wet)
6		S-3					
8		S-4				ML	Light brown to orange (2.5 y 4/3), fine sandy SILT (stiff, wet)
Test Pit Completed 06/27/08 Total Depth of Test Pit = 8.5 ft.							

TP-2

SAMPLE DATA				SOIL PROFILE			GROUNDWATER
Depth (ft)	Elevation (ft)	Sample Number & Interval	Sampler Type	Test Data	Graphic Symbol	USCS Symbol	
0							Excavation Method: <u>Rubber-tired Backhoe</u> Ground Elevation (ft): _____ Excavated By: <u>None</u> Logged By: <u>MWB</u>
1		S-1				ML	Dark Brown (2.5 y 4/4), mottled with brownish black, SILT with trace fine sand and trace roots (stiff, moist)
2						ML	ALLUVIUM Brown (2.5 y 4/3), mottled with brownish black, SILT with trace fine sand (stiff, moist)
4		S-2		W = 31 GS		ML	Dark Brown (2.5 y 4/4), mottled with blackish gray, SILT with trace fine sand (stiff, moist)
6							
8		S-3					
Test Pit Completed 06/27/08 Total Depth of Test Pit = 8.0 ft.							

- Notes:
1. Stratigraphic contacts are based on field interpretations and are approximate.
 2. Reference to the text of this report is necessary for a proper understanding of subsurface conditions.
 3. Refer to "Soil Classification System and Key" figure for explanation of graphics and symbols.

1097.01 10/9/09 \MED\MDATA\GINT\GINT\PROJECTS\1097001\010.GPJ TEST PIT LOG



City of Arlington Stormwater
Treatment Wetland
Snohomish County, Washington

Log of Test Pits

Figure
A-2

TP-3

SAMPLE DATA				SOIL PROFILE			GROUNDWATER
Depth (ft) 0 2 4 6 8 10	Elevation (ft)	Sample Number & Interval	Sampler Type	Test Data	Graphic Symbol	USCS Symbol	Excavation Method: <u>Rubber-tired Backhoe</u> Ground Elevation (ft): _____ Excavated By: <u>None</u> Logged By: <u>MWB</u>
		S-1			[Vertical Lines]	ML	Dark brown (2.5 y 4/2), SILT with trace sand and trace roots, (stiff, moist) ALLUVIUM
		S-2			[Diagonal Lines]	ML/CL	Light brown to orange (2.5 y 5/3), silty CLAY (stiff, moist) -Increased clay at 3.5 feet
		S-3			[Diagonal Lines]	ML/CL	Light brown to orange (2.5 y 5/3), silty CLAY (stiff, wet)
	S-4						Groundwater not encountered.
Test Pit Completed 06/27/08 Total Depth of Test Pit = 8.0 ft.							

- Notes:
1. Stratigraphic contacts are based on field interpretations and are approximate.
 2. Reference to the text of this report is necessary for a proper understanding of subsurface conditions.
 3. Refer to "Soil Classification System and Key" figure for explanation of graphics and symbols.

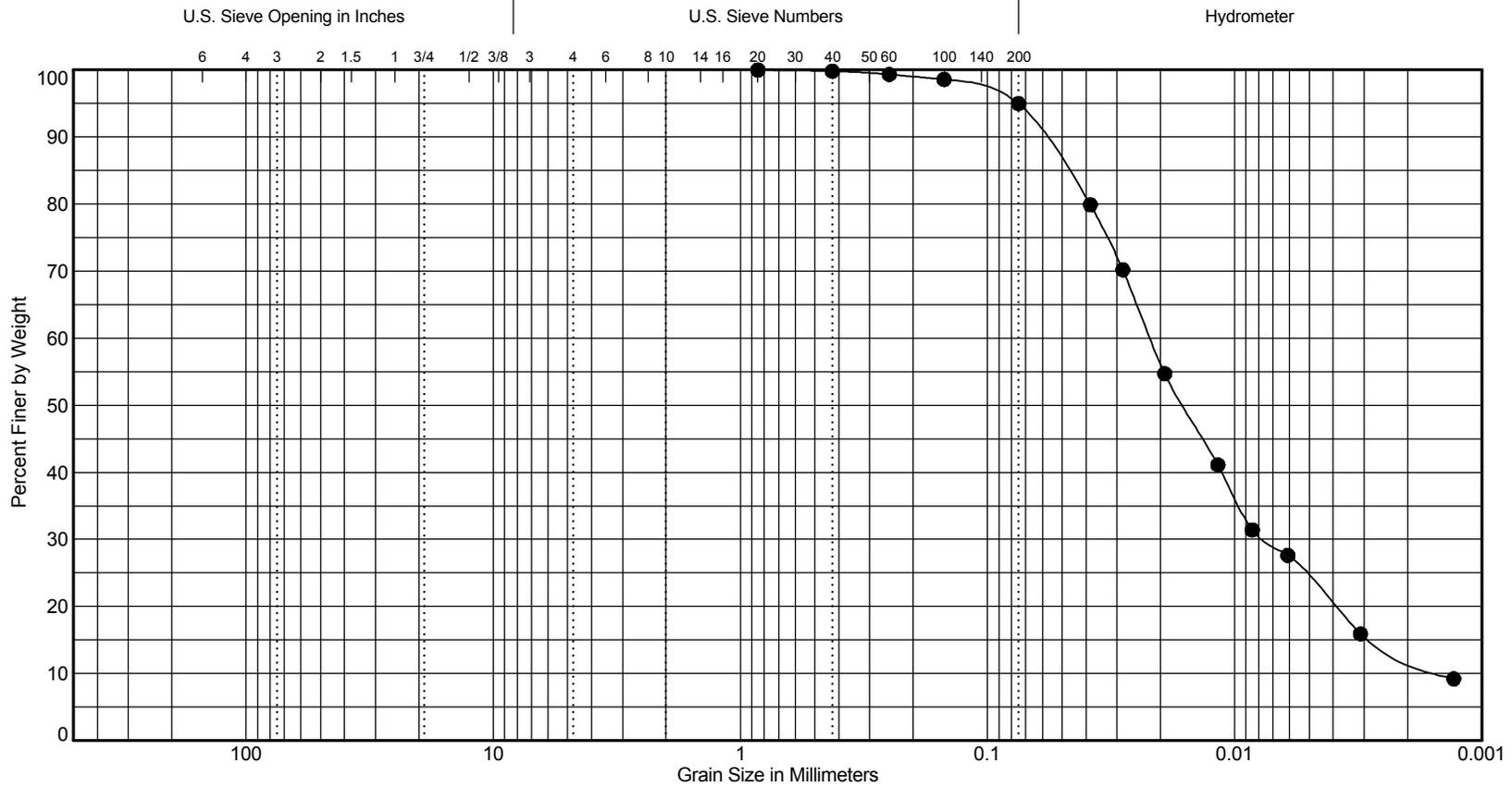
1097.01 10/9/09 \MEDM\DATA\GINT\GINT\PROJECTS\1097001.010.GPJ TEST PIT LOG



City of Arlington Stormwater
Treatment Wetland
Snohomish County, Washington

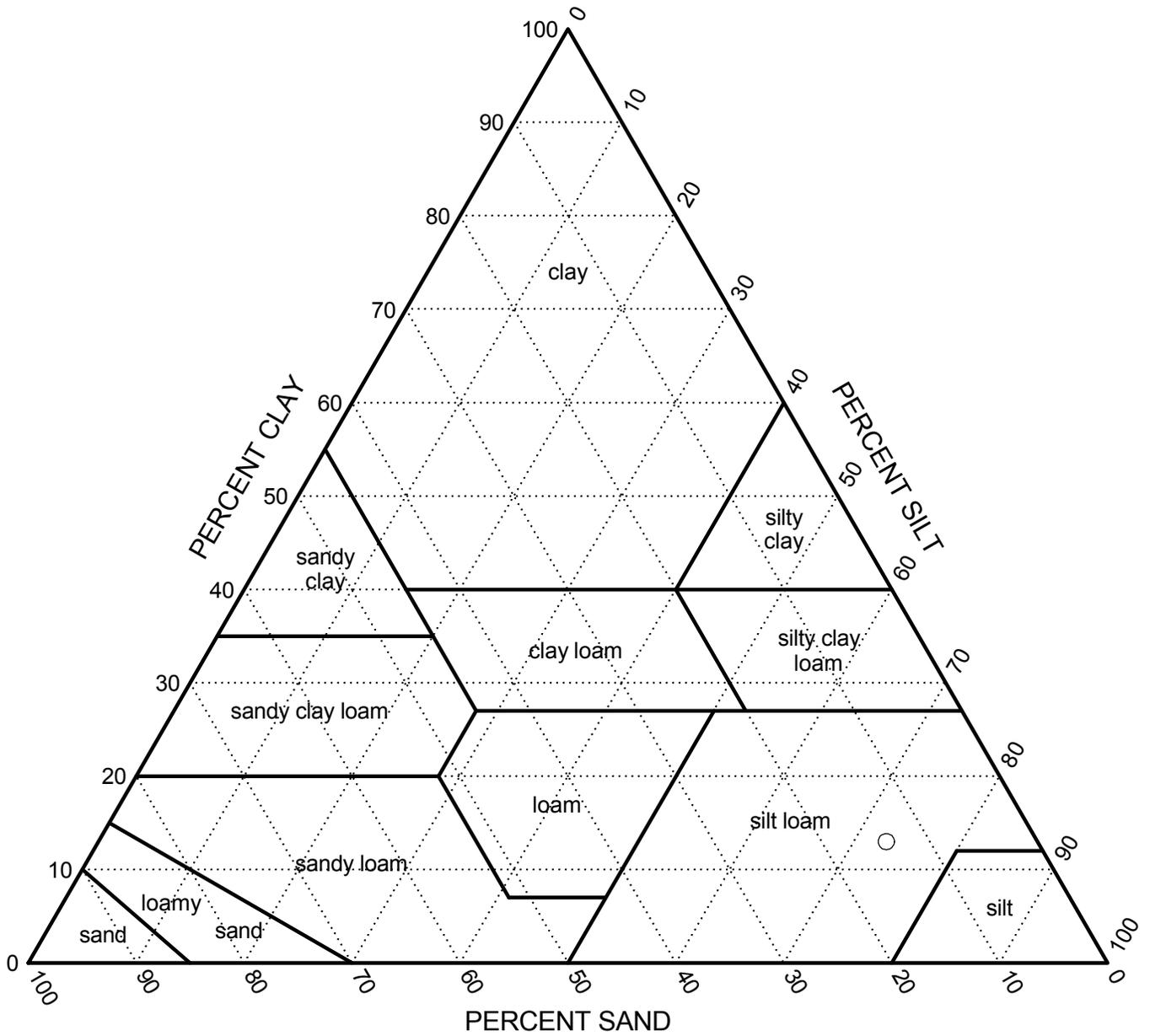
Log of Test Pits

Figure
A-3



Cobbles	Gravel		Sand			Silt or Clay
	Coarse	Fine	Coarse	Medium	Fine	

Symbol	Exploration Number	Sample Number	Depth (ft)	Natural Moisture (%)	Soil Description	Unified Soil Classification
●	TP-2		3.5	31		



	Exploration Number	Sample Number	Depth (ft)	Natural Moisture (%)	USDA Textural Classification	USCS Classification
○	TP-2		3.5	31	SILT LOAM	

APPENDIX A

SOIL DESCRIPTIONS AND PROPERTIES

LOCATION PUGET WA
Established Series
Rev. DES/RJE/TDT
05/2000

PUGET SERIES

The Puget series consists of very deep, poorly drained soils that formed in recent alluvium on floodplains and low river terraces. Slopes are 0 to 3 percent. The average annual precipitation is about 40 inches. The mean annual temperature is about 50 degrees F.

TAXONOMIC CLASS: Fine-silty, mixed, superactive, nonacid, mesic Fluvaquentic Endoaquepts

TYPICAL PEDON: Puget silty clay loam - pasture. (Colors are for moist soil unless otherwise noted.)

A1--0 to 1 inches; very dark grayish brown (2.5Y 3/2) silt loam, moderate thin platy structure; hard, firm, slightly sticky and slightly plastic; many roots; moderately acid (pH 6.0); abrupt smooth boundary. (1 to 7 inches thick)

A2--1 to 7 inches; dark grayish brown (2.5Y 4/2) silty clay loam, light gray (2.5Y 7/2) dry; common fine prominent dark brown (7.5YR 4/4) redox concentrations; moderate very coarse prismatic structure; hard, firm, moderately sticky and moderately plastic; many roots; moderately acid (pH 6.0); clear smooth boundary. (0 to 7 inches thick)

Bg1--7 to 17 inches; dark grayish brown (2.5Y 5/2) silty clay loam, light olive gray (2.5Y 7/2) dry; common medium prominent strong brown (7.5YR 5/6, 5/8) redox concentrations; moderate medium prismatic structure; hard, firm, moderately sticky and moderately plastic; many roots; slightly acid (pH 6.2); clear smooth boundary. (5 to 12 inches thick)

Bg2--17 to 25 inches; grayish brown (2.5Y 5/2) silty clay loam, light olive gray (5Y 6/2) dry; many medium prominent yellowish red (5YR 5/8, 4/8) redox concentrations; strong very coarse prismatic structure; very hard, firm, moderately sticky and moderately plastic; common roots; slightly acid (pH 6.4); abrupt smooth boundary. (0 to 12 inches thick)

Bg3--25 to 31 inches; grayish brown (2.5Y 5/2) silty clay loam, light gray (5Y 7/2) dry; many medium prominent dark yellowish brown (10YR 3/6) and yellowish red (5YR 5/8, 4/6) redox concentrations; moderate medium angular blocky structure; hard, firm, moderately sticky and moderately plastic; few roots; moderately acid (pH 6.0); abrupt wavy boundary. (0 to 6 inches thick)

Bg4--31 to 40 inches; grayish brown (2.5Y 5/2) silty clay loam, light gray (5Y 7/1) dry; common fine prominent strong brown (7.5YR 5/6) and yellowish red (5YR 4/8) redox concentrations;

strong very coarse prismatic structure; hard, firm, moderately sticky and moderately plastic; few roots; moderately acid (pH 5.8); clear smooth boundary. (0 to 10 inches thick)

Cg1--40 to 45 inches; greenish gray (5GY 5/1) silty clay loam, light gray (5Y 7/1) and white (5Y 8/1) dry; common fine prominent strong brown (7.5YR 5/6) and brown (7.5YR 4/4) redox concentrations; massive; hard, firm, moderately sticky and moderately plastic; moderately acid (pH 5.8); clear smooth boundary. (0 to 6 inches thick)

Cg2--45 to 60 inches; gray (5Y 5/1) silty clay, light gray (5Y 7/1) dry; few fine prominent yellowish red (5YR 4/8, 5/8) and common medium distinct light olive brown (2.5Y 5/4) redox concentrations; massive; very hard, firm, moderately sticky and moderately plastic, moderately acid (pH 6.0).

TYPE LOCATION: King County, Washington; 1/2 mile southwest of Carnation; 2,640 feet north and 600 feet east of the southwest corner of sec. 21, T. 25 N., R. 7 E.

RANGE IN CHARACTERISTICS: The mean annual soil temperature is 47 to 52 degrees F. The particle-size control section lacks coarse fragments, has less than 15 percent fine and coarser sand, and 18 to 35 percent clay.

The A horizon has hue of 10YR or 2.5Y, value of 3 through 5 moist, 4 through 7 dry and chroma of 1 or 2 moist and dry. Subhorizons with a value of 3 moist are less than 7 inches thick. It is moderately acid to neutral.

The Bg horizon has hue of 10YR or 2.5Y, value of 4 or 5 moist, 6 or 7 dry and chroma of 2 moist, 1 or 2 dry. It is silt loam or silty clay loam. Some pedons have thin strata (less than 2 inches thick) or sand or loamy sand. This horizon is slightly acid to very strongly acid.

The Cg horizon has hue of 10YR, 2.5Y, 5Y, or 5GY, value of 4 or 5 moist, 6 or 7 dry and chroma of 0 to 2 moist and dry. It is silt loam, silty clay loam or silty clay and has strata of sand or loamy sand. It is slightly acid to very strongly acid.

COMPETING SERIES: This is the [Aetna](#) series. Aetna soils have a buried mollic epipedon that is 10 to 24 inches thick.

GEOGRAPHIC SETTING: The Puget soils are on flood plains at elevations ranging from 10 to 650 feet. They formed in recent alluvium. Slopes are 0 to 3 percent. The soils are in a mild marine climate. Average annual precipitation ranges from 35 to 55 inches, most of which falls as rain during the winter. Snow is infrequent. The average January temperature is about 38 degrees F.; average July temperature is about 64 degrees F.; and mean annual temperature is about 50 degrees F. The frost-free season is about 150 to 190 days.

GEOGRAPHICALLY ASSOCIATED SOILS: These are the [Briscot](#), [Nooksack](#), [Oridia](#), [Puyallup](#), and [Sultan](#) soils. Briscot soils are coarse-loamy. Nooksack, Puyallup, and Sultan soils have a xeric moisture regime. In addition, Nooksack and Puyallup soils have a mollic epipedon. Oridia soils are coarse-silty.

DRAINAGE AND PERMEABILITY: Poorly drained; slow to ponded runoff; moderately slow permeability. These soils are subject to occasional periods of flooding from December to March. An apparent water table is as high as 1 foot over the surface to 1 foot below the surface at times from November to April unless the soil is drained.

USE AND VEGETATION: Most of the Puget soil has been cleared and drained for use as cropland. Seeded grass pasture, grass-legume hay, oats, and green-chop are the major crops. The natural vegetation is red alder, black cottonwood, western redcedar, and willow with an understory of trailing blackberry, salmonberry, Oregon-grape, western swordfern, Indian plum, hardhack, willow, and rush.

DISTRIBUTION AND EXTENT: The Puget Sound Basin of Western Washington; MLRA 2. The series is of moderate extent.

MLRA OFFICE RESPONSIBLE: Portland, Oregon

SERIES ESTABLISHED: Eastern part of Puget Sound Basin, Reconnaissance, 1909.

REMARKS: Laboratory data are available on this series NSSL #S74WA61-7- 341-345. Diagnostic horizons and features recognized in this pedon are an ochric epipedon and a cambic horizon from 17 to 40 inches. The sediments have more than 0.2 percent organic carbon to a depth of 60 inches or more.

National Cooperative Soil Survey

U.S.A.

**Snohomish County Area, Washington Version date: 12/12/2006
10:53:45 PM**

55—Puget silty clay loam (east portion of site)

Map Unit Setting

- *Elevation:* 10 to 650 feet
- *Mean annual precipitation:* 35 to 55 inches
- *Mean annual air temperature:* 48 to 50 degrees F
- *Frost-free period:* 160 to 180 days

Map Unit Composition

- *Puget, drained, and similar soils:* 85 percent
- *Minor components:* 6 percent

Description of Puget, Drained

Setting

- *Landform:* Flood plains
- *Parent material:* Alluvium

Properties and qualities

- *Slope:* 0 to 2 percent
- *Depth to restrictive feature:* More than 80 inches
- *Drainage class:* Poorly drained
- *Capacity of the most limiting layer to transmit water (Ksat):* Moderately low to moderately high (0.06 to 0.20 in/hr)
- *Depth to water table:* About 24 to 48 inches
- *Frequency of flooding:* Rare
- *Frequency of ponding:* None
- *Available water capacity:* High (about 12.0 inches)

Interpretive groups

- *Land capability classification (irrigated):* 3w
- *Land capability (nonirrigated):* 3w

Typical profile

- *0 to 9 inches:* Silty clay loam
- *9 to 38 inches:* Silty clay loam
- *38 to 60 inches:* Silty clay loam

Minor Components

Snohomish

- *Percent of map unit:* 3 percent
- *Landform:* Flood plains

Sumas

- *Percent of map unit:* 3 percent
- *Landform:* Flood plains

LOCATION PUYALLUP WA
Established Series
Rev. RJE
10/2002

PUYALLUP SERIES

The Puyallup series consists of deep, well drained soils formed in recent alluvium. Puyallup soils are on floodplains and low terraces. Slopes are 0 to 3 percent. The mean annual temperature is about 50 F. The average annual precipitation is about 45 inches.

TAXONOMIC CLASS: Coarse-loamy over sandy or sandy-skeletal, isotic over mixed, mesic Vitrandic Haploxerolls

TYPICAL PEDON: Puyallup fine sandy loam - cultivated. (Colors are for moist soil unless otherwise stated.)

Ap--0 to 4 inches; very dark brown (10YR 2/2) fine sandy loam, dark grayish brown (10YR 4/2) dry; moderate medium and coarse granular structure; slightly hard, friable, slightly sticky and slightly plastic; many fine and medium roots; slightly acid (pH 6.2); abrupt smooth boundary. (3 to 8 inches thick)

A2--4 to 8 inches; very dark grayish brown (10YR 3/2) loam, dark grayish brown (10YR 4/2) dry; moderate medium subangular blocky structure; slightly hard, friable, slightly sticky and slightly plastic; many fine and medium roots; moderately acid (pH 6.0); clear smooth boundary. (3 to 5 inches thick)

A3--8 to 18 inches; dark brown (10YR 3/3) fine sandy loam, grayish brown (10YR 5/2) dry; weak medium subangular blocky structure; soft, very friable, nonsticky and nonplastic; common fine roots; many fine and medium and few large pores; slightly acid (pH 6.3); abrupt wavy boundary. (8 to 20 inches thick)

2Cl--18 to 27 inches; dark brown (10YR 3/3) loamy sand, grayish brown (10YR 5/2) dry; single grained; loose; few fine roots; slightly acid (pH 6.4); abrupt wavy boundary. (8 to 15 inches thick)

2C2--27 to 60 inches; very dark grayish brown (2.5Y 3/2) gravelly sand, grayish brown (2.5Y 5/2) dry; single grained; loose; 20 percent pebbles; neutral (pH 6.8).

TYPE LOCATION: Clark County, Washington; along Lower East Fork Lewis River Road about 2,240 feet north and 600 feet west of the southeast corner of sec. 19, T. 4 N., R. 2 E.

RANGE IN CHARACTERISTICS: Depth to the 2C horizon is 15 to 30 inches. The mollic epipedon is 10 to 20 inches thick. Estimated mean annual soil temperature at a depth of 20 inches ranges from 47 to 53 degrees F. These soils are usually moist, but they are dry in all parts between depths of 7 and 20 inches for 60 to 75 consecutive days. The upper part of the particle-size control section has 5 to 15 percent clay and includes fine sandy loam or loam. The lower part has 0 to 5 percent clay. The soil ranges from neutral to moderately acid. The A horizon has hue

of 10YR or 2.5Y, value of 2 or 3 moist, 3 to 5 dry, and chroma of 1 through 3 moist and dry. Below 10 inches it is loam or fine sandy loam. Some pedons have a B or C horizon.

The 2C horizon has hue of 10YR or 2.5Y, value of 3 through 5 moist, 4 through 7 dry, and chroma of 1 through 4 moist and dry. It is sand, loamy sand, or gravelly sand and has 0 to 20 percent pebbles.

COMPETING SERIES: These are the [Boesel](#), [Chamokane](#), [Newberg](#), and [Snakelum](#) series in other families. Boesel soils are frigid. Chamokane soils have a mollic epipedon more than 20 inches thick. Newberg soils lack contrasting textures within the 10- to 40-inch control section. Snakelum soils lack a mollic epipedon and have a regular decrease in organic matter content as depth increases.

GEOGRAPHIC SETTING: These soils are on floodplains and low stream terraces at elevations of 20 to 650 feet. The soils formed in mixed recent alluvium. The climate is humid; summers are cool and dry, and winters are mild and moist. Average annual precipitation is 35 to 60 inches. Mean January temperature is 37 degrees F, mean July temperature is 62 degrees F, mean annual temperature is about 50 degrees F, and frost-free season is 170 to 200 days.

GEOGRAPHICALLY ASSOCIATED SOILS: These are the [Briscot](#), [Mt. Vernon](#), [Oridia](#), [Pilchuck](#), [Puget](#), and [Sultan](#) soils. Briscot, Oridia, and Puget soils have an aquic moisture regime. Pilchuck soils are sandy throughout. Mt. Vernon soils are medial over loamy. Sultan soils are fine-silty and have an ochric epipedon.

DRAINAGE AND PERMEABILITY: Well drained; very slow runoff; moderately rapid permeability in the upper part of the control section and rapid below. These soils are subject to occasional brief periods of flooding from November to April unless protected.

USE AND VEGETATION: Used for cropland. Principal crops are hay, pasture, and row crops. Native vegetation is Douglas-fir, western redcedar, bigleaf maple, black Cottonwood, western hemlock, and red alder, with an understory of trailing blackberry, salmonberry, Oregon-grape, western swordfern, vine maple, and western brackenfern.

DISTRIBUTION AND EXTENT: Western Washington and possibly western Oregon. The series is of moderate extent.

MLRA OFFICE RESPONSIBLE: Portland, Oregon

SERIES ESTABLISHED: Snohomish County, Washington, 1936.

REMARKS: Classification only updated 3/94 because of recent amendments to Soil Taxonomy. Diagnostic horizons and features recognized in this pedon are a mollic epipedon from the surface to 18 inches and a textural change to sandy material at 18 inches. Organic carbon content is assumed to decrease irregularly with depth.

National Cooperative Soil Survey

U.S.A.

56—Puyallup fine sandy loam (west portion of site)

Map Unit Setting

- *Mean annual precipitation:* 35 to 60 inches
- *Mean annual air temperature:* 50 degrees F
- *Frost-free period:* 170 to 200 days

Map Unit Composition

- *Puyallup and similar soils:* 85 percent
- *Minor components:* 6 percent

Description of Puyallup

Setting

- *Landform:* Terraces
- *Parent material:* Alluvium

Properties and qualities

- *Slope:* 0 to 3 percent
- *Depth to restrictive feature:* More than 80 inches
- *Drainage class:* Well drained
- *Capacity of the most limiting layer to transmit water (Ksat):* High (1.98 to 5.95 in/hr)
- *Depth to water table:* More than 80 inches
- *Frequency of flooding:* Rare
- *Frequency of ponding:* None
- *Available water capacity:* Moderate (about 6.3 inches)

Interpretive groups

- *Land capability classification (irrigated):* 2w
- *Land capability (nonirrigated):* 2s

Typical profile

- *0 to 10 inches:* Fine sandy loam
- *10 to 30 inches:* Fine sandy loam
- *30 to 60 inches:* Sand

Minor Components

Puget

- *Percent of map unit:* 3 percent
- *Landform:* Flood plains

Sumas

- *Percent of map unit:* 3 percent
- *Landform:* Flood plains

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Component name (percent)	Rating reasons (rating values)	Acres in AOI	Percent of AOI
55	Puget silty clay loam	Somewhat limited	Puget, Drained (85%)	Depth to saturated zone (0.95) Cutbanks cave (0.10)	5.5	30.9%
56	Puyallup fine sandy loam	Very limited	Puyallup (85%)	Cutbanks cave (1.00)	12.3	69.1%
Totals for Area of Interest (AOI)					17.8	100.0%



Tables — Shallow Excavations — Summary by Rating Value



Summary by Rating Value

Rating	Acres in AOI	Percent of AOI
Very limited	12.3	69.1%
Somewhat limited	5.5	30.9%
Totals for Area of Interest (AOI)	17.8	100.0%



Description — Shallow Excavations

Shallow excavations are trenches or holes dug to a maximum depth of 5 or 6 feet for graves, utility lines, open ditches, or other purposes. The ratings are based on the soil properties that influence the ease of digging and the resistance to sloughing. Depth to bedrock or a cemented pan, hardness of bedrock or a cemented pan, the amount of large stones, and dense layers influence the ease of digging, filling, and compacting. Depth to the seasonal high water table, flooding, and ponding may restrict the period when excavations can be made. Slope influences the ease of using machinery. Soil texture, depth to the water table, and linear extensibility (shrink-swell potential) influence the resistance to sloughing.

The ratings are both verbal and numerical. Rating class terms indicate the extent to which the soils are limited by all of the soil features that affect the specified use. "Not limited" indicates that the soil has features that are very favorable for the specified use. Good performance and very low maintenance can be expected. "Somewhat limited" indicates that the soil has features that are moderately favorable for the specified use. The limitations can be overcome or minimized by special planning, design, or installation. Fair performance and moderate maintenance can be expected. "Very limited" indicates that the soil has one or more features that are unfavorable for the specified use. The limitations generally cannot be overcome without major soil reclamation, special design, or expensive installation procedures. Poor performance and high maintenance can be expected.

Numerical ratings indicate the severity of individual limitations. The ratings are shown as decimal fractions ranging from 0.01 to 1.00. They indicate gradations between the point at which a soil feature has the greatest negative impact on the use (1.00) and the point at which the soil feature is not a limitation (0.00).

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	All Hydric	5.5	30.9%
56	Puyallup fine sandy loam	Partially Hydric	12.3	69.1%
Totals for Area of Interest (AOI)			17.8	100.0%



Description — Hydric Rating by Map Unit

This rating provides an indication of the proportion of the map unit that meets the criteria for hydric soils. Map units that are dominantly made up of hydric soils may have small areas, or inclusions, of nonhydric soils in the higher positions on the landform, and map units dominantly made up of nonhydric soils may have inclusions of hydric soils in the lower positions on the landform.

Hydric soils are defined by the National Technical Committee for Hydric Soils (NTCHS) as soils that formed under conditions of saturation, flooding, or ponding long enough during the growing season to develop anaerobic conditions in the upper part (Federal Register, 1994). Under natural conditions, these soils are either saturated or inundated long enough during the growing season to support the growth and reproduction of hydrophytic vegetation.

The NTCHS definition identifies general soil properties that are associated with wetness. In order to determine whether a specific soil is a hydric soil or nonhydric soil, however, more specific information, such as information about the depth and duration of the water table, is needed. Thus, criteria that identify those estimated soil properties unique to hydric soils have been established (Federal Register, 2002). These criteria are used to identify map unit components that normally are associated with wetlands. The criteria used are selected estimated soil properties that are described in "Soil Taxonomy" (Soil Survey Staff, 1999) and "Keys to Soil Taxonomy" (Soil Survey Staff, 2006) and in the "Soil Survey Manual" (Soil Survey Division Staff, 1993).

If soils are wet enough for a long enough period of time to be considered hydric, they should exhibit certain properties that can be easily observed in the field. These visible properties are indicators of hydric soils. The indicators used to make onsite determinations of hydric soils are specified in "Field Indicators of Hydric Soils in the United States" (Hurt and Vasilas, 2006).

References:

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- Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.
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- Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation

Service. U.S. Department of Agriculture Handbook 436.
 Soil Survey Staff. 2006. Keys to soil taxonomy. 10th edition. U.S. Department of
 Agriculture, Natural Resources Conservation Service.

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	Prime farmland if drained and either protected from flooding or not frequently flooded during the growing season	5.5	30.9%
56	Puyallup fine sandy loam	All areas are prime farmland	12.3	69.1%
Totals for Area of Interest (AOI)			17.8	100.0%



Description — Farmland Classification

Farmland classification identifies map units as prime farmland, farmland of statewide importance, farmland of local importance, or unique farmland. It identifies the location and extent of the soils that are best suited to food, feed, fiber, forage, and oilseed crops. NRCS policy and procedures on prime and unique farmlands are published in the "Federal Register," Vol. 43, No. 21, January 31, 1978.

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	3	5.5	30.9%
56	Puyallup fine sandy loam	2	12.3	69.1%
Totals for Area of Interest (AOI)			17.8	100.0%



Description — Nonirrigated Capability Class

Land capability classification shows, in a general way, the suitability of soils for most kinds of field crops. Crops that require special management are excluded. The soils are grouped according to their limitations for field crops, the risk of damage if they are used for crops, and the way they respond to management. The criteria used in grouping the soils do not include major and generally expensive landforming that would change slope, depth, or other characteristics of the soils, nor do they include possible but unlikely major reclamation projects. Capability classification is not a substitute for interpretations that show suitability and limitations of groups of soils for rangeland, for woodland, or for engineering purposes.

In the capability system, soils are generally grouped at three levels-capability class, subclass, and unit. Only class and subclass are included in this data set.

Capability classes, the broadest groups, are designated by the numbers 1 through 8. The numbers indicate progressively greater limitations and narrower choices for

practical use. The classes are defined as follows:

Class 1 soils have few limitations that restrict their use.

Class 2 soils have moderate limitations that reduce the choice of plants or that require moderate conservation practices.

Class 3 soils have severe limitations that reduce the choice of plants or that require special conservation practices, or both.

Class 8 soils and miscellaneous areas have limitations that preclude commercial plant production and that restrict their use to recreational purposes, wildlife habitat, watershed, or esthetic purposes.

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating (centimeters)	Acres in AOI	Percent of AOI
55	Puget silty clay loam	92	5.5	30.9%
56	Puyallup fine sandy loam	>200	12.3	69.1%
Totals for Area of Interest (AOI) 17.8100.0%				



Description — Depth to Water Table

"Water table" refers to a saturated zone in the soil. It occurs during specified months. Estimates of the upper limit are based mainly on observations of the water table at selected sites and on evidence of a saturated zone, namely grayish colors (redoximorphic features) in the soil. A saturated zone that lasts for less than a month is not considered a water table.

This attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.



Rating Options — Depth to Water Table

Units of Measure: centimeters

Aggregation Method: Dominant Component

Component Percent Cutoff: *None Specified*

Tie-break Rule: Lower

Interpret Nulls as Zero: No

Beginning Month: January

Ending Month: December

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	Rare	5.5	30.9%
56	Puyallup fine sandy loam	Rare	12.3	69.1%
Totals for Area of Interest (AOI) 17.8100.0%				



Description — Flooding Frequency Class

Flooding is the temporary inundation of an area caused by overflowing streams, by runoff from adjacent slopes, or by tides. Water standing for short periods after rainfall or snowmelt is not considered flooding, and water standing in swamps and marshes is considered ponding rather than flooding.

Frequency is expressed as none, very rare, rare, occasional, frequent, and very frequent.

"None" means that flooding is not probable. The chance of flooding is nearly 0 percent in any year. Flooding occurs less than once in 500 years.

"Very rare" means that flooding is very unlikely but possible under extremely unusual weather conditions. The chance of flooding is less than 1 percent in any year.

"Rare" means that flooding is unlikely but possible under unusual weather conditions. The chance of flooding is 1 to 5 percent in any year.



Rating Options — Flooding Frequency Class

Aggregation Method: Dominant Condition

Component Percent Cutoff: *None Specified*

Tie-break Rule: More Frequent

Beginning Month: January

Ending Month: December

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	None	5.5	30.9%
56	Puyallup fine sandy loam	None	12.3	69.1%
Totals for Area of Interest (AOI) 17.8100.0%				



Description — Ponding Frequency Class

Ponding is standing water in a closed depression. The water is removed only by deep percolation, transpiration, or evaporation or by a combination of these processes. Ponding frequency classes are based on the number of times that ponding occurs over a given period. Frequency is expressed as none, rare,

occasional, and frequent.

"None" means that ponding is not probable. The chance of ponding is nearly 0 percent in any year.

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	C	5.5	30.9%
56	Puyallup fine sandy loam	B	12.3	69.1%
Totals for Area of Interest (AOI)		17.8	100.0%	



Description — Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	CL	5.5	30.9%
56	Puyallup fine sandy loam	SM	12.3	69.1%
Totals for Area of Interest (AOI)		17.8	100.0%	



Description — Unified Soil Classification (Surface)

The Unified soil classification system classifies mineral and organic mineral soils for engineering purposes on the basis of particle-size characteristics, liquid limit, and plasticity index. It identifies three major soil divisions: (i) coarse-grained soils having less than 50 percent, by weight, particles smaller than 0.074 mm in diameter; (ii) fine-grained soils having 50 percent or more, by weight, particles smaller than 0.074 mm in diameter; and (iii) highly organic soils that demonstrate certain organic characteristics. These divisions are further subdivided into a total of 15 basic soil groups. The major soil divisions and basic soil groups are determined on the basis of estimated or measured values for grain-size distribution and

Atterberg limits. ASTM D 2487 shows the criteria chart used for classifying soil in the Unified system and the 15 basic soil groups of the system and the plasticity chart for the Unified system.

The various groupings of this classification correlate in a general way with the engineering behavior of soils. This correlation provides a useful first step in any field or laboratory investigation for engineering purposes. It can serve to make some general interpretations relating to probable performance of the soil for engineering uses.

For each soil horizon in the database one or more Unified soil classifications may be listed. One is marked as the representative or most commonly occurring. The representative classification is shown here for the surface layer of the soil.

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating (centimeters per centimeter)	Acres in AOI	Percent of AOI
55	Puget silty clay loam	0.20	5.5	30.9%
56	Puyallup fine sandy loam	0.10	12.3	69.1%
Totals for Area of Interest (AOI)		17.81	100.0%	



Description — Available Water Capacity

Available water capacity (AWC) refers to the quantity of water that the soil is capable of storing for use by plants. The capacity for water storage is given in centimeters of water per centimeter of soil for each soil layer. The capacity varies, depending on soil properties that affect retention of water. The most important properties are the content of organic matter, soil texture, bulk density, and soil structure, with corrections for salinity and rock fragments. Available water capacity is an important factor in the choice of plants or crops to be grown and in the design and management of irrigation systems. It is not an estimate of the quantity of water actually available to plants at any given time.

Available water supply (AWS) is computed as AWC times the thickness of the soil. For example, if AWC is 0.15 cm/cm, the available water supply for 25 centimeters of soil would be 0.15 x 25, or 3.75 centimeters of water.

For each soil layer, AWC is recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
55	Puget silty clay loam	6.3	5.5	30.9%

Summary by Map Unit — Snohomish County Area, Washington

56	Puyallup fine sandy loam	6.5	12.3	69.1%
Totals for Area of Interest (AOI) 17.8100.0%				



Description — pH (1 to 1 Water)

Soil reaction is a measure of acidity or alkalinity. It is important in selecting crops and other plants, in evaluating soil amendments for fertility and stabilization, and in determining the risk of corrosion. In general, soils that are either highly alkaline or highly acid are likely to be very corrosive to steel. The most common soil laboratory measurement of pH is the 1:1 water method. A crushed soil sample is mixed with an equal amount of water, and a measurement is made of the suspension.

For each soil layer, this attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

0-36 inches

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating (percent)	Acres in AOI	Percent of AOI
55	Puget silty clay loam	0	5.5	30.9%
56	Puyallup fine sandy loam	0	12.3	69.1%
Totals for Area of Interest (AOI) 17.8100.0%				



Description — Calcium Carbonate (CaCO₃)

Calcium carbonate equivalent is the percent of carbonates, by weight, in the fraction of the soil less than 2 millimeters in size. The availability of plant nutrients is influenced by the amount of carbonates in the soil. Incorporating nitrogen fertilizer into calcareous soils helps to prevent nitrite accumulation and ammonium-N volatilization.

For each soil layer, this attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

0-36 inches

Summary by Map Unit — Snohomish County Area, Washington

Map unit symbol	Map unit name	Rating (milliequivalents per 100 grams)	Acres in AOI	Percent of AOI
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Summary by Map Unit — Snohomish County Area, Washington

55	Puget silty clay loam	23.0	5.5	30.9%
56	Puyallup fine sandy loam	33.0	12.3	69.1%
Totals for Area of Interest (AOI) 17.8100.0%				



Description — Cation-Exchange Capacity (CEC-7)

Cation-exchange capacity (CEC-7) is the total amount of extractable bases that can be held by the soil, expressed in terms of milliequivalents per 100 grams of soil at neutrality (pH 7.0) or at some other stated pH value. Soils having a low cation-exchange capacity hold fewer cations and may require more frequent applications of fertilizer than soils having a high cation-exchange capacity. The ability to retain cations reduces the hazard of ground-water pollution.

For each soil layer, this attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

0-36 inches

Snohomish County Area, Washington

Map symbol and soil name	Pct. of map unit	Awm - overland flow process treatment of wastewater		Awm - rapid infiltration disposal of wastewater		Awm - slow rate process treatment of wastewater	
		Rating class and limiting features	Value	Rating class and limiting features	Value	Rating class and limiting features	Value
55—Puget silty clay loam							
Puget, drained	85	Very limited		Very limited		Somewhat limited	
		Seepage	1.00	Slow water movement	1.00	Slow water movement	0.96
		Depth to saturated zone	0.43	Depth to saturated zone	1.00	Depth to saturated zone	0.43
		Flooding	0.40				
Snohomish	3	Not rated		Not rated		Not rated	
Sumas	3	Not rated		Not rated		Not rated	
56—Puyallup fine sandy loam							
Puyallup	85	Very limited		Somewhat limited		Very limited	
		Seepage	1.00	Slow water movement	0.32	Filtering capacity	0.99
		Flooding	0.40				
Puget	3	Not rated		Not rated		Not rated	
Sumas	3	Not rated		Not rated		Not rated	

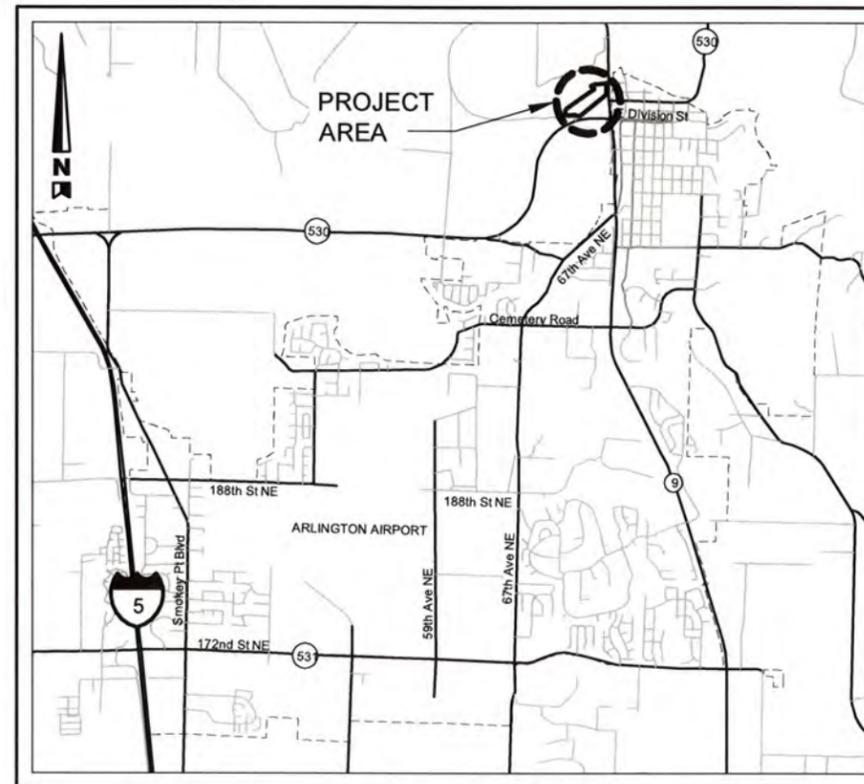
Construction Drawings (Landau Associates 2009)

STORMWATER WETLAND PROJECT

201 WEST COX

PROJECT FILE NO:

<u>SHEET INDEX</u>	
SHEET NAME	SHEET #
COVER SHEET	1
HORIZONTAL CONTROL PLAN	2
TEMPORARY EROSION / SEDIMENT CONTROL PLAN	3
GENERAL NOTES & T.E.S.C. DETAILS	4
GRADING AND SHORELINE / FLOOD HAZARD PLAN	5
GRADING SECTIONS & PROFILES	6
GRADING DETAILS & SECTIONS	7
WEIR PLANS, SECTIONS & DETAILS	8
PIPING PLAN	9
PIPING DETAILS	10
SOIL AMENDMENT, PUBLIC ACCESS & SITE PROTECTION PLAN	11
PLANTING PLAN	12
PLANTING LIST & DETAILS	13



VICINITY MAP
N.T.S.



City of Arlington Officials

DIRECTOR OF PUBLIC WORKS
JAMES KELLY, P.E.

MAYOR
MARGARET LARSON

COUNCIL MEMBERS
STEVE BAKER
CHRIS RAEZER
SCOTT SOLLA
SALLY LIEN
MARILYN OERTLE
RICHARD BUTNER
LINDA BYRNES

CORE DESIGN TEAM

ENGINEERING GROUP
 JAMES KELLEY, P.E. (CITY OF ARLINGTON)
 KELLEY WRIGG, P.E. (LANDAU ASSOCIATES, INC.)
 DAVE FISCHER, P.E. (LANDAU ASSOCIATES, INC.)
 WETLAND SCIENTIST GROUP
 BILL BLAKE (CITY OF ARLINGTON)
 MIKE WOLANEK (CITY OF ARLINGTON)
 SACHA MAXWELL (LANDAU ASSOCIATES, INC.)
 PERRY WELCH (LANDAU ASSOCIATES, INC.)
 FINANCING
 KRIS WALLACE (CITY OF ARLINGTON)
 PERMITTING
 BRENDA FECHT (CITY OF ARLINGTON)



APPROVED FOR CONSTRUCTION

PAUL ELLIS

CAPITAL IMPROVEMENT
PROJECT MANAGER

**CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
FULL DRAINAGE AND GRADING PLAN
PORTION OF SW 1/4 OF SEC 2, TWP 31 N, R 5 E, W.M.**



LEGEND

---	PROPERTY BOUNDARY
---	PROPOSED CONTOURS (1 FT)
---	PROPOSED CONTOURS (.5 FT)
---	EXISTING CONTOURS (1 FT)
---	SECTION SUBDIVISION LINE
---	CLEARING AND GRADING LIMITS
---	SILT FENCE
---	HIGH VISIBILITY FENCE
---	EXISTING WIRE FENCE
---	ROAD CENTERLINE
---	PROPOSED PIPE
---	EXISTING PIPE
---	N.G.P.A. / E. BOUNDARY
---	APPROXIMATE LOCATION OF FLOODWAY BOUNDARY (SNOHOMISH COUNTY PERMIT AND ZONING MAP 2009)
---	ORDINARY HIGH WATER MARK
---	WETLAND/STREAM BUFFER
---	EXISTING TRAIL
---	EXISTING GRAVEL ROAD
---	EXISTING PAVED ROAD
---	PROPOSED MOWED GRASS PATH
---	PROPOSED GRAVEL PATH
---	PROPOSED IMPROVED ROADWAY
---	EXISTING WETLAND
---	HISTORIC SITE PROTECTION AREA (NO GRADING)
---	EXISTING BUILDING
---	CROSS SECTION LOCATION

LEGAL DESCRIPTION
SEC 02 TWP 31 RGE 05 RT-22-22B) BEG S1/4 COR TH N88°42'50W 241.8 FT TH N01°06'00W 502.4 FT TH N09°23'00W 942.74 FT TH S89°24'30W 1452.22 FT TO TPB TH N00°22'00E 687.43 FT TH N56°42'00E 590.28 FT TH N71°34'30E 515 FT ML TO ELY MGN VAC MARKET ST EXT NLY PLAT HALLER CITY TH SLY ALG EXT ELY MGN AFORSO MARKET ST TO NLY MGN RIVER- SIDE AVE TH ELY ALG SD NLY MGN RIVER- SIDE AVE TO WLY MGN MINOR ST TH SLY ALG SD WLY MGN MINOR ST TO NLY MGN DIV AVE TH WLY ALG SD NLY MGN DIV AVE TO TPB LESS PTN DD OR DEDICATED TO CO OR ST FOR RD SUBJ ESE PUD LESS SHY & LESS ADDL RW TO STATE PER SCC 97-2-07732-8
RECORDS OF SNOHOMISH COUNTY, WASHINGTON.

TAX PARCEL NUMBER: 31050200300200
SITE ADDRESS: 201 WEST COX
CONTACT PERSONS: DAVE FISCHER, PE
SACHA MAXWELL
LANDAU ASSOCIATES
130 2ND AVE S.
EDMONDS, WA, 98020
PHONE: (425) 778-0907

APPLICANT: CITY OF ARLINGTON
BILL BLAKE
238 NORTH OLYMPIC AVE.
ARLINGTON, WA, 98223
PHONE: (360) 403-3500

SITE INFORMATION
CURRENT ZONING: AG 10 (SNOHOMISH COUNTY)
GENERAL POLICY PLAN: 111 SINGLE FAMILY RESIDENCE - DETACHED
PROPOSED LAND USE: STORMWATER WETLAND CONSTRUCTION
GROSS SQUARE FOOTAGE: 21.90 ACRES
WATER SUPPLY: CITY OF ARLINGTON
SEWAGE DISPOSAL: ON SITE SEPTIC SYSTEM
SCHOOL DISTRICT: CITY OF ARLINGTON
FLOOD INSURANCE RATE MAP: 53061C0384

- SHORELINE PERMIT INFORMATION**
- TOTAL VALUE OF CONSTRUCTION PROJECT IS ESTIMATED TO BE \$900,000.
 - THIS PROJECT LOCATION IS NECESSARY AS IT IS LOCATED IMMEDIATELY DOWNSTREAM OF STORMWATER INPUT SOURCES AND IMMEDIATELY UPSTREAM OF THE EXISTING STORMWATER OUTFALL. THE PROJECT IS IN COMPLIANCE WITH CURRENT SHORELINE MANAGEMENT ACT POLICIES BECAUSE IT PRESERVES AND ENHANCES SHORELINE HABITAT, IMPROVES WATER QUALITY IN WATER OF THE STATE / SHORELINE OF THE STATE RIVER, AND IT INCREASES PUBLIC ACCESS, EDUCATION, AND RECREATION OPPORTUNITIES WITHIN THE SHORELINE ENVIRONMENT.
 - PERMITS FOR THIS PROJECT INCLUDE:

JURISDICTION	PERMIT/CERTIFICATION/APPROVAL
SNOHOMISH COUNTY	SHORELINE PERMIT FLOODPLAIN MANAGEMENT / FLOOD HAZARD CLEARING AND GRADING PERMIT
WASHINGTON STATE DEPARTMENT OF ECOLOGY	WATER QUALITY CERTIFICATION GENERAL CONSTRUCTION STORMWATER NPDES PERMIT
WASHINGTON STATE DEPARTMENT OF ARCHAEOLOGY AND HISTORIC PRESERVATION	FINDING OF 'NO CULTURAL RESOURCE IMPACT'
CITY OF ARLINGTON (LEAD AGENCY)	STATE ENVIRONMENTAL POLICY ACT (SEPA) DNS
WASHINGTON STATE DEPARTMENT OF TRANSPORTATION / CITY OF ARLINGTON	RIGHT OF WAY USE PERMITS

- SOURCES**
- NOT SURVEYED / APPROXIMATE (FROM AS-BUILTS)
 - SNOHOMISH COUNTY PERMITS PLANNING AND ZONING MAP 2009 / FEMA FIRM
 - NORTHWEST ARCHAEOLOGICAL ASSOCIATES, INC. 2006
 - WSDOT 1998

- MISC. NOTES**
- THERE IS AN EXISTING WSDOT BIOSWALE LOCATED WITHIN 15' OF THE PROPERTY IN THE WSDOT RIGHT OF WAY EAST OF THE ROUND HOUSE SITE.
 - THE STORMWATER POLLUTION SOURCE CONTROL B.M.P.'S BEING PROPOSED ON THIS SITE ARE GOOD HOUSEKEEPING.
 - THE PERMANENT WATER QUALITY AND QUANTITY CONTROL FACILITY BEING PROPOSED IS A CREATED WETLAND.
 - THE TEMPORARY B.M.P.'S BEING PROPOSED ON THIS SITE SHALL CONSIST OF THE INSTALLATION OF SILT FENCES, A CONSTRUCTION ENTRANCE AND ONSITE INFILTRATION OF CONSTRUCTION STORMWATER.

**CALL 48 HOURS BEFORE YOU DIG
1-800-424-5555**

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
APPROVED FOR CONSTRUCTION

By: _____ Date: _____
R/W PERMIT NO. _____ PFN _____

SITE CONDITIONS
SOILS: ACCORDING TO THE NRCS SOIL SURVEY OF SNOHOMISH COUNTY THE EASTERN TWO-FIFTHS OF THE ON-SITE SOILS CONSIST OF THE PUGET SILTY CLAY LOAM AND THE WESTERN THREE-FIFTHS ARE PUYALLUP FINE SANDY LOAM. BOTH SOILS CAN BE FOUND ON OUTWASH PLAINS AND THE PERMEABILITY RANGES FROM LOW TO MODERATE. WATER CAPACITY OF THESE SOILS IS LOW TO MODERATELY HIGH. ONSITE INVESTIGATIONS INDICATED THAT THESE SOILS WERE NOT SEPARATED AS SHOWN ON THE SC5 MAP BUT RATHER INTERMIXED THROUGHOUT THE SITE. THE SOILS ARE INTERMIXED VERTICALLY AS WELL AS HORIZONTALLY. IN GENERAL THE PERMEABILITY AND WATER CAPACITY OF THE SOILS IS HIGHER IN THE NORTHERN PORTION OF THE PROPOSED WETLAND AREA THAN IN THE SOUTHERN PORTION.

VEGETATION:
THE MAJORITY OF THE SITE IS DOMINATED BY GRASSES AND FORBS INCLUDING VELVETGRASS, BENTGRASSES, CREEPING BUTTERCUP AND WHITE CLOVER. THE RIPARIAN AREA IS VEGETATED WITH SHRUBS AND TREES INCLUDING DOUGLAS FIR, RED ALDER, WESTERN RED CEDAR, WILLOWS AND HIMALAYAN BLACKBERRY. AFTER CONSTRUCTION IS COMPLETED, THE WETLAND AND BERMS WILL BE RE-VEGETATED WITH A MIX OF WETLAND AND UPLAND PLANTS (SEE THE WETLAND PLANTING PLAN).

DRAINAGE:
CURRENTLY THE RAINFALL THAT LANDS ON THE SITE TENDS TO INFILTRATE INTO THE NATIVE SOILS. DURING FLOODING EVENTS, THE ENTIRE PROPERTY CAN BECOME FLOODED WITH A FEW FEET OF WATER, AND FLOW CAN EXIT THE PROPERTY ACROSS DIKE ROAD TO THE WEST, AND UNDER SR 530 VIA A CULVERT. AFTER FLOOD LEVELS FALL BELOW THE LEVEL OF THE OUTLETS, ALL THE REMAINING FLOOD WATER INFILTRATES INTO THE SOILS.

RISK ASSESSMENT
SLOPE: THE SITE HAS GENTLE SLOPES THAT GENERALLY DRAIN TO A LOW AREA IN THE CENTER OF THE PROPERTY. RAINFALL UPON THE PROPERTY INFILTRATES INTO THE NATIVE SOILS.

SOURCE OF WATER EROSION
RAINFALL AND FLOOD EVENTS

SOILS: RIVER DEPOSITS, MAPPED AS PUYALLUP FINE SANDY LOAM, AND PUGET SILTY CLAY LOAM.

CONCLUSION: SIGNIFICANT EROSION/SILTATION IMPACT IS LOW FOR THE FOLLOWING REASONS: 1. PROPOSED AREAS OF DEVELOPMENT ARE IN GENTLE SLOPE AREAS; 2. THE SOIL PERMEABILITY IS MODERATE; 3. AVAILABLE WATER CAPACITY OF THE SOIL IS MODERATE; 4. WATER EROSION IS SLIGHT (RUNOFF IS SLOW); 5. EROSION CONTROL MEASURES AND GOOD CONSTRUCTION SITE MANAGEMENT IS TO BE FOLLOWED AS NOTED HEREIN.

SIGNIFICANT LANDSCAPING FEATURES AND NATIVE VEGETATION
ALL VEGETATION BEYOND THE LIMITS OF DISTURBANCE SHALL REMAIN.

AFTER GRADING, THE RAINFALL WILL CONTINUE TO INFILTRATE IN AREAS WITHIN AND OUTSIDE OF THE PROPOSED WETLAND. THE WETLAND AREA WILL DRAIN THROUGH PIPING TO THE RIVER AFTER COMPLETION OF WORK.



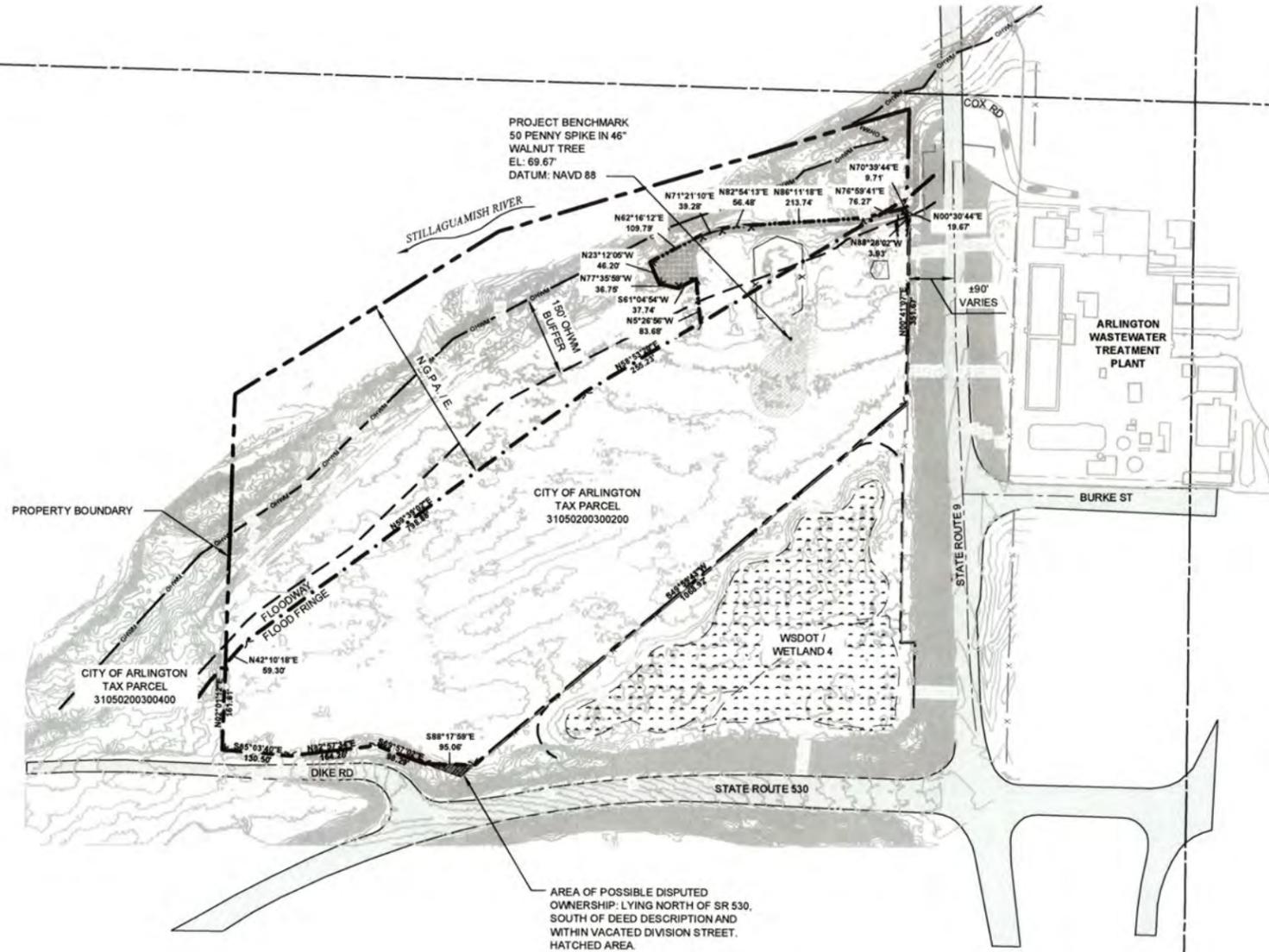
NO.	DATE	REVISIONS	DESIGNED	REVIEWED	APPROVED	STATUS	INITIAL	DATE

DRAFTED BY:	B. TAYLOR	BLT	1-12-2010
DESIGNED BY:	K. WRIGG	KEW	1-12-2010
REVIEWED BY:	D. FISCHER	DAP	1-12-2010
APPROVED BY:			

LANDAU ASSOCIATES
130 2ND AVENUE S.
EDMONDS, WASHINGTON 98020
(425) 778-0907, FAX (425) 778-6409

CITY OF ARLINGTON STORMWATER WETLAND PROJECT SNOHOMISH COUNTY, WASHINGTON	PROJECT NO.	1097001.010.015
	DATE	1-12-2010
	SHEET	1 OF 13
	DRAWING NO.	1

FOUND QUARTER SECTION CORNER
1 1/2" OUTSIDE DIAMETER IRON PIPE
1.2' BELOW SURFACE IN STEEL CASE
VISITED: 8-21-2008
PT. NO.: 176-3-4



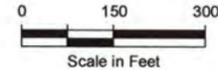
LEGEND

- - - - - PROPERTY BOUNDARY
- - - - - SECTION SUBDIVISION LINE
- - - - - EXISTING WIRE FENCE
- - - - - ROAD CENTERLINE
- - - - - N.G.P.A. / E. BOUNDARY
- - - - - APPROXIMATE LOCATION OF FLOODWAY BOUNDARY (SNOHOMISH COUNTY PERMIT AND ZONING MAP 2009)
- - - - - ORDINARY HIGH WATER MARK
- - - - - WETLAND/STREAM BUFFER
- - - - - EXISTING TRAIL
- - - - - GRAVEL ROAD
- - - - - PAVED ROAD
- - - - - EXISTING WETLAND

NOTE
PROPERTY DIMENSIONS PER TOPOGRAPHIC SURVEY MAP BY HUEY AND ASSOCIATES (2008). COORDINATES ON THIS PLAN SET PROVIDED FOR CONSTRUCTION PURPOSES ONLY, AND ARE NOT INTENDED TO REPRESENT A BOUNDARY SURVEY OF ANY KIND.

SURVEY NOTES
DATUM
DATUM: NAVD 88
BENCH MARK
PROJECT BENCHMARK
50 PENNY SPIKE IN 46" WALNUT TREE
EL: 69.67
DATUM: NAVD 88

TOPOGRAPHIC SURVEY FOR: THE CITY OF ARLINGTON
 IN THE SOUTHWEST 1/4 OF SECTION 2, TOWNSHIP 31 NORTH,
 RANGE 5 EAST, W.M., SNOHOMISH COUNTY, WASHINGTON
 HUEY SURVEYING & LAND CONSULTING, INC.
 18933 59TH AVENUE NORTHEAST, SUITE 115
 ARLINGTON, WASHINGTON 98223
 TEL: (360) 474-9945; FAX: (360) 435-7360
 METHOD OF SURVEY: COMBINATION CLOSED G.P.S. TRAVERSE USING TOPCON HIPERLITE SURVEY GRADE G.P.S. EQUIPMENT AND CONVENTIONAL RANDOM TRAVERSE USING TOPCON ITS-1 1" TOTAL STATION INSTRUMENT & GEODIMETER 600 3" ROBOTIC INSTRUMENT.
 THIS SURVEY MEETS OR EXCEEDS THE REQUIREMENTS OF W.A.C. 332-130-090 AND THE STATE SURVEY RECORDING ACT. THIS SURVEY DOES NOT PURPORT TO SHOW ALL EASEMENTS, RESTRICTIONS, RESERVATIONS, OR ENCUMBRANCES OF RECORD OR NOT OF RECORD.



FOUND QUARTER SECTION CORNER
2 1/2" OUTSIDE DIAMETER IRON PIPE WITH BRASS
NAIL TIP 0.7' BELOW SURFACE IN STEEL CASE
VISITED: 8-21-2008
PT. NO.: 176-3-1-21-35-1

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
APPROVED FOR CONSTRUCTION

By: _____ Date: _____
 R/W PERMIT NO. _____ PFN _____



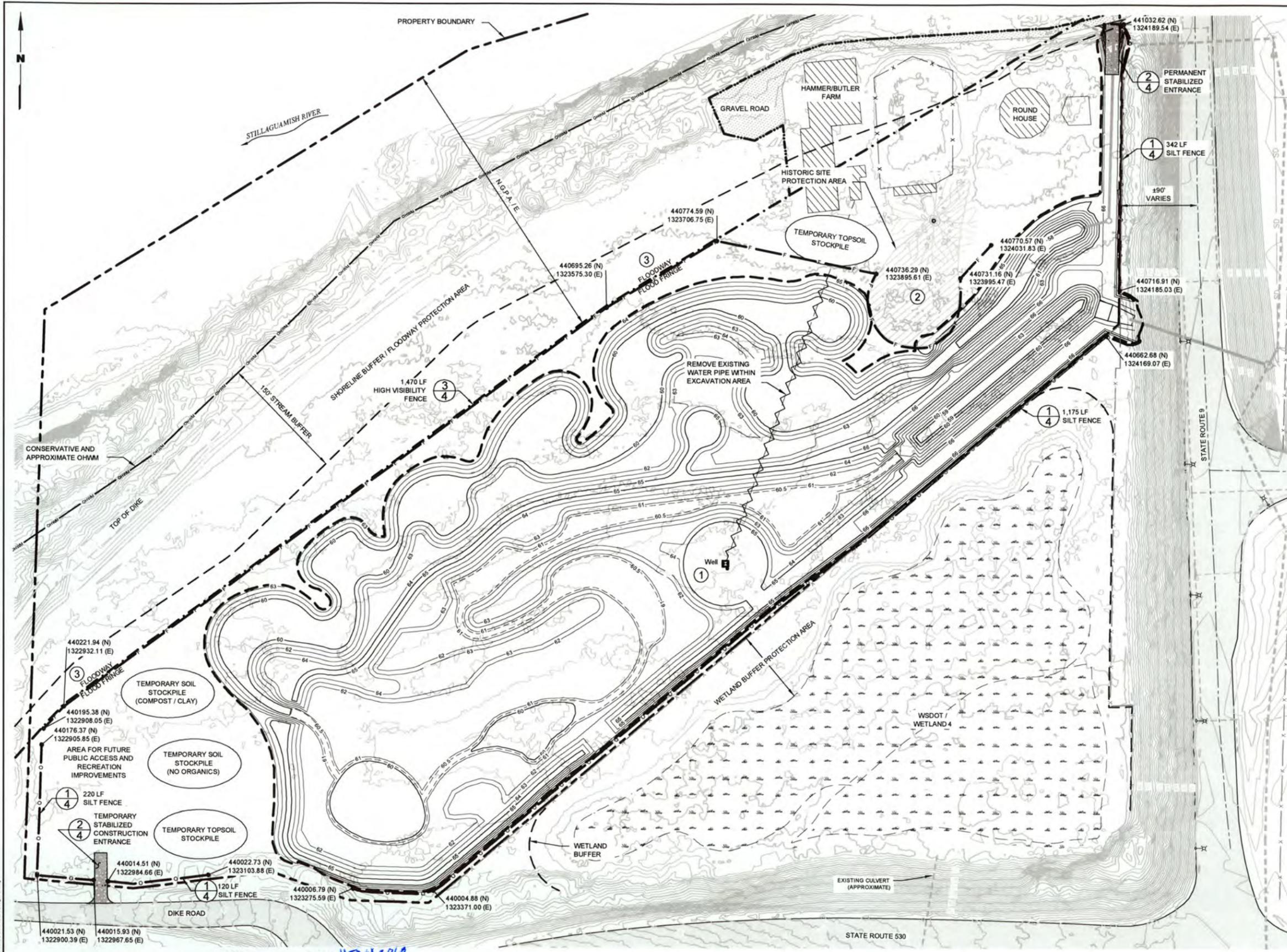
NO.	DATE	REVISIONS	DESIGNED	REVIEWED	APPROVED	STATUS	INITIAL	DATE
						DRAFTED BY: B. TAYLOR	BLT	1-12-2010
						DESIGNED BY: K. WRIGG	KEW	1-12-2010
						REVIEWED BY: D. PISCHER	DAP	1-12-2010
						APPROVED BY:		

LANDAU ASSOCIATES
 130 2ND AVENUE S.
 EDMONDS, WASHINGTON 98020
 (425) 778-0907, FAX (425) 778-6409

**CITY OF ARLINGTON
 STORMWATER WETLAND PROJECT
 SNOHOMISH COUNTY, WASHINGTON**

HORIZONTAL CONTROL PLAN

PROJECT NO.
1097001.010.015
 DATE
1-12-2010
 SHEET
2 OF 13
 DRAWING NO.
2



- LEGEND**
- PROPERTY BOUNDARY
 - PROPOSED CONTOURS (1 FT)
 - PROPOSED CONTOURS (5 FT)
 - EXISTING CONTOURS (1 FT)
 - CLEARING AND GRADING LIMITS
 - SILT FENCE
 - HIGH VISIBILITY FENCE
 - EXISTING WIRE FENCE
 - ROAD CENTERLINE
 - PROPOSED PIPE
 - EXISTING PIPE
 - N.G.P.A. / E. BOUNDARY
 - APPROXIMATE LOCATION OF FLOODWAY BOUNDARY (SNOHOMISH COUNTY PERMIT AND ZONING MAP 2009)
 - ORDINARY HIGH WATER MARK
 - WETLAND/STREAM BUFFER
 - EXISTING GRAVEL ROAD
 - EXISTING PAVED ROAD
 - EXISTING WETLAND

- KEY NOTES**
- 1 CONTRACTOR TO FLAG AND PROTECT EXISTING WELL. EXISTING WELL PIPE TO BE REMOVED. PROTECT AREA SURROUNDING WELL.
 - 2 NO EXCAVATION TO OCCUR WITHIN HISTORIC SITE PROTECTION AREA.
 - 3 NO EARTHWORK TO OCCUR WITHIN FLOODWAY, EXCEPT FOR ACCESS TRAILS.

CONSTRUCTION SEQUENCING NOTES

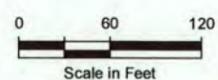
WORK IS EXPECTED TO BE CONDUCTED AS FOLLOWS UNLESS IT WILL CAUSE INEFFICIENCIES. THE GOAL OF THE SCHEDULE BELOW IS TO INCREASE LIKELIHOOD FOR PROJECT SUCCESS AND FACILITATE APPROVAL BY PROJECT BIOLOGIST/ENGINEER AT END OF EACH TASK.

NOTE: BORING AND PIPING WORK TO BE PHASED AT CONTRACTOR'S DISCRETION.

- UPON CONTRACT AWARD:**
- A. CONDUCT ONSITE PRE-CONSTRUCTION MEETING WITH CITY BIOLOGISTS AND ENGINEERS.
 - B. SECURE MATERIALS AND PROVIDE DOCUMENTATION TO CITY FOR APPROVAL FOR ANY MODIFICATIONS OR SUBSTITUTIONS.
- EARLY SUMMER (EXPECTED JUNE 2010):**
- A. INSTALL EROSION CONTROLS AND STAKE GRUBBING LIMITS.
 - B. STOCKPILE SOILS, GRAVELS, ETC. ORDER PLANTS.
 - C. CLEAR ORGANIC LAYER (APPROXIMATELY 1 FOOT DEEP) AND STOCKPILE ONSITE AND STAKE ELEVATION.
 - D. CREATE ROUGH SUBGRADE (OVER-EXCAVATE BY 1 FT) AND STAKE ELEVATION.
 - E. INSTALL WEIRS, SPALLS, SPILLWAY, TRAIL, ROADWAY, AND OTHER SUCH HARD FEATURES.
 - F. CREATE FINAL ROUGH GRADE USING STOCKPILED ORGANIC MATERIAL AND STAKE ELEVATION.
 - G. INSTALL ADDITIONAL SOIL AMENDMENT AND STABILIZATION FEATURES.
 - H. INSTALL SIGNAGE, FENCING, TRAIL SURFACING, ETC.
 - I. HYDROSEED ENTIRE AREA (REFER TO SEEDING NOTES).
 - J. ALLOW STORMWATER TO ENTER SITE (OR AS DETERMINED BY PROJECT BIOLOGIST/ENGINEER).
- NOTE: ABOVE-LISTED TASKS SHALL BE COMPLETED BY SEPTEMBER 30.
- OCTOBER-NOVEMBER:**
- A. CONDUCT PLANTING AND WETLAND SEEDING.
 - B. PREPARE AND SUBMIT AS-BUILT SURVEY AND DOCUMENTATION.
 - C. PORTION OF PROJECT CLOSEOUT (OR AS DESCRIBED IN CONTRACT).
- MAY-JUNE 2011:**
- A. REMOVE BMPs, RESEED, AND RECTIFY ANY SITE ISSUES, PER DIRECTION OF PROJECT BIOLOGIST/ENGINEER.
- OCTOBER 2011:**
- A. REPLACE PLANTS, ETC. PER PROJECT WARRANTIES (SEE SPECIFICATIONS FOR DETAILS).
 - B. FINAL PROJECT CLOSE OUT.

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
APPROVED FOR CONSTRUCTION

By: _____ Date: _____
R/W PERMIT NO. _____ PFN _____



NO.	DATE	REVISIONS	DESIGNED	REVIEWED	APPROVED	STATUS	INITIAL	DATE

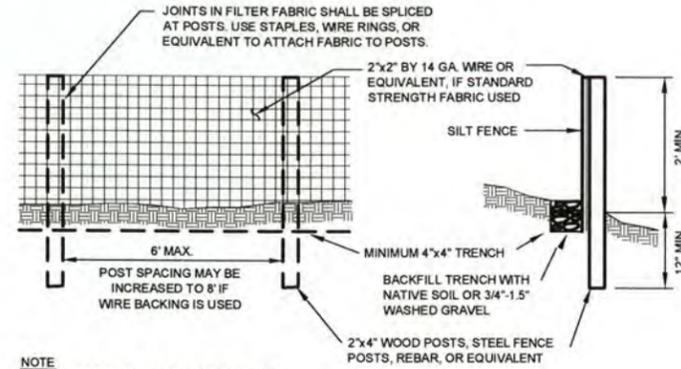
DRAFTED BY: B. TAYLOR BLT 1-12-2010
 DESIGNED BY: K. WRIGG KEW 1-12-2010
 REVIEWED BY: D. FISCHER DAP 1-12-2010
 APPROVED BY: _____

LANDAU ASSOCIATES
 130 2ND AVENUE S.
 EDMONDS, WASHINGTON 98020
 (425) 778-0907, FAX (425) 778-6409

CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON

TEMPORARY EROSION /
SEDIMENT CONTROL PLAN

PROJECT NO: 1097001.010.015
 DATE: 1-12-2010
 SHEET: 3 OF 13
 DRAWING NO: 3

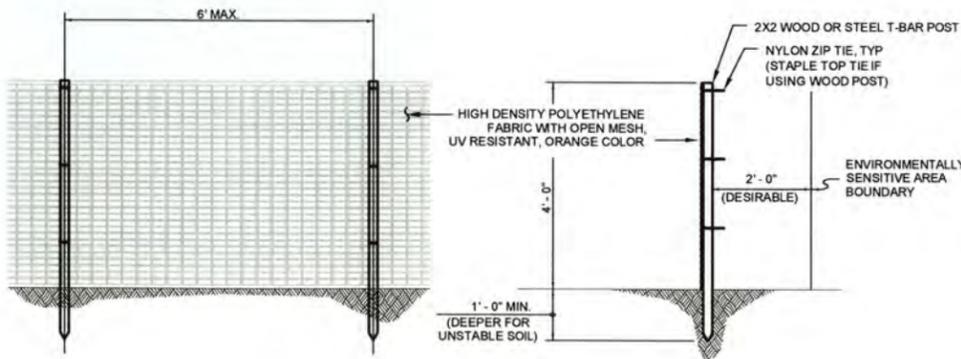


NOTE
SILT FENCES SHALL BE INSTALLED ALONG CONTOURS WHENEVER POSSIBLE.

1 SILT FENCE
3/4 NTS

SILT FENCE MAINTENANCE STANDARDS:

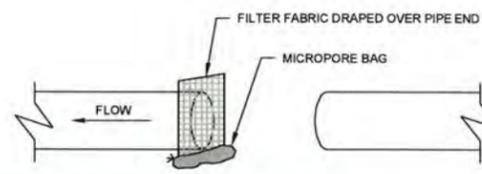
- ANY DAMAGE SHALL BE REPAIRED IMMEDIATELY.
- IF CONCENTRATED FLOWS ARE EVIDENT UPHILL OF THE FENCE, THEY MUST BE INTERCEPTED AND CONVEYED TO A SEDIMENT TRAP OR POND.
- IT IS IMPORTANT TO CHECK THE UPHILL SIDE OF THE FENCE FOR SIGNS OF THE FENCE CLOGGING AND ACTING AS A BARRIER TO FLOW AND THEN CAUSING CHANNELIZATION OF FLOWS PARALLEL TO THE FENCE. IF THIS OCCURS, REPLACE THE FENCE OR REMOVE THE TRAPPED SEDIMENT.
- SEDIMENT MUST BE REMOVED WHEN THE SEDIMENT IS 6 INCHES HIGH.
- IF THE FILTER FABRIC (GEOTEXTILE) HAS DETERIORATED DUE TO ULTRAVIOLET BREAKDOWN, IT SHALL BE REPLACED.



3 HIGH VISIBILITY FENCE
3/4 NTS

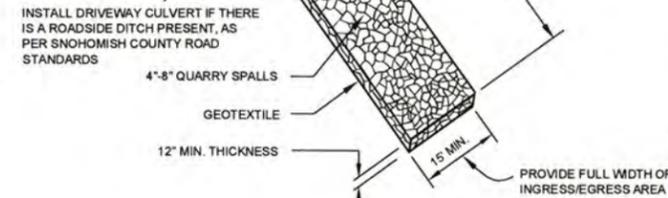


TYPICAL PIPE INLET PROTECTION



TYPICAL CUT PIPE PROTECTION

2 STABILIZED CONSTRUCTION ENTRANCE
3/4 NTS



GENERAL NOTES

- NON-COMPLIANCE WITH THE EROSION CONTROL REQUIREMENTS, WATER QUALITY REQUIREMENTS, AND CLEARING LIMITS VIOLATIONS MAY RESULT IN REVOCATION OF PROJECT PERMITS, PLAN APPROVAL, AND BOND FORECLOSURES.
- PRIOR TO ANY SITE CONSTRUCTION TO INCLUDE CLEARING/LOGGING OR GRADING, THE SITE/OT CLEARING LIMITS SHALL BE LOCATED AND FIELD-IDENTIFIED BY THE PROJECT SURVEYOR/ENGINEER AS REQUIRED BY THESE PLANS. THE PROJECT SURVEYOR/ENGINEER'S NAME AND TELEPHONE NUMBER ARE LANDAU ASSOCIATES INC., (425) 778-0907.
- THE DEVELOPER/OWNER IS RESPONSIBLE FOR WATER QUALITY.
- PRIOR TO ANY SITE WORK, THE CONTRACTOR SHALL CONTACT THE CHIEF INSPECTOR FOR LAND DEVELOPMENT DIVISION AT (425) 388-3385 TO SCHEDULE A PRECONSTRUCTION CONFERENCE. DUE TO FIELD CHANGES (REVISIONS), ENGINEERED AS-BUILTS SHALL BE REQUIRED PRIOR TO SITE APPROVAL.
- THE TEMPORARY EROSION/SEDIMENTATION CONTROL FACILITY SHALL BE CONSTRUCTED PRIOR TO ANY GRADING OR EXTENSIVE LAND CLEARING IN ACCORDANCE WITH THE APPROVED TEMPORARY EROSION/SEDIMENTATION CONTROL PLAN. THESE FACILITIES MUST BE SATISFACTORILY MAINTAINED UNTIL CONSTRUCTION AND LANDSCAPING IS COMPLETED AND THE POTENTIAL FOR ON-SITE EROSION HAS BEEN REDUCED.
- ALL SITE WORK MUST COMPLY TO CHAPTER 33 OF THE INTERNATIONAL BUILDING CODE AND TITLES 30.63B AND 30.63A OF THE SNOHOMISH COUNTY CODE.
- ALL EARTHWORK SHALL BE PERFORMED IN ACCORDANCE WITH COUNTY STANDARDS. PRECONSTRUCTION SOILS INVESTIGATION MAY BE REQUIRED TO EVALUATE SOILS STABILITY.
- T.E.S.C. MEASURES SHALL BE INSTALLED PRIOR TO ANY OTHER SITE WORK. THESE T.E.S.C. MEASURES SHALL BE CONSIDERED MINIMUM TO BE INSTALLED UNLESS DIRECTED OTHERWISE.
- A GRADING PERMIT ISSUED PURSUANT TO TITLE 30.63B SCC AND CHAPTER 33 IBC AND APPROVAL OF THE TEMPORARY EROSION AND SEDIMENTATION CONTROL PLAN SHALL HAVE BEEN OBTAINED FROM SNOHOMISH COUNTY PDS FOR ANY ON-SITE GRADING WHICH IS NOT EXPRESSLY EXCEPTED BY SECTION SCC 30.68B.020.
- PUBLIC STREETS ARE TO BE KEPT CLEAR OF DIRT AND DEBRIS DURING DURATION OF WORK.
- THE SITE SOILS ARE GENERALLY PARTICULARLY MOISTURE SENSITIVE AND AS SUCH ARE SUSCEPTIBLE TO DISTURBANCE BY CONSTRUCTION EQUIPMENT, PARTICULARLY DURING PERIODS OF WET WEATHER. THE GRADING CONTRACTOR SHALL TAKE ALL PRECAUTIONS TO LIMIT SURFACE DISTURBANCE AND PROTECT THE SITE GRADING AREA FROM EXCESSIVE RUNOFF EROSION.
- SOIL STOCKPILES SHALL BE STABILIZED OR PROTECTED WITH SEDIMENT RETENTION BMP'S WITHIN 24 HOURS OF FORMATION.
- GRADING AND CONSTRUCTION SHALL BE TIMED AND CONDUCTED IN STAGES TO MINIMIZE SOIL EXPOSURE.
- CLEAR PLASTIC SHEETING SHALL HAVE A MIN. THICKNESS OF 6 MIL. AND MEET THE REQUIREMENTS OF WSDOT/APWA SECTION 9-14.5.
- PLASTIC SHEETING SHOULD BE TOED IN AT THE TOP OF THE SLOPE TO PREVENT SURFACE FLOW BENEATH THE PLASTIC.
- PLASTIC SHEETING SHALL BE ANCHORED WITH SANDBAGS OR TIRES ON ROPES WITH A MAXIMUM GRID SPACING OF 10' IN ALL DIRECTIONS.
- WHERE POSSIBLE, NATURAL VEGETATIVE FILTER STRIPS SHOULD BE RETAINED TO LESSEN RUNOFF ON STEEP SLOPES.
- UNVEGETATED SLOPES SHOULD BE TEMPORARILY SCARIFIED TO MINIMIZE RUNOFF. THIS MAY BE ACCOMPLISHED BY RUNNING A DOZER UP THE SLOPE TO CREATE GROOVES PERPENDICULAR TO THE SLOPE DIRECTION.
- INLETS OF THE PERMANENT DRAINAGE SYSTEM SHALL BE PROTECTED FROM SEDIMENT INFLUX BY USE OF FILTER FABRIC, MICROPORE BAGS, AND SIMILAR FILTERING MATERIALS AND METHODS. (SEE DETAIL ON THIS SHEET).
- THE T.E.S.C. MEASURES ARE TO BE REMOVED UPON THE COMPLETION OF SITE WORK AND/OR SITE STABILIZATION UPON APPROVAL OF THE ENGINEER.
- CONSTRUCTION ACCEPTANCE WILL BE SUBJECT TO A WELL ESTABLISHED VEGETATION AND GROUND COVER THAT FULFILLS THE REQUIREMENT OF THE APPROVED CONSTRUCTION PLANS AND TITLE 30.63B SCC.
- WHEN RAINFALL IS HEAVY (DEFINED AS RAINFALL HARD ENOUGH TO PRODUCE SEDIMENT RUN OFF FROM EXPOSED DIRT), ALL EXPOSED EARTHWORK SHALL BE COVERED. NO OTHER CONSTRUCTION ACTIVITY SHALL OCCUR ON PVIOUS SURFACES DURING THESE PERIODS OF HEAVY RAIN.
- SEDIMENT DEPOSITS SHALL BE REMOVED FROM ALL TEMP. DRAINAGE FACILITIES AND STRUCTURES UPON REACHING A DEPTH OF 6 INCHES.
- SUFFICIENT T.E.S.C. BMP MATERIALS AND SUPPLIES TO PROTECT THE ENTIRE SITE SHALL BE STOCKPILED ON-SITE.
- FROM OCTOBER 1 TO APRIL 30, NO SOIL MAY REMAIN EXPOSED FOR MORE THAN 2 DAYS.
- FROM MAY 1 TO SEPTEMBER 30, NO SOIL MAY REMAIN EXPOSED FOR MORE THAN 7 DAYS.
- DENUDED AREAS SHALL BE COVERED BY MULCH, SOD, PLASTIC, OR OTHER BMP'S AS NEEDED.
- ALL LOCATIONS OF EXISTING UTILITIES HAVE BEEN ESTABLISHED BY FIELD SURVEY OR OBTAINED FROM AVAILABLE RECORDS AND SHOULD THEREFORE BE CONSIDERED APPROXIMATE ONLY AND NOT NECESSARILY COMPLETE. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO INDEPENDENTLY VERIFY THE ACCURACY OF ALL UTILITY LOCATIONS, AND TO FURTHER DISCOVER AND AVOID ANY OTHER UTILITIES WHICH MAY BE AFFECTED BY HIS WORK. THE CONTRACTOR SHALL CONTACT THE UTILITIES UNDERGROUND LOCATION SERVICE (1-800-424-5555) PRIOR TO CONSTRUCTION. THE OWNER OR HIS REPRESENTATIVE SHALL BE IMMEDIATELY CONTACTED IF A UTILITY CONFLICT EXISTS. A FEE OF \$35.00 WILL BE CHARGED FOR EACH RE-LOCATE REQUEST.
- ALL PIPE SHALL BE PLACED ON STABLE EARTH, OR IF IN THE OPINION OF THE ENGINEER THE EXISTING FOUNDATION IS UNSATISFACTORY, THEN IT SHALL BE EXCAVATED BELOW GRADE AND BACKFILLED TO GRADE WITH SAND-GRAVEL, CRUSHED ROCK, OR OTHER SUITABLE MATERIAL. NEVER INSTALL PIPE ON SOD, FROZEN EARTH, LARGE BOULDERS OR ROCK.
- THE BACKFILL SHALL BE PLACED EQUALLY ON BOTH SIDES OF THE PIPE OR PIPE-ARCH IN LAYERS WITH A LOOSE AVERAGE DEPTH OF 6" MAXIMUM DEPTH 8", THOROUGHLY TAMPING EACH LAYER. THESE COMPACTED LAYERS MUST EXTEND FOR ONE DIAMETER ON EACH SIDE OF THE PIPE OR TO THE SIDE OF THE TRENCH. MATERIALS TO COMPLETE THE FILL OVER PIPE SHALL BE THE SAME AS DESCRIBED (REFER TO WSDOT STD. SPEC).
- ALL FILLS SHALL BE COMPACTED TO A MINIMUM OF 95% OF MAXIMUM DRY DENSITY BY MODIFIED PROCTOR TEST, UNLESS OTHERWISE NOTED.
- ALL WETLAND CELL AREAS SHALL HAVE TOPSOIL/ORGANIC SOIL THAT IS ROUGH-GRADED TO AVOID OVER-COMPACTION AND TO CREATE SMALL HUMMOCKS IN SURFACE.

GEOTEXTILES
GEOTEXTILE USED FOR PERMANENT EROSION CONTROL BELOW SLOPE PROTECTION MATERIALS SHALL BE A WOVEN GEOTEXTILE MEETING THE REQUIREMENTS IN WSDOT STANDARD SPECIFICATION SECTION 9-33.2(1), TABLE 4 HIGH SURVIVABILITY AND TABLE 5 DRAINAGE CLASS C, AND SHALL BE INSTALLED IN ACCORDANCE WITH WSDOT STANDARD SPECIFICATION SECTION 2-12. A MINIMUM 6 INCHES OF AGGREGATE CUSHION MEETING THE REQUIREMENTS IN WSDOT STANDARD SPECIFICATION SECTION 9-33.3 SHALL BE PLACED BETWEEN THE GEOTEXTILE AND RIPRAP MATERIALS.

GEOTEXTILE USED FOR SOIL SEPARATION OR STABILIZATION BELOW FILL MATERIALS SHALL BE A NONWOVEN GEOTEXTILE MEETING THE REQUIREMENTS IN WSDOT STANDARD SPECIFICATION SECTION 9-33.2(1), TABLE 3 FOR SOIL STABILIZATION AND SHALL BE INSTALLED IN ACCORDANCE WITH WSDOT STANDARD SPECIFICATION SECTION 2-12.

GRAVEL SURFACING FOR ROADWAYS AND TRAILS
GRAVEL SURFACING FOR ACCESS ROADS AND TRAILS SHALL MEET THE REQUIREMENTS IN WSDOT STANDARD SPECIFICATION SECTION 9-03.9(3) FOR CRUSHED SURFACING TOP COURSE, AND SHALL BE PLACED IN MAXIMUM 8 INCH LOOSE LIFTS AND COMPACTED TO A FIRM AND UNYIELDING CONDITION.

TOPSOIL STOCKPILING AND PLACEMENT
EXISTING ORGANIC TOPSOIL WITHIN THE WORK AREA SHALL BE STRIPPED/GRUBBED AND STOCKPILED AT THE DESIGNATED LOCATIONS. THE MATERIAL SHALL BE PROCESSED (MIXED AND BLENDED) TO FORM A CONSISTENT TOPSOIL MIXTURE, AND SHALL BE PLACED IN ALL WETLAND POOL AND CHANNEL AREAS TO FORM A SURFICIAL 1 FT PLANTING LAYER. TRACK THE TOPSOIL IN PLACE AND AVOID OVER-COMPACTION OF THE SURFICIAL PLANTING LAYER. SEPARATE AND MAINTAIN THE MAXIMUM AMOUNT OF TOPSOIL/ORGANIC MATERIAL (TURF/SOD) FROM NON-ORGANIC SOILS.

CLAYEY SOILS AND NATIVE SOIL STOCKPILING AND PLACEMENT
NON-ORGANIC CLAY AND SILTY CLAY MATERIALS ENCOUNTERED IN EXCAVATIONS SHALL BE SEGREGATED AND STOCKPILED AS DIRECTED BY THE ENGINEER. SUCH CLAYEY MATERIALS SHALL BE PLACED AND COMPACTED WITHIN CELLS 1, 2, AND 3 AS DIRECTED BY THE ENGINEER TO HELP REDUCE SOIL INFILTRATION IN THESE CELLS.

OTHER NON-ORGANIC NATIVE MATERIALS ENCOUNTERED IN EXCAVATIONS SHALL BE SEGREGATED AND STOCKPILED AS DIRECTED BY THE ENGINEER. SUCH MATERIALS SHALL BE PLACED AND COMPACTED TO FORM THE WETLAND BERMS, WITH EXCESS MATERIAL TO BE TRANSPORTED TO AN APPROVED OFFSITE FILL AREA.

WEIR SUBGRADE PREPARATION
IF DIRECTED BY THE ENGINEER, TEMPORARILY STOCKPILE EXCAVATED NATIVE SOIL OVER THE WEIR FOUNDATION AREA TO SURCHARGE THE SUBGRADE SOILS AND LIMIT THE POTENTIAL FOR POST-CONSTRUCTION SETTLEMENT.

EXCAVATE EXISTING SOILS TO ACHIEVE THE DESIRED SUBGRADE ELEVATION AT EACH WEIR LOCATION. REMOVE ANY DELETERIOUS MATERIAL AS DIRECTED BY THE ENGINEER. REWORK AND COMPACT THE EXPOSED SOIL SO THAT THE UPPER 1 FOOT OF THE NATIVE SUBGRADE MATERIAL IS COMPACTED TO AT LEAST 95 PERCENT OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY THE ASTM D 1557 TEST PROCEDURE (MODIFIED PROCTOR). ANY AREAS OF SOFT/LOOSE SOIL OR AREAS EXHIBITING SIGNIFICANT DEFLECTION/PUMPING/WEAVING THAT CANNOT BE ADEQUATELY REWORKED AND/OR COMPACTED SHALL BE OVEREXCAVATED AND REPLACED WITH COMPACTED CRUSHED ROCK.

PLACE A SOIL STABILIZATION GEOTEXTILE OVER THE PREPARED SUBGRADE PRIOR TO PLACEMENT AND COMPACTION OF THE CRUSHED ROCK LAYER BELOW THE WEIR FOUNDATION. CRUSHED ROCK SHALL MEET THE REQUIREMENTS IN WSDOT STANDARD SPECIFICATION SECTION 9-03.9(3) FOR CRUSHED SURFACING BASE COURSE, AND SHALL BE COMPACTED TO AT LEAST 95 PERCENT OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY THE ASTM D 1557 TEST PROCEDURE (MODIFIED PROCTOR).

BERM CONSTRUCTION
FOLLOWING STRIPPING AND GRUBBING OF ORGANIC TOPSOIL AND ANY OTHER DELETERIOUS MATERIAL WITHIN THE FOOTPRINT OF THE BERM, REWORK AND COMPACT THE EXPOSED SOIL SO THAT THE UPPER 1 FOOT OF THE NATIVE SUBGRADE MATERIAL IS COMPACTED TO AT LEAST 92 PERCENT OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY THE ASTM D 1557 TEST PROCEDURE (MODIFIED PROCTOR). ANY AREAS OF SOFT/LOOSE SOIL OR AREAS EXHIBITING SIGNIFICANT DEFLECTION, PUMPING, OR WEAVING THAT CANNOT BE ADEQUATELY REWORKED AND/OR COMPACTED SHALL BE OVEREXCAVATED AND REPLACED WITH COMPACTED FILL MATERIAL.

MOISTURE CONDITION, PLACE, AND COMPACT NON-ORGANIC NATIVE FILL MATERIALS IN MAXIMUM 8 INCH LOOSE LIFTS TO CONSTRUCT THE WETLAND BERMS. COMPACT THE BERM FILL MATERIAL TO AT LEAST 95 PERCENT OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY THE ASTM D 1557 TEST PROCEDURE (MODIFIED PROCTOR). IF NEEDED TO ACHIEVE ADEQUATE COMPACTION NEAR THE FACE OF MORE CRITICAL BERM AREAS, OVERBUILD AND THEN TRIM THE BERM FILL TO ITS LINES AND GRADES.

RIPPRAP AND QUARRY SPALLS
RIPPRAP AND QUARRY SPALLS USED FOR SLOPE PROTECTION AND EROSION CONTROL SHALL MEET THE REQUIREMENTS IN WSDOT STANDARD SPECIFICATION SECTION 9-13.1(2) FOR LIGHT LOOSE RIPRAP, AND SHALL BE PLACED IN ACCORDANCE WITH WSDOT STANDARD SPECIFICATION SECTION 8-15.

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
APPROVED FOR CONSTRUCTION

By: _____ Date: _____
R/W PERMIT NO. _____ PFN _____



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DESIGNED BY: K. WRIGG KEW 1-12-2010
REVIEWED BY: D. FISCHER DAP 1-12-2010
APPROVED BY: _____
LANDAU ASSOCIATES
130 2ND AVENUE S.
EDMONDS, WASHINGTON 98020
(425) 778-0907, FAX (425) 778-6409

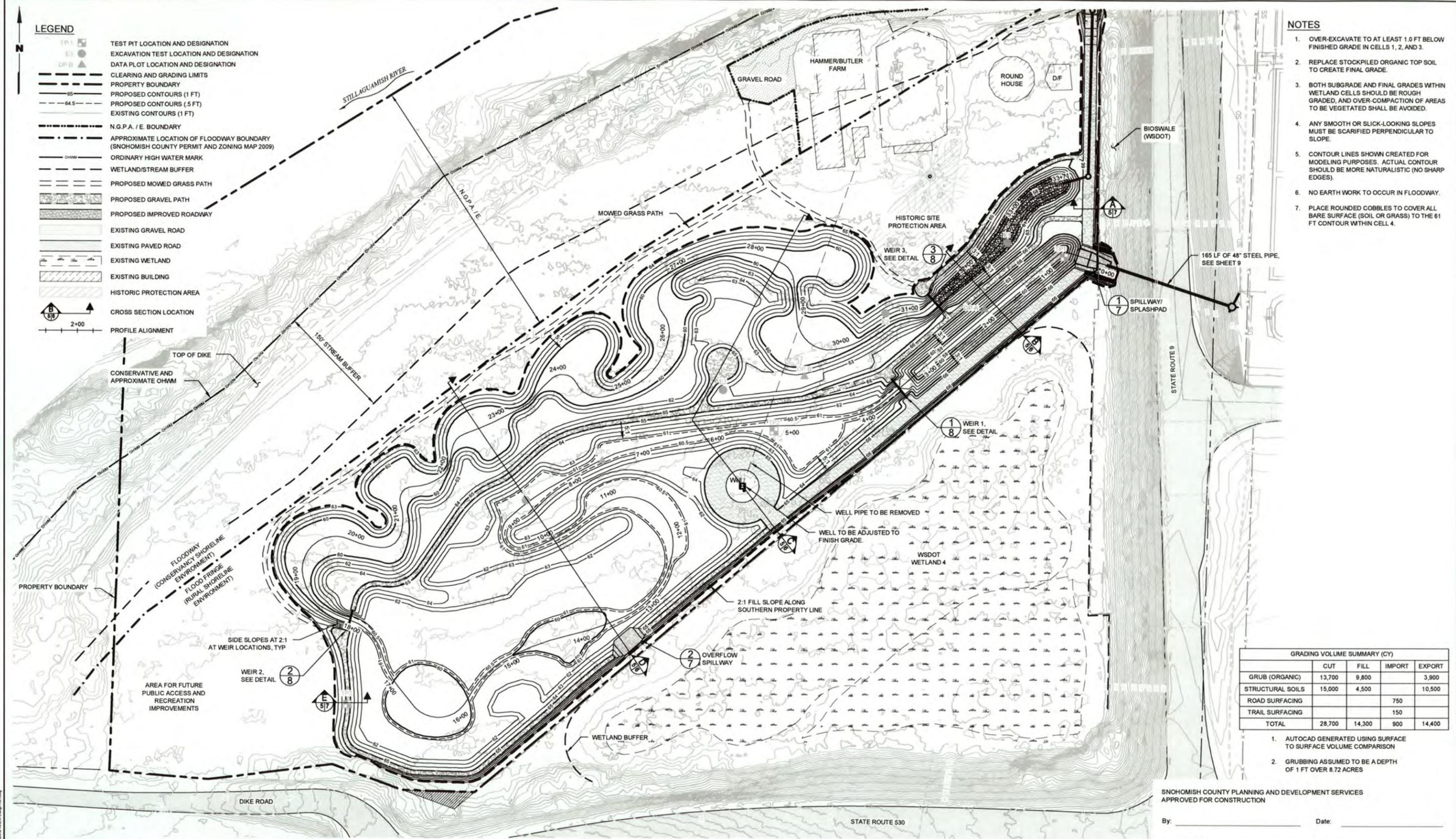
CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON
PROJECT NO: 1097001.010.015
DATE: 1-12-2010
SHEET: 4 OF 13
DRAWING NO: 4
GENERAL NOTES & T.E.S.C DETAILS

LEGEND

- TP-1 TEST PIT LOCATION AND DESIGNATION
- E3 EXCAVATION TEST LOCATION AND DESIGNATION
- DP-B DATA PLOT LOCATION AND DESIGNATION
- CLEARING AND GRADING LIMITS
- PROPERTY BOUNDARY
- 65 PROPOSED CONTOURS (1 FT)
- 64.5 PROPOSED CONTOURS (.5 FT)
- EXISTING CONTOURS (1 FT)
- N.G.P.A. / E. BOUNDARY
- APPROXIMATE LOCATION OF FLOODWAY BOUNDARY (SNOHOMISH COUNTY PERMIT AND ZONING MAP 2009)
- ORDINARY HIGH WATER MARK
- WETLAND/STREAM BUFFER
- PROPOSED MOWED GRASS PATH
- PROPOSED GRAVEL PATH
- PROPOSED IMPROVED ROADWAY
- EXISTING GRAVEL ROAD
- EXISTING PAVED ROAD
- EXISTING WETLAND
- EXISTING BUILDING
- HISTORIC PROTECTION AREA
- CROSS SECTION LOCATION
- 2+00 PROFILE ALIGNMENT

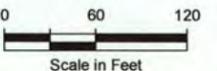
NOTES

1. OVER-EXCAVATE TO AT LEAST 1.0 FT BELOW FINISHED GRADE IN CELLS 1, 2, AND 3.
2. REPLACE STOCKPILED ORGANIC TOP SOIL TO CREATE FINAL GRADE.
3. BOTH SUBGRADE AND FINAL GRADES WITHIN WETLAND CELLS SHOULD BE ROUGH GRADED, AND OVER-COMPACTION OF AREAS TO BE VEGETATED SHALL BE AVOIDED.
4. ANY SMOOTH OR SLICK-LOOKING SLOPES MUST BE SCARIFIED PERPENDICULAR TO SLOPE.
5. CONTOUR LINES SHOWN CREATED FOR MODELING PURPOSES. ACTUAL CONTOUR SHOULD BE MORE NATURALISTIC (NO SHARP EDGES).
6. NO EARTH WORK TO OCCUR IN FLOODWAY.
7. PLACE ROUNDED COBBLES TO COVER ALL BARE SURFACE (SOIL OR GRASS) TO THE 61 FT CONTOUR WITHIN CELL 4.



GRADING VOLUME SUMMARY (CY)				
	CUT	FILL	IMPORT	EXPORT
GRUB (ORGANIC)	13,700	9,800		3,900
STRUCTURAL SOILS	15,000	4,500		10,500
ROAD SURFACING			750	
TRAIL SURFACING			150	
TOTAL	28,700	14,300	900	14,400

1. AUTOCAD GENERATED USING SURFACE TO SURFACE VOLUME COMPARISON
2. GRUBBING ASSUMED TO BE A DEPTH OF 1 FT OVER 8.72 ACRES



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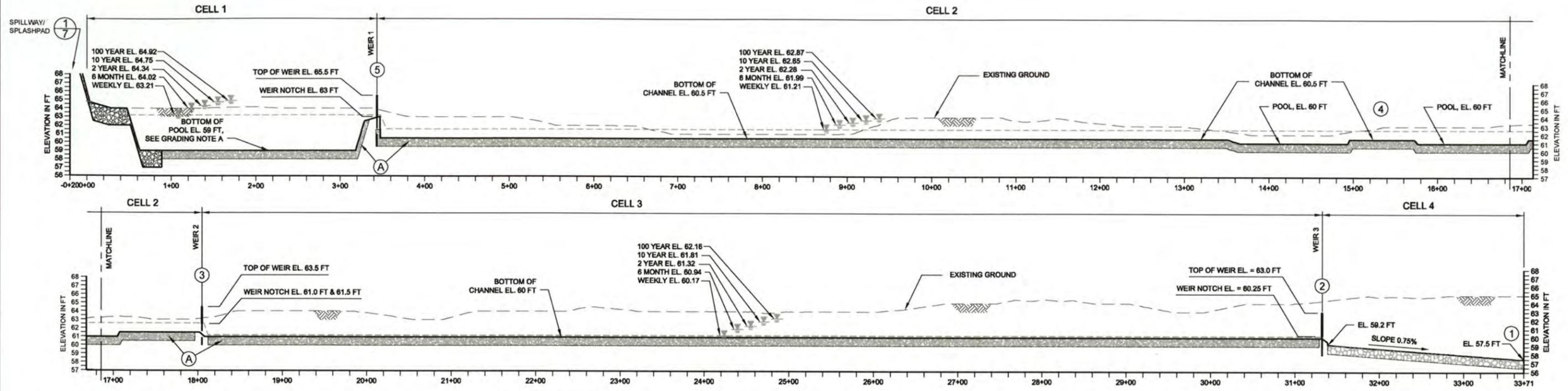
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 STATUS: _____

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SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
APPROVED FOR CONSTRUCTION

By: _____ Date: _____
R/W PERMIT NO. _____ PFN _____

CITY OF ARLINGTON
 STORMWATER WETLAND PROJECT
 SNOHOMISH COUNTY, WASHINGTON
GRADING AND SHORELINE / FLOOD HAZARD PLAN
 PROJECT NO: 1097001.010.015
 DATE: 1-12-2010
 SHEET: 5 OF 13
 DRAWING NO: **5**



GRADING NOTE

(A) EXISTING SUBSURFACE SOIL SHALL BE OVER EXCAVATED BY 1.0 FT IN ALL POOL & CHANNEL AREAS AND REPLACED WITH PROCESSED NATIVE TOPSOIL.

HYDROLOGIC PROFILE

HORIZ SCALE: 1" = 60'
 VERT SCALE: 1" = 6'

RIVER AND PIPE BACKWATER

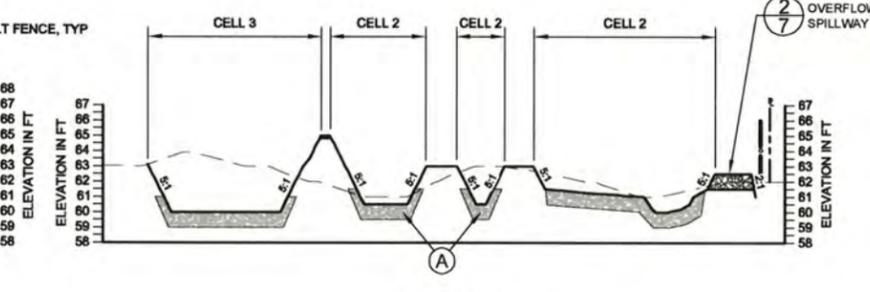
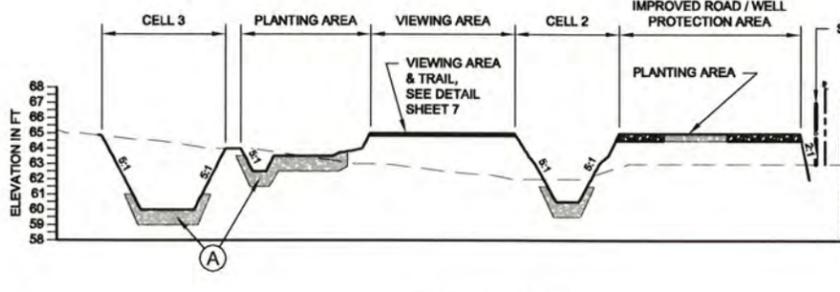
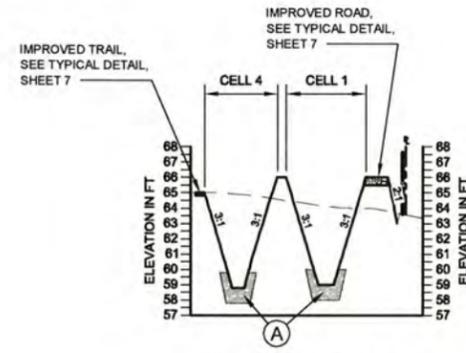
THE EXISTING BUTLER OUTFALL IS OFTEN INUNDATED DURING WINTER AND SPRING RIVER FLOWS. THE BUTLER OUTFALL IS OFTEN AT OR JUST ABOVE THE WATER LEVEL DURING THE SUMMER. DURING THE WINTER THE OUTFALL IS OFTEN UNDERWATER. IN THE EXISTING CONDITION THERE IS NO FLOODGATE ON THE OUTFALL PREVENTING RIVER WATER FROM FLOWING BACK TO THE PROPOSED WETLAND WHEN THE RIVER LEVEL RISES. A FLOODGATE SHOULD BE INSTALLED WHEN THE BUTLER OUTFALL IS REPAIRED AS PART OF A FUTURE PROJECT.

BACKWATER WILL OCCUR IN THE PIPING SYSTEM AND THE WETLAND WHEN THE FLOWS THROUGH THE WETLAND ARE ABOVE THE LEVEL OF A 10 YEAR STORM EVENT. IT CAN ALSO OCCUR WHEN THE RIVER LEVEL RISES HIGH ENOUGH TO PREVENT WETLAND FLOW FROM EXITING THE BUTLER OUTFALL OR RESTRICTS IT ENOUGH TO CAUSE BACKWATER CONDITIONS FOR A LOWER STORM EVENT.

(1) IN THESE SITUATIONS THE BACKWATER BEGINS IN THE BUTLER OUTFALL PIPE. AS WETLAND WATER FLOWS TOWARDS THE OUTFALL AND CANNOT EXIT EITHER AS FAST AS IT ENTERS OR AT ALL, A POOL BEGINS TO FORM IN THE BUTLER OUTFALL PIPE. THIS POOL INTERFERES WITH THE PERFORMANCE OF THE NEXT PIPE IN THE SYSTEM, AND THIS CONTINUES UP THE CHAIN OF PIPE UNTIL THE WATER BEGINS TO POOL IN CELL 4.

- (2) AFTER THE POOL IN CELL 4 REACHES AN ELEVATION OF 60.25 FT, IT WILL BEGIN TO INTERFERE WITH THE FLOWS FROM CELL 3 INTO CELL 4. EVENTUALLY CELL 3 AND CELL 4 WILL BECOME A SINGLE POOL OF WATER WHICH RISES AT THE SAME RATE.
- (3) AFTER THE COMBINED POOL (CELL 3 AND 4) REACHES AN ELEVATION OF 61.0 FT IT WILL BEGIN TO INTERFERE WITH THE FLOWS FROM THE LOWER WEIR 2 NOTCH INTO CELL 3. AT A BACKWATER ELEVATION OF 61.5 FT, IT WILL BEGIN TO INTERFERE WITH THE UPPER (HIGH FLOW) NOTCH FROM CELL 2 INTO CELL 3. EVENTUALLY CELLS 2, 3, AND 4 WILL BECOME A SINGLE POOL OF WATER WHICH RISES AT THE SAME RATE.
- (4) AT A BACKWATER ELEVATION OF 62.5 FT, FLOWS WILL BEGIN INTO THE WSDOT WETLAND THROUGH THE OVERFLOW WEIR FROM CELL 2.
- (5) AT A BACKWATER ELEVATION OF 63 FT, FLOWS WILL BEGIN TO OVERTOP THE WETLAND ON THE NORTHERN SIDE OF THE WETLAND AND FLOW OVER DIKE ROAD TO THE WEST.

RAINFALL SUMMARY	
WEEKLY STORM EVENT	0.38 INCHES
6 MONTH STORM EVENT	1.26 INCHES
2 YEAR STORM EVENT	1.80 INCHES
10 YEAR STORM EVENT	2.75 INCHES
100 YEAR STORM EVENT	3.75 INCHES



SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
 APPROVED FOR CONSTRUCTION

By: _____ Date: _____
 R/W PERMIT NO. _____ PFN _____



NO.	DATE	REVISIONS	DESIGNED	REVIEWED	APPROVED	STATUS	INITIAL	DATE

DRAFTED BY: B. TAYLOR BLT 1-12-2010
 DESIGNED BY: K. WRIGG KEW 1-12-2010
 REVIEWED BY: D. PISCHER DAP 1-12-2010
 APPROVED BY: _____
 STATUS: _____

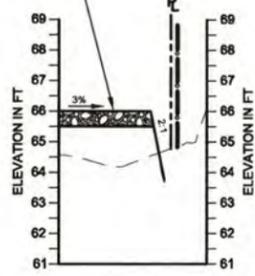


**CITY OF ARLINGTON
 STORMWATER WETLAND PROJECT
 SNOHOMISH COUNTY, WASHINGTON**

GRADING SECTIONS & PROFILES

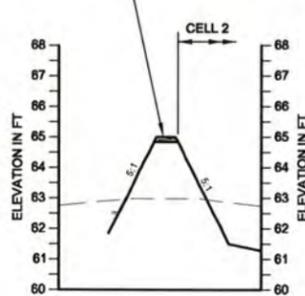
PROJECT NO: 1097001.010.015
 DATE: 1-12-2010
 SHEET: 6 OF 13
 DRAWING NO: 6

IMPROVED ROAD, SEE TYPICAL SECTION

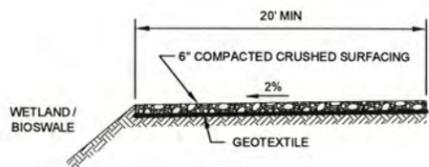


A SECTION A-A
HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 3'

IMPROVED TRAIL, SEE TYPICAL SECTION

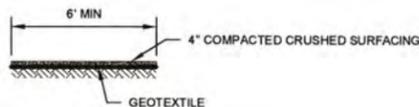


E SECTION E-E
HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 3'

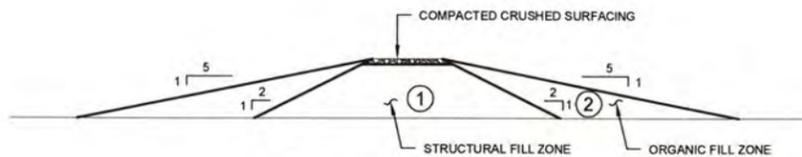


- ROAD SHALL BE MAINTAINED BY THE CITY OF ARLINGTON
- SUBGRADE SHALL BE PREPARED AND COMPACTED IN A MANNER SIMILAR TO THE BERM CONSTRUCTION NOTES ON SHEET 4.

TYPICAL IMPROVED ROAD SECTION
N.T.S.



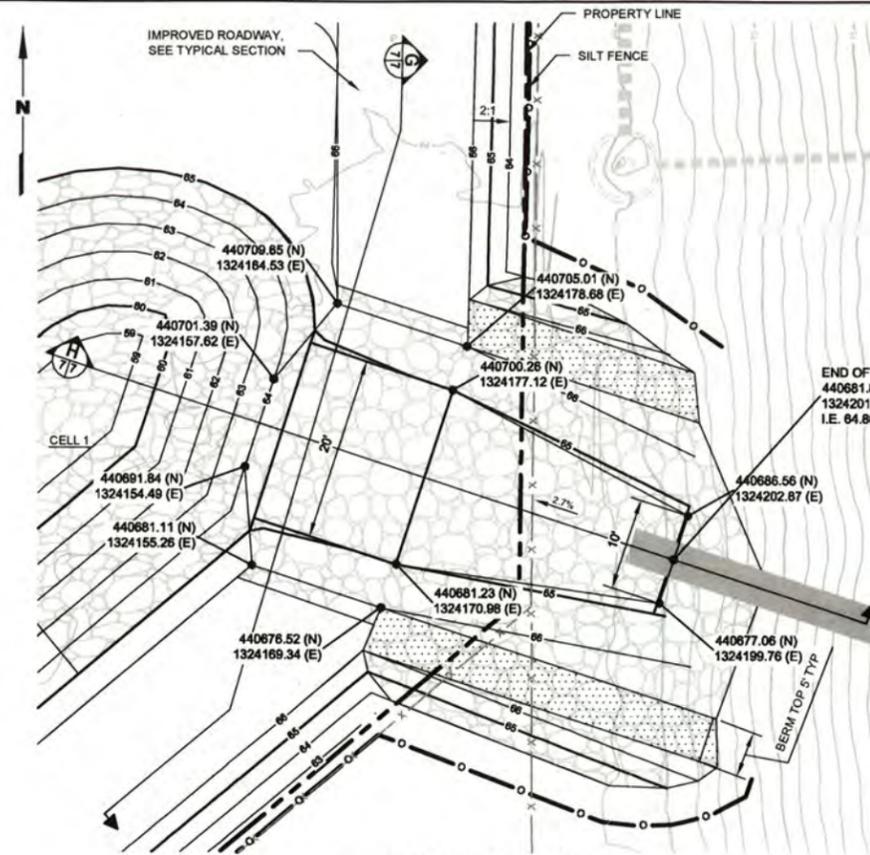
TYPICAL IMPROVED PATH SECTION
N.T.S.



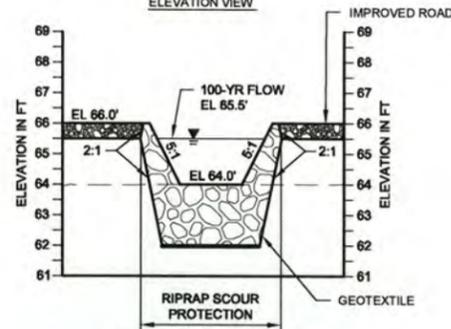
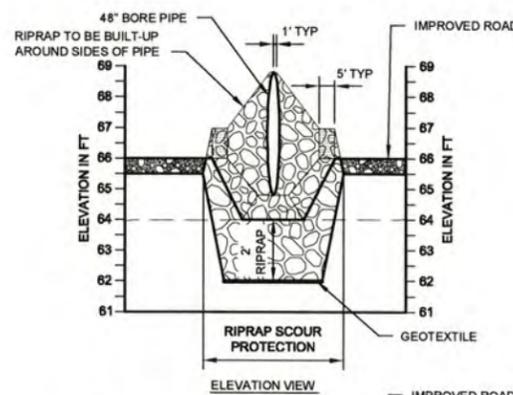
TYPICAL BERM CONSTRUCTION SECTION
N.T.S.

KEY NOTES

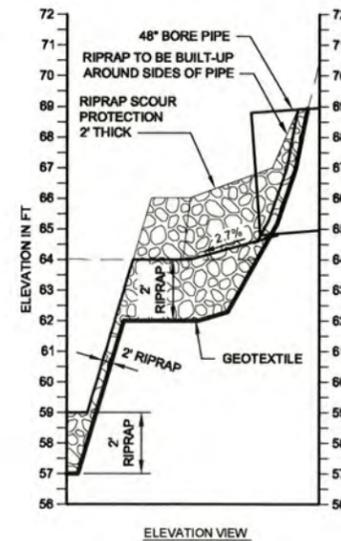
- COMPACT, SEE NOTE 31 ON SHEET 4.
- AVOID OVER-COMPACTION, SEE NOTE 32 ON SHEET 4.



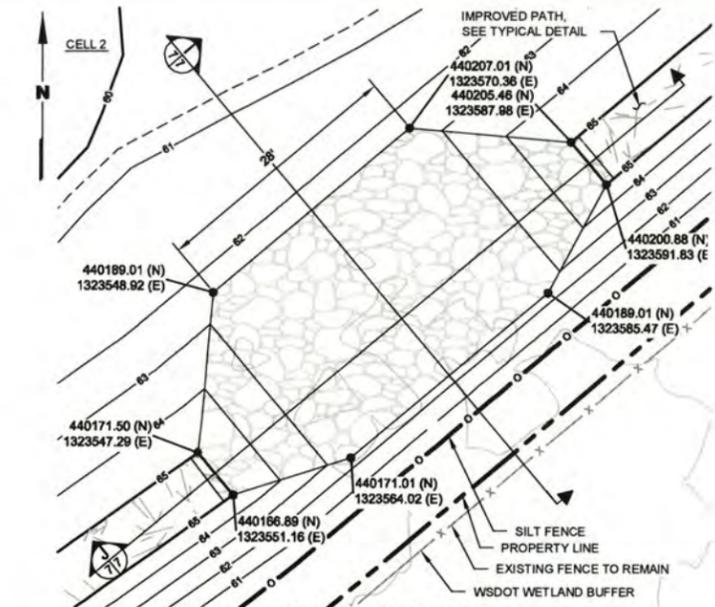
I SPILLWAY/SPLASHPAD
SCALE: 1" = 10'



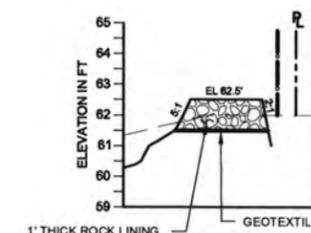
G SECTION G-G
HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 3'



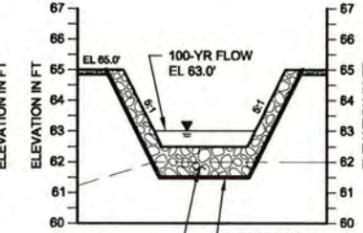
H SECTION H-H
HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 3'



2 OVERFLOW SPILLWAY
SCALE: 1" = 10'



I SECTION I-I
HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 3'



J SECTION J-J
HORIZ SCALE: 1" = 30'
VERT SCALE: 1" = 3'

LEGEND

- CLEARING AND GRADING LIMITS
- SILT FENCE
- PROPERTY BOUNDARY
- PROPOSED CONTOURS (1 FT)
- PROPOSED CONTOURS (5 FT)
- EXISTING CONTOURS (1 FT)
- PROPOSED GRAVEL PATH
- PROPOSED IMPROVED ROADWAY
- CROSS SECTION LOCATION

SCOUR PROTECTION SPECIFICATIONS

ROCK LINING MUST BE QUARRY SPALLS WITH GRADATION AS FOLLOWS:
PASSING 8-INCH-SQUARE SIEVE: 100%
PASSING 3-INCH-SQUARE SIEVE: 40%-60% MAX.
PASSING 3/4-INCH-SQUARE SIEVE: 0-10% MAX.

RIPRAP MUST BE REASONABLY WELL-GRADED WITH GRADATION AS FOLLOWS:
MAXIMUM STONE SIZE: 24-INCH (NOMINAL DIAMETER)
MEDIAN STONE SIZE: 16-INCH
MINIMUM STONE SIZE: 4-INCH

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
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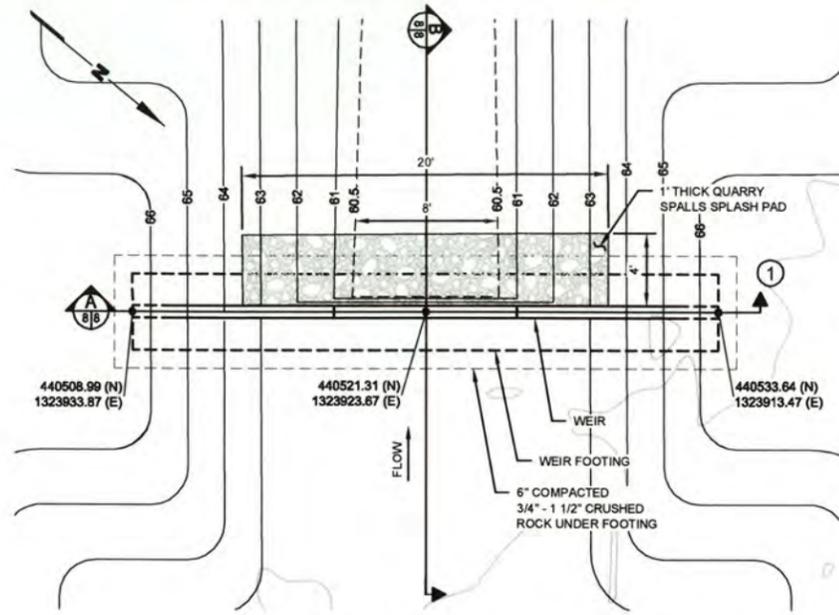
DRAFTED BY:	B. TAYLOR	BLT	1-12-2010
DESIGNED BY:	K. WRIGG	KEW	1-12-2010
REVIEWED BY:	D. PISCHER	DAP	1-12-2010
APPROVED BY:			

LANDAU ASSOCIATES
130 2ND AVENUE S.
EDMONDS, WASHINGTON 98020
(425) 778-0907, FAX (425) 778-6409

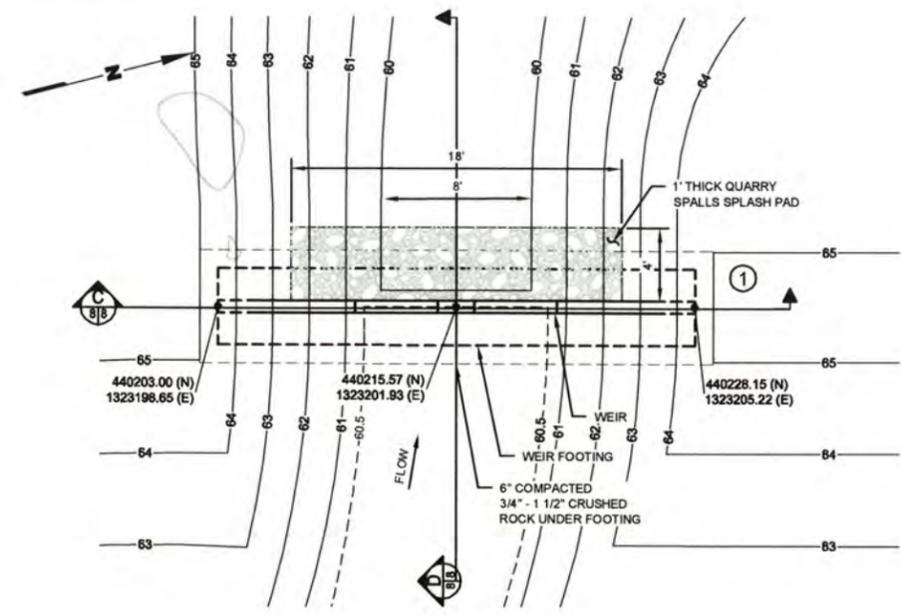
CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON

GRADING DETAILS & SECTIONS

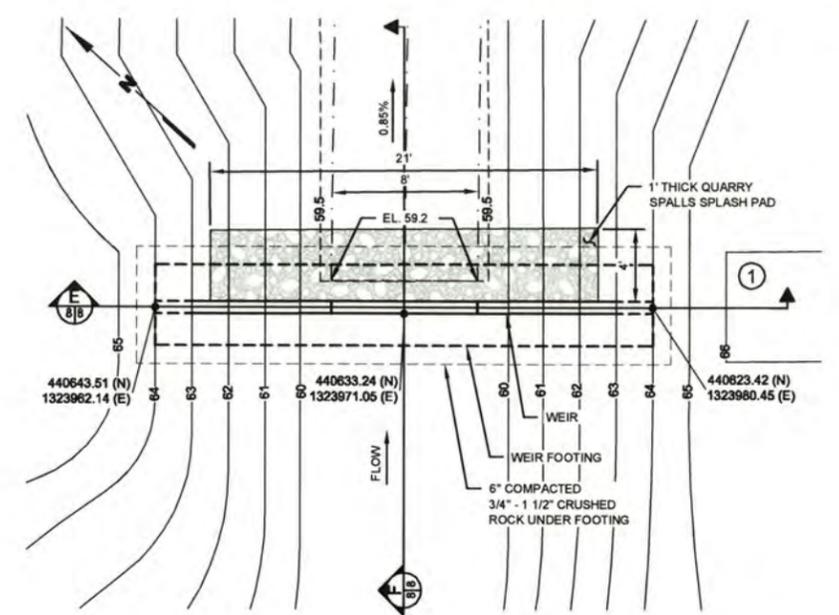
PROJECT NO.	1097001.010.015
DATE	1-12-2010
SHEET	7 OF 13
DRAWING NO.	7



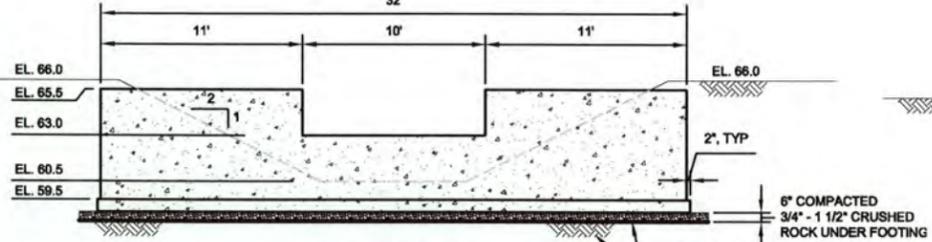
WEIR NO. 1
SCALE: 1" = 5'



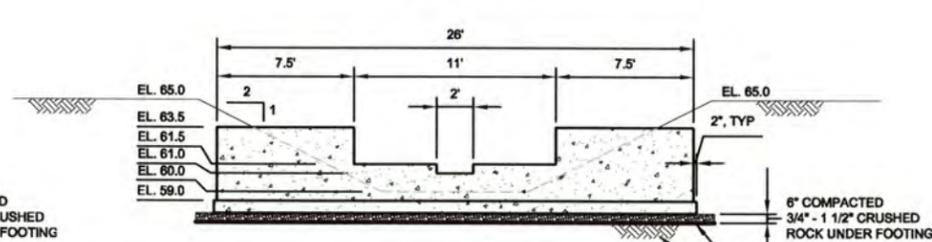
WEIR NO. 2
SCALE: 1" = 5'



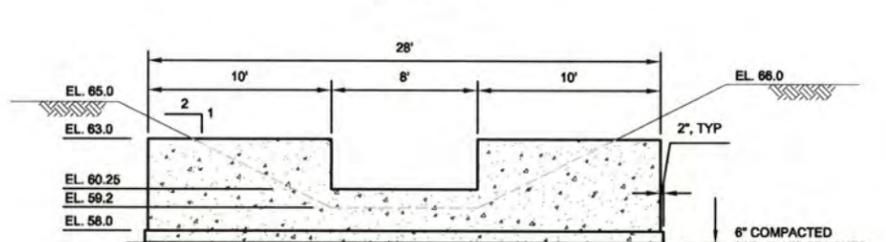
WEIR NO. 3
SCALE: 1" = 5'



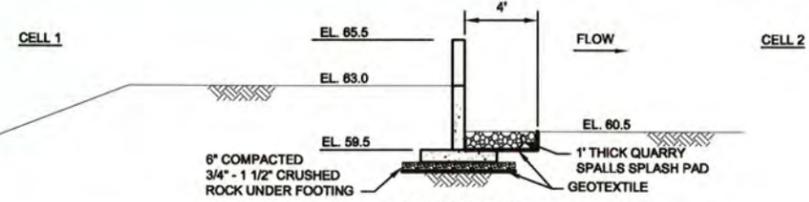
SECTION A-A
SCALE: 1" = 5'



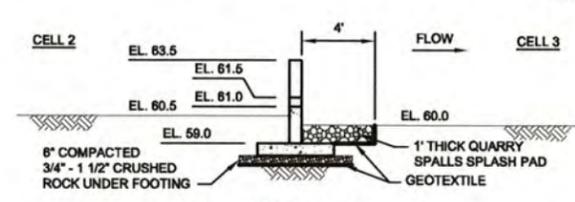
SECTION C-C
SCALE: 1" = 5'



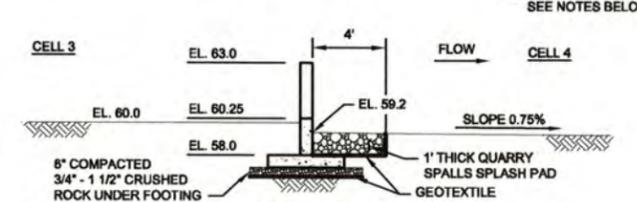
SECTION E-E
SCALE: 1" = 5'



SECTION B-B
SCALE: 1" = 5'



SECTION D-D
SCALE: 1" = 5'



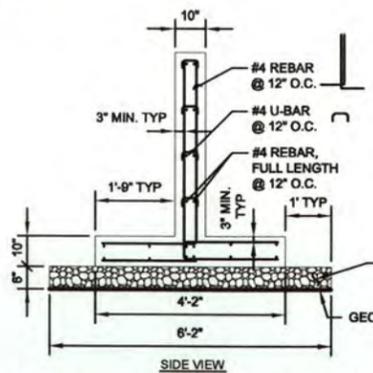
SECTION F-F
SCALE: 1" = 5'

WEIR SUBGRADE PREPARATION

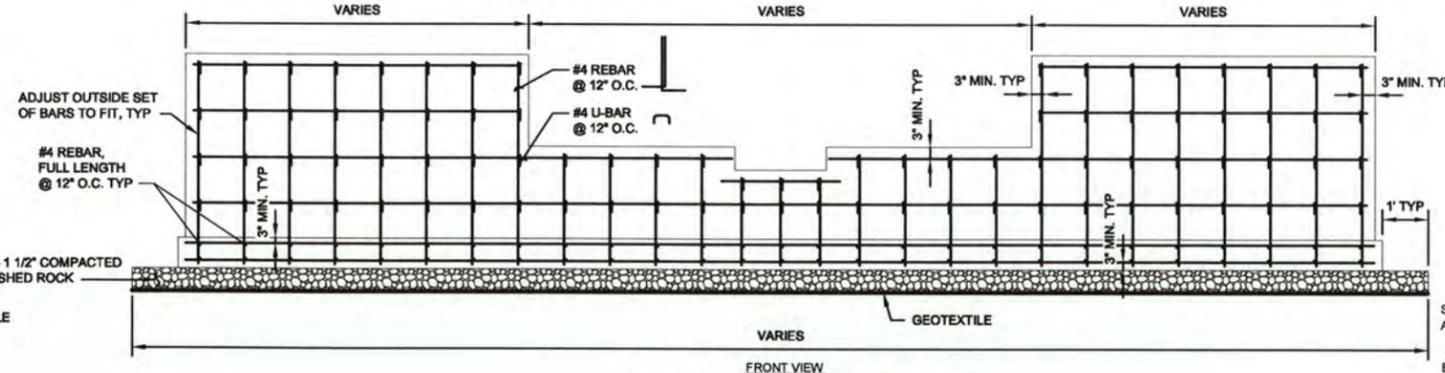
IF DIRECTED BY THE ENGINEER, TEMPORARILY STOCKPILE EXCAVATED NATIVE SOIL OVER THE WEIR FOUNDATION AREA TO SURCHARGE THE SUBGRADE SOILS AND LIMIT THE POTENTIAL FOR POST-CONSTRUCTION SETTLEMENT.

EXCAVATE EXISTING SOILS TO ACHIEVE THE DESIRED SUBGRADE ELEVATION AT EACH WEIR LOCATION. REMOVE ANY DELETERIOUS MATERIAL AS DIRECTED BY THE ENGINEER. REWORK AND COMPACT THE EXPOSED SOIL SO THAT THE UPPER 1 FOOT OF THE NATIVE SUBGRADE MATERIAL IS COMPACTED TO AT LEAST 95 PERCENT OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY THE ASTM D 1557 TEST PROCEDURE (MODIFIED PROCTOR). ANY AREAS OF SOFT/LOOSE SOIL OR AREAS EXHIBITING SIGNIFICANT DEFLECTION/PUMPING/WEAVING THAT CANNOT BE ADEQUATELY REWORKED AND/OR COMPACTED SHALL BE OVEREXCAVATED AND REPLACED WITH COMPACTED CRUSHED ROCK.

PLACE A SOIL STABILIZATION GEOTEXTILE OVER THE PREPARED SUBGRADE PRIOR TO PLACEMENT AND COMPACTION OF THE CRUSHED ROCK LAYER BELOW THE WEIR FOUNDATION. CRUSHED ROCK SHALL MEET THE REQUIREMENTS IN WSDOT STANDARD SPECIFICATION SECTION 9-03.9(3) FOR CRUSHED SURFACING BASE COURSE, AND SHALL BE COMPACTED TO AT LEAST 95 PERCENT OF ITS MAXIMUM DRY DENSITY AS DETERMINED BY THE ASTM D 1557 TEST PROCEDURE (MODIFIED PROCTOR).



TYPICAL WEIR REINFORCEMENT DETAIL
SCALE: 1" = 2'



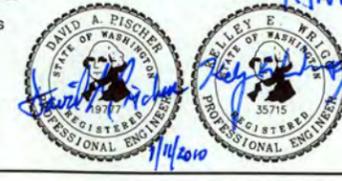
TYPICAL WEIR REINFORCEMENT DETAIL
SCALE: 1" = 2'

KEY NOTE

1 CONTRACTOR TO INSTALL FOOTBRIDGES TO SPAN EACH OF THE WEIR CONSTRUCTIONS. BRIDGES TO MEET THE REQUIREMENTS OF THE PROJECT SPECIFICATIONS. BOTTOM OF BRIDGE STRUCTURE TO HAVE 1 FT MINIMUM CLEARANCE FROM THE TOP OF THE WEIR WALL. BRIDGES ARE NOT TO BE CONNECTED OR ANCHORED TO THE WEIR STRUCTURE IN ANY WAY.

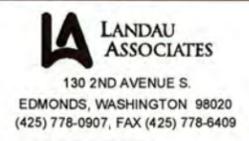
SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES APPROVED FOR CONSTRUCTION

By: _____ Date: _____
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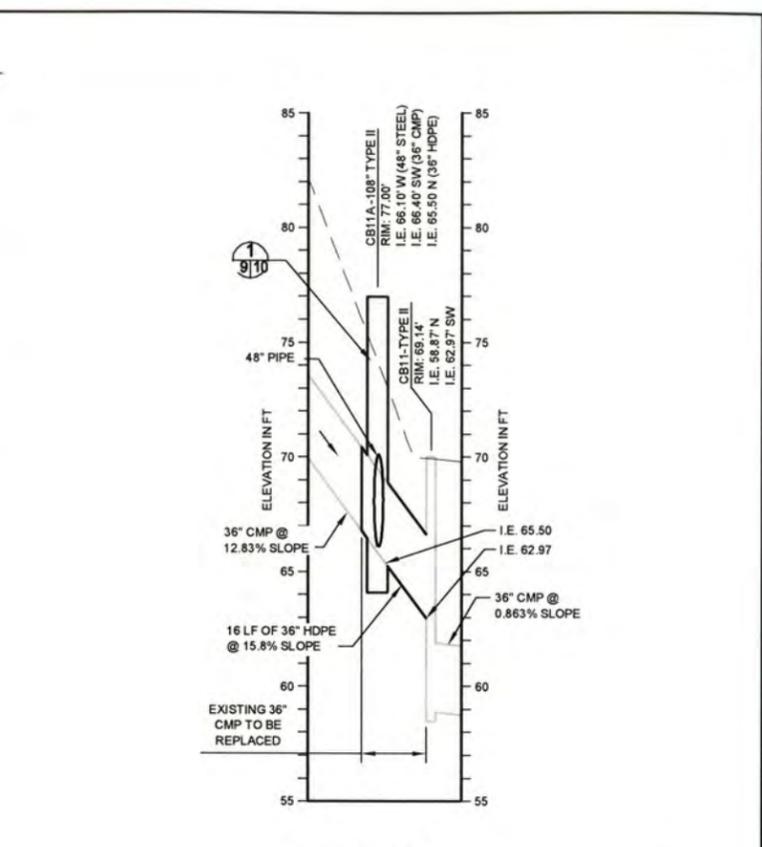
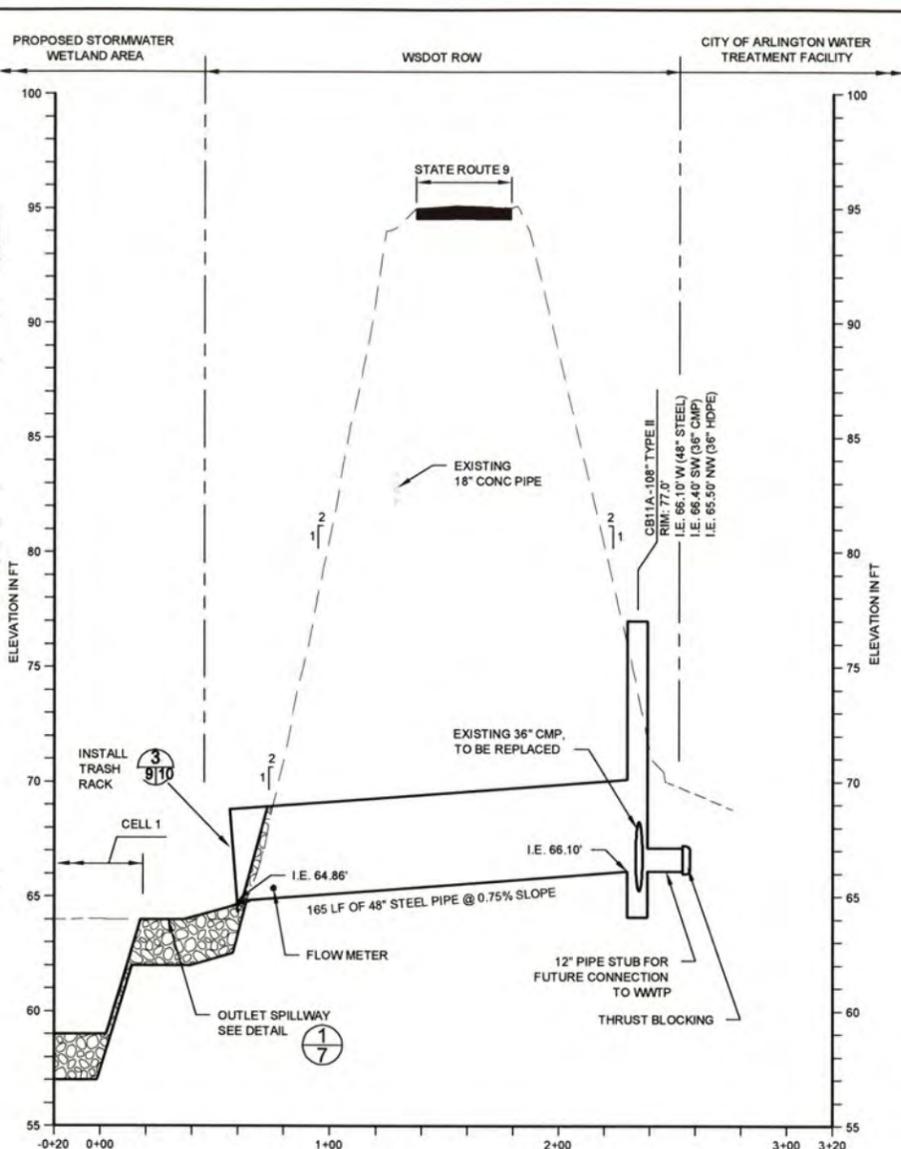
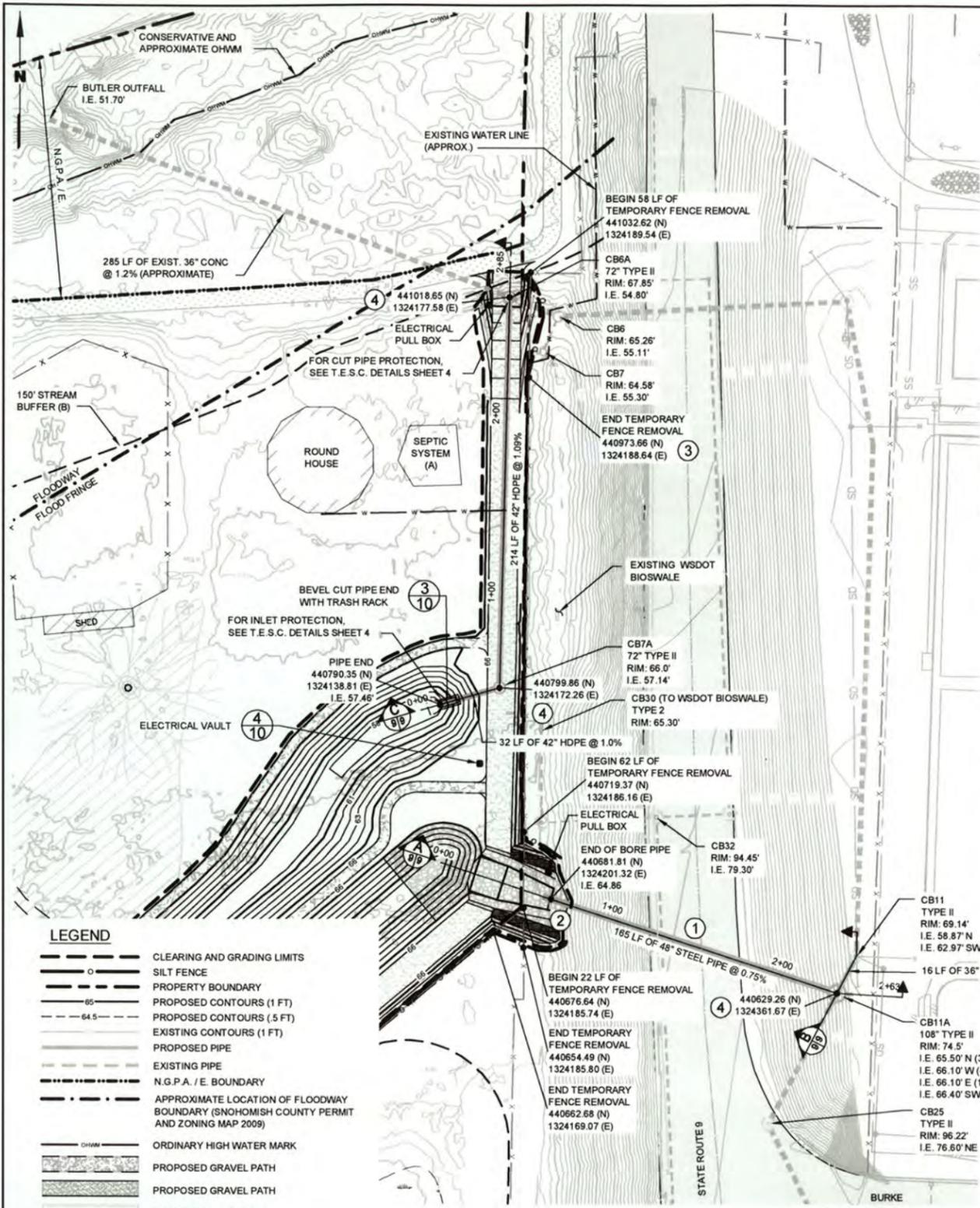
DRAFTED BY:	B. TAYLOR	BLT	1-12-2010
DESIGNED BY:	K. WRIGG	KEW	1-12-2010
REVIEWED BY:	D. PISCHER	DAP	1-12-2010
APPROVED BY:			



CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON

WEIR PLANS, SECTIONS & DETAILS

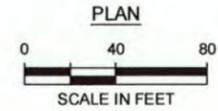
PROJECT NO.	1097001.010.015
DATE	1-12-2010
SHEET	8 OF 13
DRAWING NO.	8



- LEGEND**
- CLEARING AND GRADING LIMITS
 - SILT FENCE
 - PROPERTY BOUNDARY
 - PROPOSED CONTOURS (1 FT)
 - PROPOSED CONTOURS (5 FT)
 - EXISTING CONTOURS (1 FT)
 - PROPOSED PIPE
 - EXISTING PIPE
 - N.G.P.A. / E. BOUNDARY
 - APPROXIMATE LOCATION OF FLOODWAY BOUNDARY (SNOHOMISH COUNTY PERMIT AND ZONING MAP 2009)
 - OHWM
 - PROPOSED GRAVEL PATH
 - EXISTING GRAVEL ROAD
 - EXISTING PAVED ROAD
 - EXISTING WETLAND
 - EXISTING BUILDING
 - HISTORIC PROTECTION AREA
 - CROSS SECTION LOCATION

KEY NOTES

- ① PIPE TO BE INSTALLED VIA A BORE, JACK, OR TUNNELING MACHINE PER THE PROJECT SPECIFICATIONS.
- ② RIPRAP HEADWALL TO BE INSTALLED AROUND PIPE OUTLET, SEE DETAIL 1 SHEET 7.
- ③ CONTRACTOR TO REPLACE FENCE REMOVED TO INSTALL PROJECT IMPROVEMENTS. REPLACED FENCE TO GO AROUND SPILLWAY AND TO MATCH TO EXISTING FENCE MATERIALS.
- ④ CATCH BASIN REFERENCE LOCATIONS ARE THE CENTER OF STRUCTURE.



SECTION A-A
HORIZONTAL SCALE: 1" = 40'
VERTICAL SCALE: 1" = 4'

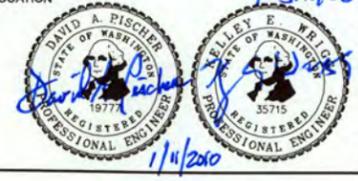
QUARRY SPALLS HEADWALL
SCALE: 1" = 5'

SECTION C-C
HORIZONTAL SCALE: 1" = 40'
VERTICAL SCALE: 1" = 4'

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
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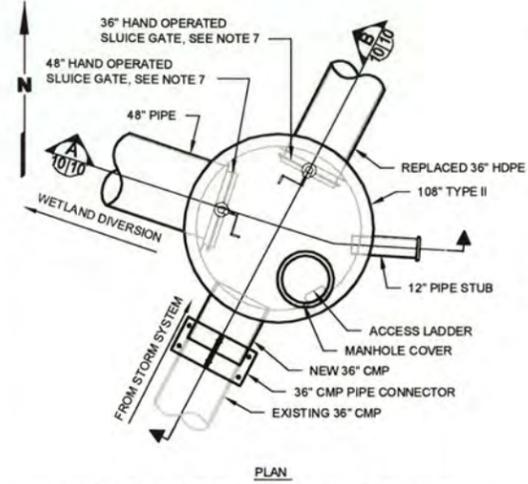
DRAFTED BY:	B. TAYLOR	BLT	1-12-2010
DESIGNED BY:	K. WRIGG	KEW	1-12-2010
REVIEWED BY:	D. FISCHER	DAP	1-12-2010
APPROVED BY:			

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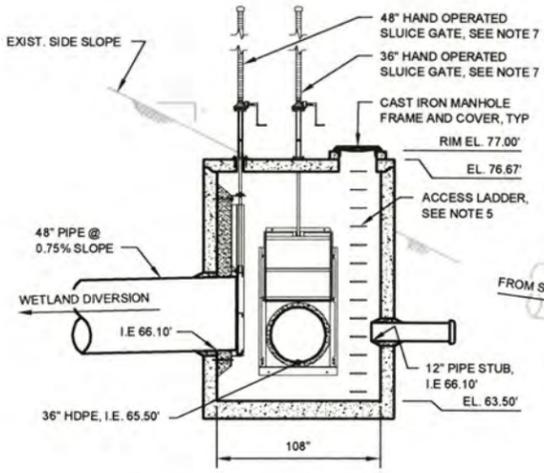
**CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON**

PROJECT NO.	1097001.010.015
DATE	1-12-2010
SHEET	9 OF 13
DRAWING NO.	9

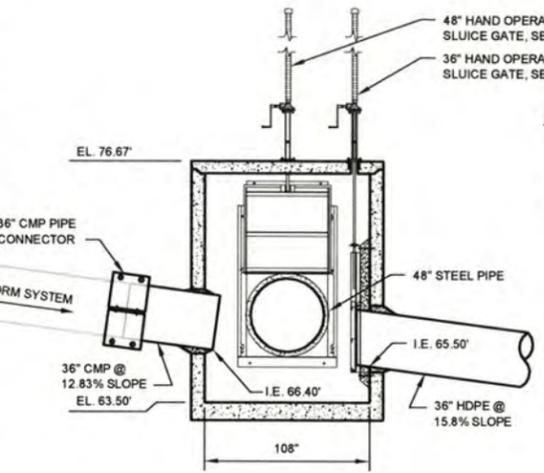
PIPING PLAN



1 CB11A - 108" TYPE II CATCH BASIN DETAIL
SCALE: 1" = 5'

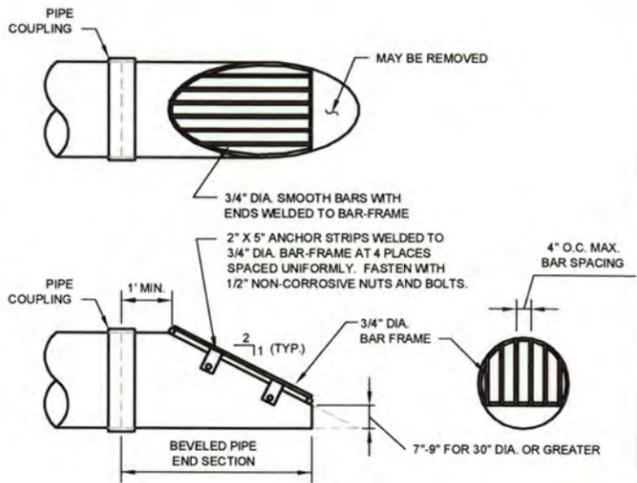


A CB11A - SECTION A-A
SCALE: 1" = 5'



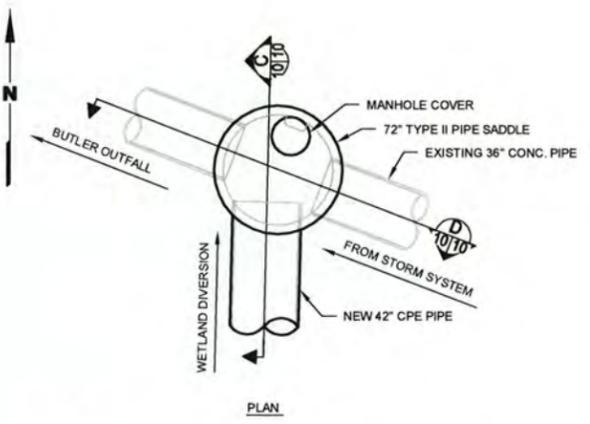
B CB11A - SECTION B-B
SCALE: 1" = 5'

- NOTES**
- PIPE SIZES AND SLOPES: PER PLANS.
 - OUTLET CAPACITY: NOT LESS THAN COMBINED INLETS.
 - EXCEPT AS SHOWN OR NOTED, UNITS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE REQUIREMENTS FOR CATCH BASIN TYPE 2, 108" MIN. DIA.
 - OUTLET SHALL BE CONNECTED TO CULVERT OR SEWER PIPE WITH A STANDARD COUPLING BAND FOR CORRUGATED METAL PIPE, OR GROUTED INTO THE BELL OF CONCRETE PIPE.
 - FRAME AND LADDER OR STEPS OFFSET SO THAT CLIMB DOWN SPACE IS CLEAR OF RISER AND CLEANOUT GATE.
 - IF METAL OUTLET PIPE CONNECTS TO CEMENT CONCRETE PIPE: OUTLET PIPE TO HAVE SMOOTH O.D. EQUAL TO CONCRETE PIPE I.D. LESS 1/4".
 - SLUICE GATES SHALL BE STAINLESS STEEL, WALL MOUNTED, NON-SELF CONTAINED FRAME, WITH GEARED CRANK OPERATED GOLDEN HARVEST, INC. MODEL NUMBER GH-46 TYPE 304 SLIDE GATE OR APPROVED EQUAL.

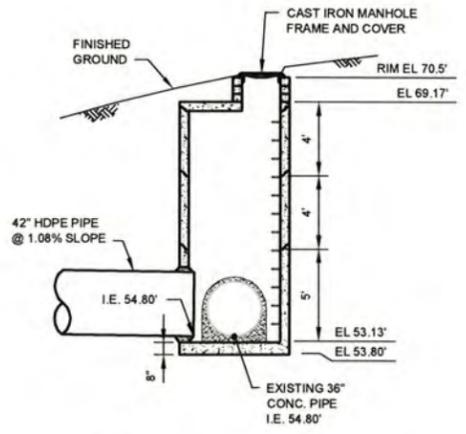


- NOTES**
- SIDE SLOPE SHALL BE WARPED TO MATCH THE BEVELED PIPE END. WHEN CULVERT IS ON SKEW, BEVELED END SHALL BE ROTATED TO CONFORM TO SLOPE. IF SLOPE DIFFERS FROM 2:1, PIPE SHALL BE BEVELED TO MATCH SLOPE.
 - 3/4" DIA. BAR FRAME TO BE CAPABLE OF BOLTING TO FRAME WHEN NOT BEING ACCESSED.
 - ALL STEEL PARTS MUST BE GALVANIZED AND ASPHALT COATED (TREATMENT 1 OR BETTER).

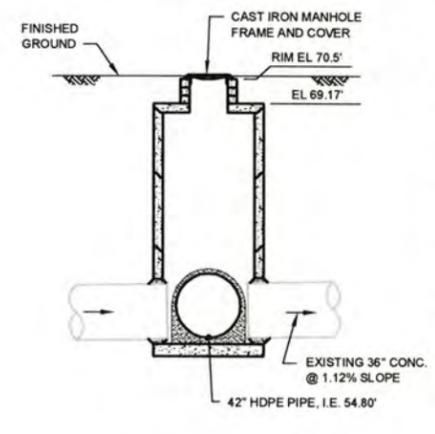
3 BEVELED END PIPE SECTION AND TRASH RACK
N.T.S.



2 CB6A - 72" TYPE II SADDLE DETAIL
SCALE: 1" = 5'

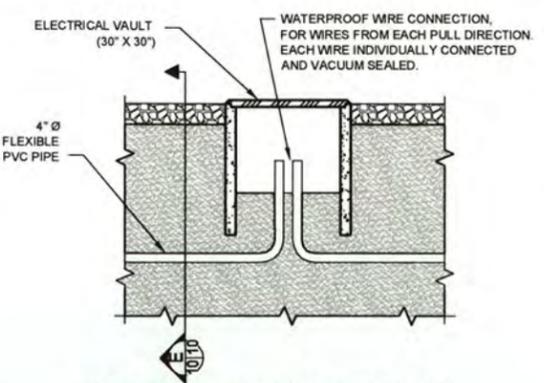


C CB6A - SECTION C-C
SCALE: 1" = 5'

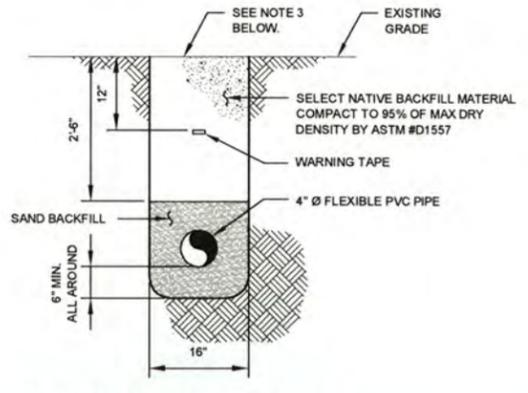


D CB6A - SECTION D-D
SCALE: 1" = 5'

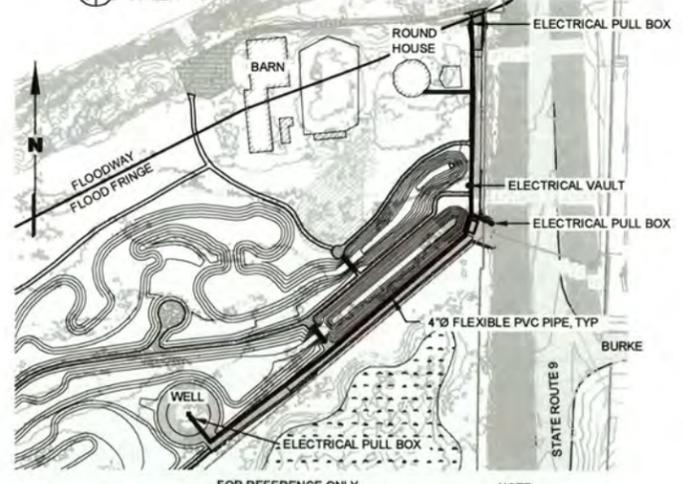
- NOTES**
- PIPE SIZES AND SLOPES: PER PLANS.
 - OUTLET CAPACITY: NOT LESS THAN COMBINED INLETS.
 - EXCEPT AS SHOWN OR NOTED, UNITS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE REQUIREMENTS FOR CATCH BASIN TYPE 2, 108" MIN. DIA.
 - OUTLET SHALL BE CONNECTED TO CULVERT OR SEWER PIPE WITH A STANDARD COUPLING BAND FOR CORRUGATED METAL PIPE, OR GROUTED INTO THE BELL OF CONCRETE PIPE.
 - FRAME AND LADDER OR STEPS OFFSET SO THAT CLIMB DOWN SPACE IS CLEAR OF RISER AND CLEANOUT GATE.
 - IF METAL OUTLET PIPE CONNECTS TO CEMENT CONCRETE PIPE: OUTLET PIPE TO HAVE SMOOTH O.D. EQUAL TO CONCRETE PIPE I.D. LESS 1/4".
 - PRIOR TO CUTTING THE EXISTING STORMWATER PIPE THE CONTRACTOR SHALL HAVE EXACT MEASUREMENTS OF THE EXISTING PIPE INVERTS AND LENGTH MEASUREMENTS FOR INSTALLATION OF THE NEW MANHOLE.
 - THE CONTRACTOR'S EXCAVATION SHALL BE ADEQUATE TO INSTALL THE SADDLE MANHOLE PRIOR TO CUTTING THE EXISTING STORMWATER PIPE.
 - DOWNSTREAM INLET SHALL BE PROTECTED AT ALL TIMES FROM SEDIMENT DURING SADDLE INSTALLATION, FOR CONTROL MEASURES SEE T.E.S.C. DETAILS SHEET 4.



4 TYPICAL ELECTRICAL VAULT DETAIL
SCALE: NOT TO SCALE



E TYPICAL TRENCH SECTION
SCALE: 1" = 5'



ELECTRICAL CONDUIT ALIGNMENT
SCALE: NOT TO SCALE

NOTE
CONTRACTOR TO PROVIDE ASBUILT SURVEY OF COMPLETED CONDUIT ALIGNMENT TO CITY OF ARLINGTON.

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
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By: _____ Date: _____
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CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON

PIPING DETAILS

PROJECT NO.	1097001.010.015
DATE	1-12-2010
SHEET	10 OF 13
DRAWING NO.	10



LEGEND

- * OSPREY NEST, WOOD DUCK OR OTHER BIRD HOUSE
- * BRUSH PILE
- * ROCK PILE
- * WETLAND SNAG
- * PRE-VEGETATED COIR LOG
- CLEARING AND GRADING LIMITS
- FINAL CONTOURS (1 FT)
- FINAL CONTOURS (5 FT)
- EXISTING CONTOURS (1 FT)
- N.G.P.A. / E. BOUNDARY
- APPROXIMATE LOCATION OF FLOODWAY BOUNDARY (SNOHOMISH COUNTY PERMIT AND ZONING MAP 2009)
- ORDINARY HIGH WATER MARK
- WETLAND/STREAM BUFFER
- RIVER ROCK (ROUNDED COBBLES)
- PROPOSED MOWED GRASS PATH
- PROPOSED GRAVEL PATH
- PROPOSED IMPROVED ROADWAY
- EXISTING GRAVEL ROAD
- EXISTING WETLAND
- EXISTING BUILDING
- HISTORIC PROTECTION AREA

PLANTED AREA

AREA	SF	AC
ASSEMBLAGE 1	97,205	2.23
ASSEMBLAGE 2	78,435	1.80
ASSEMBLAGE 3	67,835	1.56
ASSEMBLAGE 4	113,180	2.60
ALL ASSEMBLAGES	356,655	8.19
TEMPORARY DISTURBANCE (SEED)	72,215	1.66
GRAND TOTAL	428,870	9.85

FEATURE NOTE

* FEATURES TO BE DESIGNED AND / OR INSTALLED AS PART OF A FUTURE VOLUNTEER EFFORT. ADDITIONAL LOG COMPLEXES TO BE INSTALLED IN CELLS 2 & 3 TO PROVIDE HABITAT STRUCTURE AND MAXIMIZE CIRCULATION OF WATER. LOGS TO BE PROVIDED BY CITY AND TO BE INSTALLED AND ANCHORED UNDER SUPERVISION OF CITY NATURAL RESOURCES MANAGER.

PLANTING ASSEMBLAGES (ELEVATIONS IN FT)

SYMBOL	DESIGNATION	CELL 1	CELL 2	CELL 3	CELL 4
	ASSEMBLAGE 1	59-63 (9,730 SQ FT)	60-61 (42,055 SQ FT)	60 (45,420 SQ FT)	NA
	ASSEMBLAGE 2	63-64 (1,890 SQ FT)	61-62 (61,255 SQ FT)	60-61 (15,290 SQ FT)	NA
	ASSEMBLAGE 3	64+ (6,360 SQ FT)	62-63 (18,655 SQ FT)	61-63 (39,590 SQ FT)	61-63 (3,230 SQ FT)
	ASSEMBLAGE 4	(2,800 SQ FT)	63+ (47,070 SQ FT)	63+ (55,010 SQ FT)	63+ (8,300 SQ FT)
(NOT SHOWN)	SEEDING ONLY (ALL TEMPORARY DISTURBANCE AREAS)				

GENERAL NOTES

- ASSEMBLAGES MAY SHIFT SLIGHTLY AT DISCRETION OF THE PROJECT BIOLOGIST. PLANTING WORK MAY BE CONDUCTED BY VOLUNTEERS OR UNDER A SEPARATE CONTRACT, AT THE DISCRETION OF THE CITY.
- INSTALL PLANTS IN FOLLOWING ORDER: 1) COIR LOGS; 2) TREES AND LARGE SHRUBS; 3) SHRUBS AND LIVE CUTTINGS; 4) EMERGENTS AND GROUND COVER (INCLUDING ADDITIONAL SEEDING OF DISTURBED AREAS).
- PLANT SPACING IS AVERAGE ON CENTER. PLANTS SHOULD BE DISTRIBUTED IN A NATURALISTIC PATTERN, WHICH MAY INCLUDE CLUSTERING OF LIKE SPECIES. FINAL LOCATIONS MUST BE APPROVED BY A PROJECT BIOLOGIST PRIOR TO INSTALLATION, BASED ON FINAL TOPOGRAPHY. REFER TO DETAILS AND SPECIFICATIONS.
- LARGE TREES SHOULD BE LOCATED AT LEAST 6 FT FROM THE EDGES OF THE TRAIL.
- LIVE CUTTINGS (STAKES) MUST BE AT LEAST 4 FT LONG SO THAT AT LEAST 2 FT CAN BE BURIED.
- ALL DECIDUOUS TREES AND SHRUBS (EXCLUDING STAKES) TO BE LABELED WITH SPECIES (OR ACRONYM) AND HIGH-VISIBILITY FLAGGING.
- PROTECTIVE PLANT COLLARS ARE TO BE PLACED ON ALL WOODY VEGETATION WITHIN ASSEMBLAGES 3 AND 4.
- COIR LOGS SHALL BE PRE-VEGETATED IF AVAILABLE, OR PLUGS SHALL BE PLANTED INTO THEM AFTER INSTALLATION.
- REFER TO SPECIFICATIONS FOR SPECIAL PLANTING, PROTECTION, AND HANDLING INSTRUCTIONS.
- KEY TO PLANTING DESIGNATIONS:
 ASSEMBLAGE 1 - PERMANENTLY SATURATED, SEASONALLY INUNDATED TO 1.5 FT
 ASSEMBLAGE 2 - PERMANENTLY TO SEASONALLY SATURATED, OCCASIONALLY INUNDATED TO 0.5 FT
 ASSEMBLAGE 3 - SEASONALLY TO PERMANENTLY SATURATED
 ASSEMBLAGE 4 - UPLAND/FLOODPLAIN
- REFER TO SPECIFICATIONS FOR MORE INFORMATION.

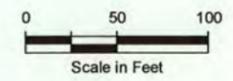
SEEDING NOTES

- PHASE 1: IMMEDIATELY FOLLOWING COMPLETION OF EARTH WORK (WHICH SHALL OCCUR DURING SUMMER), OR WHENEVER SOILS WILL REMAIN UNWORKED FOR SEVEN DAYS, HYDROSEED ALL BARE AREAS WITH "DRY AREA SEED MIX". SEED ON TOP OF ANY COIR LOGS.
- PHASE 2: DURING SEPTEMBER, HYDROSEED "WET AREA SEED MIX" TO AREAS DEDICATED FOR WETLAND PLANT ASSEMBLAGES (ALL AREAS EXCEPT FOR UPLAND/FLOODPLAIN).
- HAND SEED EMERGENT VEGETATION SEED MIX PER PLAN AND TABLE, FOLLOWING RAIN EVENT AND UNDER DIRECTION OF PROJECT BIOLOGIST/ENGINEER. SEED ON COIR LOGS.
- IF UNSURE WHETHER AN AREA IS DRY OR WET, OVERLAP SEED MIXES.
- WHERE JUTE FABRIC IS PRESENT, APPLY A LAYER OF COMPOST 4 INCHES DEEP, FOLLOWED BY SEED WITH A TACKIFIER.

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
APPROVED FOR CONSTRUCTION

By: _____ Date: _____

R/W PERMIT NO. _____ PFN _____



NO.	DATE	REVISIONS	DESIGNED	REVIEWED	APPROVED	STATUS	DRAFT	INITIAL	DATE

DRAFTED BY:	C. NELSON	CEN	1-12-2010
DESIGNED BY:	S. MAXWELL	SM	1-12-2010
REVIEWED BY:	P. WELCH	WPW	1-12-2010
APPROVED BY:			

LANDAU ASSOCIATES
 130 2ND AVENUE S.
 EDMONDS, WASHINGTON 98020
 (425) 778-0907, FAX (425) 778-6409

CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON

PLANTING AND HABITAT PLAN

PROJECT NO.	1097001.010.015
DATE	1-12-2010
SHEET	12 OF 13
DRAWING NO.	12

Assemblage 1 (seasonally inundated emergent wetland to 1.5 ft)					
Species		Form	Specifications		
Scientific Name	Common Name		Quantity	Size / Type	Density **
<i>Salix lucida</i>	Pacific willow	Tree	100	4' stake/ live cutting	8' - plant on hummocks
<i>Salix sitchensis</i>	Sitka willow	Shrub	100	1 gal	
(Wet Area Seed Mix)	(See Seed Mix Table)	Emergent	184 lbs	Seed	100 lbs/ac
(Emergent Seed Mix)	(See Seed Mix Table)	Emergent	184 lbs	Seed	100 lbs/ac

Assemblage 2 (permanently saturated / seasonally inundated scrub-shrub wetland to 0.5 ft)					
Species		Form	Specifications		
Scientific Name	Common Name		Quantity	Size	Density **
<i>Fraxinus latifolia</i>	Oregon ash	Tree	600	4' stake	6'
<i>Salix lucida</i>	Pacific willow	Tree	600	4' stake	6'
<i>Cornus sericea</i>	Red osier dogwood	Shrub	1500	Bare root	6'
<i>Rosa pisocarpa</i>	Swamp rose	Shrub	420	1 gal	4'
<i>Physocarpus capitatus</i>	Pacific ninebark	Shrub	1500	Bare root	4'
<i>Salix sitchensis</i>	Sitka willow	Shrub	1500	4' stake	6'
(Wet Area Seed Mix)	(See Seed Mix Table)	Emergent	175 lbs	Seed	100 lbs/ac
(Emergent Seed Mix)	(See Seed Mix Table)	Emergent	175 lbs	Seed	100 lbs/ac

Assemblage 3 (seasonally saturated forested wetland)					
Species		Form	Specifications		
Scientific Name	Common Name		Quantity	Size	Density **
<i>Malus fusca</i>	Western crabapple	Tree	100	Bare root	6'
<i>Picea sitchensis</i>	Sitka spruce	Tree	100	Bare root	6'
<i>Populus balsamifera</i>	Black cottonwood	Tree	100	Bare root	8'
<i>Alnus rubra</i>	Red Alder	Tree	100	Bare root	6'
<i>Rhamnus purshiana</i>	Cascara	Tree	100	Bare root	6'
<i>Thuja plicata</i>	Western red cedar	Tree	200	Bare root	8'*
<i>Populus tremuloides</i>	Quaking aspen	Tree	100	1 gal	8'
<i>Cornus sericea</i>	Red osier dogwood	Shrub	366	Bare root	4'
<i>Lonicera involucrata</i>	Black twinberry	Shrub	366	Bare root	4'
<i>Physocarpus capitatus</i>	Pacific ninebark	Shrub	366	Bare root	4'
<i>Rosa nutkana</i>	Nootka rose	Shrub	366	1 gal	4'
<i>Rosa pisocarpa</i>	Swamp rose	Shrub	366	1 gal	4'
<i>Rubus spectabilis</i>	Salmonberry	Shrub	366	Bare root	4'
<i>Salix scouleriana</i>	Scouler's willow	Shrub	366	4' stake	4'
<i>Symphoricarpos albus</i>	Snowberry	Shrub	366	1 gal	4'
(Wet Area Seed Mix)	(See Seed Mix Table)	Emergent	80 lbs	Seed	75 lbs/ac
(Dry Area Seed Mix)	(See Seed Mix Table)	Herb.	80 lbs	Seed	75 lbs/ac

Assemblage 4 (upland/floodplain)					
Species		Form	Specifications		
Scientific Name	Common Name		Quantity	Size	Density **
<i>Acer macrophyllum</i>	Big leaf maple	Tree	240	Bare root	8'
<i>Abies grandis</i>	Grand fir	Tree	240	Bare root	8'
<i>Alnus rubra</i>	Red Alder	Tree	240	Bare root	6'
<i>Pseudotsuga menziesii</i>	Douglas fir	Tree	240	Bare root	8'
<i>Tsuga heterophylla</i>	Western hemlock	Tree	240	Bare root	8'*
<i>Acer circinatum</i>	Vine maple	Tree	240	Bare root	6'
<i>Amelanchier alnifolia</i>	Serviceberry	Shrub	600	Bare root	6'
<i>Holodiscus discolor</i>	Oceanspray	Shrub	600	Bare root	6'
<i>Mahonia aquifolium</i>	Tall Oregon grape	Shrub	600	Bare root	5'
<i>Oemleria cerasiformis</i>	Indian plum	Shrub	600	Bare root	5'*
<i>Ribes sanguineum</i>	Red flowering currant	Shrub	600	1 gal	4'
<i>Rosa nutkana</i>	Nootka rose	Shrub	600	1 gal	4'
<i>Symphoricarpos albus</i>	Snowberry	Shrub	600	1 gal	4'
<i>Gaultheria shallon</i>	Salal	Shrub	210	1 gal	4'*
<i>Polystichum munitum</i>	Sword fern	Fern	210	1 gal	4'*
(Dry Area Seed Mix)	(See Seed Mix Table)	Herb.	800 lbs	Seed	100 lbs/ac

* Plant of northeast side of slope or other shading feature (e.g. a bushy shrub/tree, bridge, etc.).

**Density is calculated at rate of 8 feet on center for trees over area, plus 4 feet on center for shrubs over area (average density is 6 feet on center), with the exception of emergent wetland areas.

Seed Mixes			
Species		Percentage of Mix	Location, Rate, Area and Amount
Scientific Name	Common Name		
Dry Area Mix			
<i>Bromus carinatus</i>	California brome	20%	Assemblage 3 - 1.1 ac (75 lb/ac) and Assemblage 4 - 2.1 ac (100 lb/ac) TOTAL 210 lbs
<i>Elymus glaucus</i>	Blue wildrye	20%	
<i>Poa compressa</i>	Canada wildrye	20%	
<i>Festuca rubra commutata</i> OR <i>Festuca longifolia</i>	Chewings fescue OR Hard fescue	20%	
Multiple species (TBD)*	Pacific Northwest wildflower mix	20%	

Wet Area Mix			
<i>Agrostis gigantea</i>	Redtop	20%	Assemblages 1 and 2 - 3.6 ac (100 lb/ac) and Assemblage 3 - 1.1 ac (75 lb/ac) TOTAL 360 lbs
<i>Eleocharis palustris</i>	Common spikerush	20%	
<i>Glyceria occidentalis</i>	Western mannagrass	20%	
<i>Deschampsia caespitosa</i>	Tufted hairgrass	20%	
<i>Agrostis palustris</i>	Creeping bentgrass	20%	

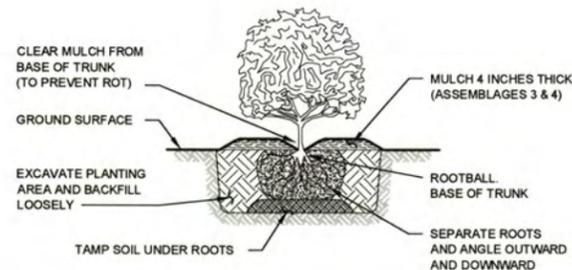
Emergent Mix (Pools and wettest areas)			
<i>Typha latifolia</i>	Common cattail	20%	Estimated to be Assemblages 1 and parts of 2 - 2 ac (100 lb/ac) TOTAL 200 lbs
<i>Scirpus microcarpus</i>	Small-fruited bulrush	20%	
<i>Glyceria occidentalis</i>	Western mannagrass	20%	
<i>Deschampsia caespitosa</i>	Tufted hairgrass	20%	
<i>Scirpus acutus</i>	Hardstem bulrush	20%	

* Species to be selected based on availability at time of planting.

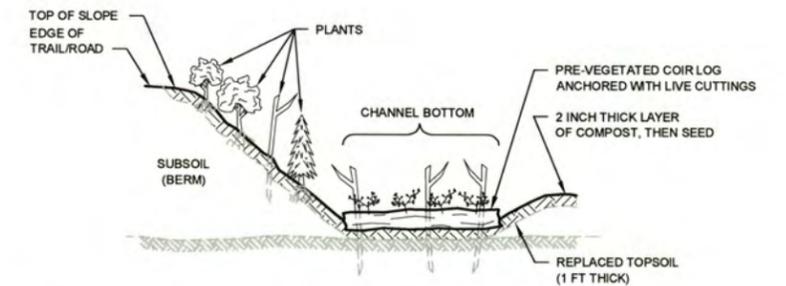
** Locations to be determined based on observation of ponded areas following rain events, under direction of the Project Biologist/Engineer - likely to be Assemblage 1.

Note that Assemblage 3 has an overlapping of both Dry and Wet Area seed mixes.

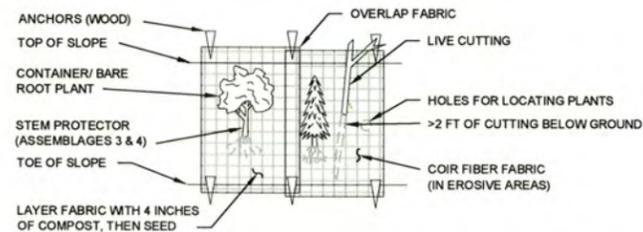
Summary Table	
Size / Type	#
1 Gallon	3,838
Bare Root	9,004
Live Cutting	3,266
Seed	1,678 lbs
Tree	3,540
Shrub	12,358



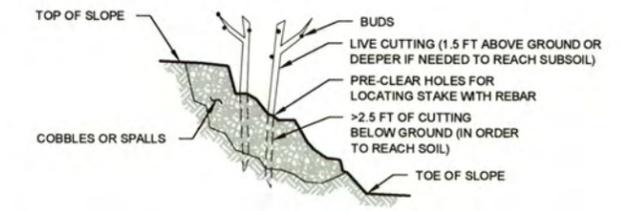
1 CONTAINER/BAREROOT PLANTING
SCALE: NONE



2 SOIL AMENDMENT, COIR LOG INSTALLATION AND PLANTING
SCALE: NONE



3 PLANTS IN COIR FIBER NETTING
SCALE: NONE



4 PLANTING WITH RIP-RAP/COBBLE
SCALE: NONE

SNOHOMISH COUNTY PLANNING AND DEVELOPMENT SERVICES
APPROVED FOR CONSTRUCTION

By: _____ Date: _____

R/W PERMIT NO. _____ PFN _____

NO.	DATE	REVISIONS	DESIGNED	REVIEWED	APPROVED	STATUS	INITIAL	DATE
						DRAFT		

DRAFTED BY:	C. NELSON	CEN	1-12-2010
DESIGNED BY:	S. MAXWELL	SM	1-12-2010
REVIEWED BY:	P. WELCH	WPW	1-12-2010
APPROVED BY:			

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CITY OF ARLINGTON
STORMWATER WETLAND PROJECT
SNOHOMISH COUNTY, WASHINGTON

PROJECT NO.	1097001.010.015
DATE	1-12-2010
SHEET	13 OF 13
DRAWING NO.	13

Calculation No. HM-1 (Excerpt from URS 2008)



1500 Fourth Avenue, Suite 1400
Seattle, Washington 98101
Ph (206) 438 - 2700
Fx (206) 438 - 2699

Client: City of Arlington Project: Stormwater Comprehensive Plan
Location: Arlington, WA Subject: XP-SWMM Hydraulic Model
Project No: 33757537 Calculation No: HM-1 Revision No. 0 Sheet 1 of _____

(Title of Calculations/Document)

STEP 1

	Signature	Date
Methodology & Assumptions Developed	<u></u>	<u>3/20/08</u>
Methodology & Assumptions Approved	<u></u>	<u>3/20/08</u>

STEP 2

Calculations Completed	<u></u>	<u>3/24/08</u>
Calculations Checked	<u></u>	<u>3/25/08</u>

STEP 3

Task Manager Approved _____

Project Manager Approved _____

COMMENTS

Purpose: Evaluate the existing stormwater conveyance system that routes flows from within the Downtown 4th tier basin to the Stillaguamish River.

Background: Initial modeling was conducted using elevation data obtained via GPS surveying provided by the City of Arlington (City). The rim elevations of the manholes were measured via GPS and invert elevations were determined by measuring from the rim elevation to the pipe invert. To improve the accuracy of the elevation data, a second, more accurate survey was conducted using a total station in December 2006. The survey generated new rim elevations and new invert elevations were created based on subtracting the original measure down distances from the new survey data. This stormwater model used the most current total station survey data combined with as-built information for the Olympic stormwater line.

The focus of this evaluation was to locate potential problems within the existing Downtown Basin stormwater system. Stormwater flow routing was completed using XP-SWMM to identify necessary improvements to the stormwater drainage system.

Stormwater hydraulic problems previously identified in 1995, 1999, and 2003 assessment memos include: local flooding along First Street (between McLeod and Lenore Avenues), Lenore Avenue (between First and Third Streets), and along French Avenue (between First and Third Streets); and conveyance limitations within manholes along SR9 (north of Burke Avenue).

Approach: The Downtown Basin was evaluated to determine the capacity of the existing storm drainage network under fully developed conditions. The storm drainage network is not a continuous diameter storm drain; it includes a network of pipes with diameters ranging from 2- to 36-inch pipe diameters.

The location of the Downtown Basin (a.k.a Old Town) within the City is shown in Figure 1 and Figure 2. The model area schematic is shown in Figure 3.

The hydrology was developed based on the 24-hour frequency design storm (See Attachment 1). This evaluation used the rainfall data from the 2-year, 10-year, 25-year, and 100-year 24-hour storms and identified areas where flooding would be expected during similar conditions. Areas with flooding and capacity issues are identified for future stormwater drainage system upgrades. A summary of the rainfall data is presented in Table 1.

The existing storm drainage network was modeled in XP-SWMM using 24-hour frequency design storm hydrology and fully developed conditions. Hydraulics were routed for each event using XP-SWMM. Input to the model network for the Downtown Basin is included in Attachment 2. This information includes assumed drainage area and invert elevations of storage nodes and links.

Areas of flooding observed by model results under these design conditions were identified. Several nodes throughout the drainage network were surcharged during the peak flood conditions.

Results: Attachment 2 presents selected XP-SWMM model results. Based on the assumptions made for pipe geometry and precipitation, described previously, the existing network was modeled under fully developed conditions.

The model results identified surcharging problems in the following locations:

- The storm drain along Lenore Avenue, near East 2nd Street (location BE-09);
- The storm drain along First Street, between Gifford and Lenore Avenues (locations: BE-10 and BE-11);
- Trunk line along SR-9 near Burke Avenue (location: DM-05);

- The storm drain along West Division Street, near North Dunham Avenue(location: DM-17);
- The storm drain along S West Avenue , south of E 3rd Street (location: W-09); and
- The storm drain along S Olympic Avenue, north of Maple Street (location: OL-22).

See Table 2 for a summary of flooding conditions within the Downtown Basin.

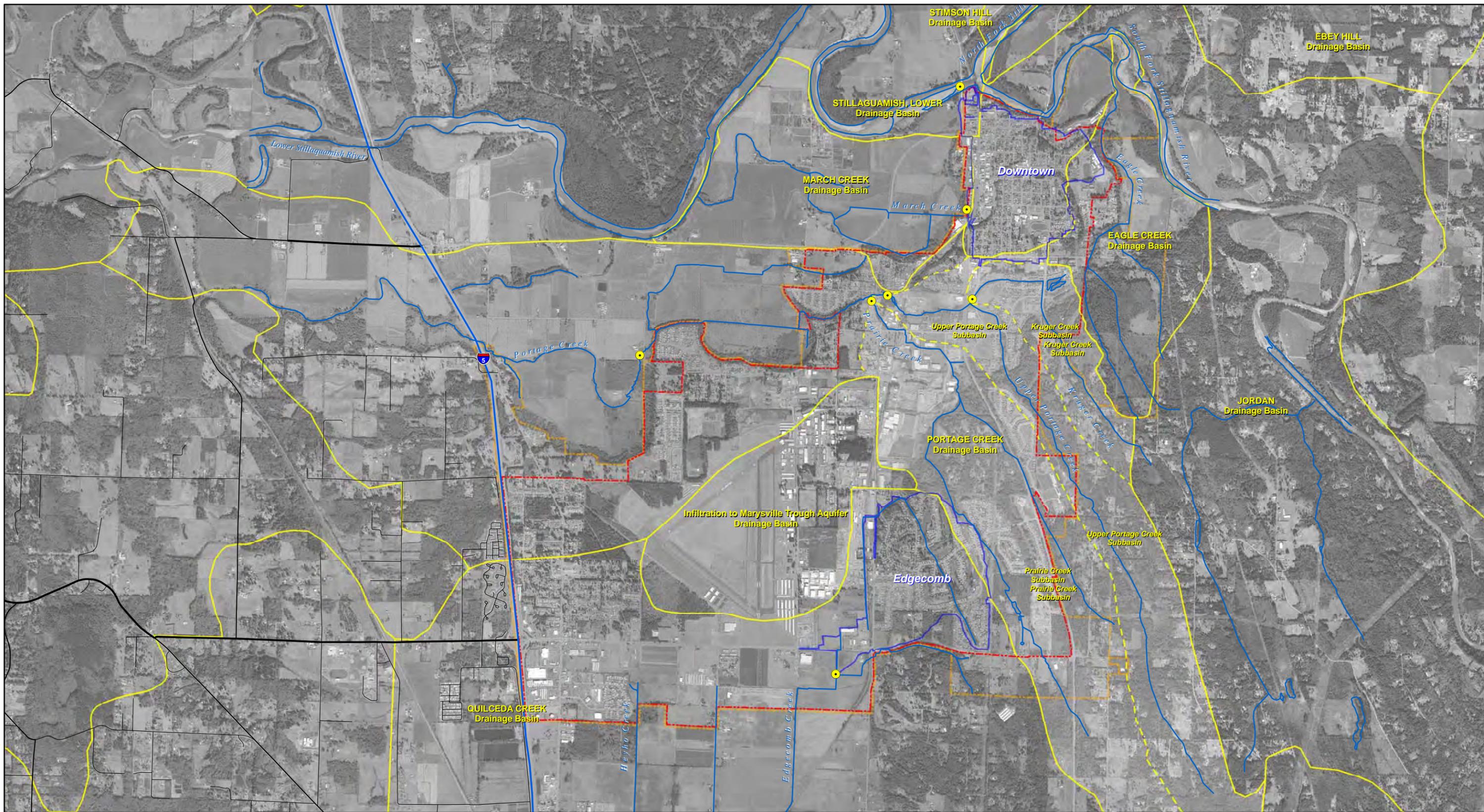
Surcharging issues at BE-09, BE-10, BE-11, and DM-05 has already been identified as problems in the 1995, 1999, and 2003 assessment reports and the City has planned projects to resolve these issues. The problems at BE-09, BE-10, and BE-11 should be resolved during the completion of Projects No. 11 and 201. The problem at DM-05 should be resolved during the completion of Project No. 12. See Attachment 3 for a brief description of these projects.

The model indicated flooding issues at DM-17 during the 10-year storm event, which is a significant event. Downstream of the DM-17 manhole, the slope becomes flatter and the pipe decreases from a 24-inch to a 21-inch diameter. These two reductions in the stormwater conveyance performance potential triggered the model to detect flooding during the 10-year storm. It is recommended that the City monitor and record observations in this area during and following storm events to determine if additional modeling, and potentially upgrading the stormwater system, at this location is necessary.

The model indicated a problem at W-09, but it is believed that these results occurred due to incorrect elevation data. According to the model, W-09 will flood during the 2-year, 24-hour storm event. A profile of the storm drain line near W-09 indicates an unusual low point at this structure. The model estimated that OL-22 will flood during the 10-year storm event. The conduits downstream of OL-22 have the same diameter and approximately the same slope, but do not produce any surcharging issues. It appears that the estimated flooding at OL-22 is a program error. Attachment 2 shows profiles of the storm drain line at W-09 and OL-22. Since the data for these two structures is believed to be flawed, upgrades are not recommended for these two areas.

Return Period (years)	Rainfall Depth (inches)	Return Period (years)	Rainfall Depth (inches)
2-year	1.80	25-year	3.20
10-year	2.75	100-year	3.75

Node	100-year Storm			25-year Storm			10-year Storm			2-year Storm		
	Flooded	Flood Volume		Flooded	Flood Volume		Flooded	Flood Volume		Flooded	Flood Volume	
	Time (min)	(cf)	(ac-ft)	Time (min)	(cf)	(ac-ft)	Time (min)	(cf)	(ac-ft)	Time (min)	(cf)	(ac-ft)
DM-05	127	169,408	3.89	96	128,582	2.95	74	98,538	2.26	40	30,966	0.71
DM-17	38	10,460	0.24	30	8,000	0.18	24	4,687	0.11	--	--	--
BE-09	50	15,207	0.35	42	9,756	0.22	35	5,769	0.13	9	145	0
BE-10	45	7,229	0.17	37	5,909	0.14	30	4,000	0.09	--	--	--
BE-11	26	1,872	0.04	19	708	0.02	--	--	--	--	--	--



- Legend**
- Drainage Basin Outfalls
 - State Highway
 - Arterial
 - Street
 - ▭ Arlington City Limits
 - ▭ UGA Zones
 - ▭ Drainage Basins
 - ▭ Subbasins

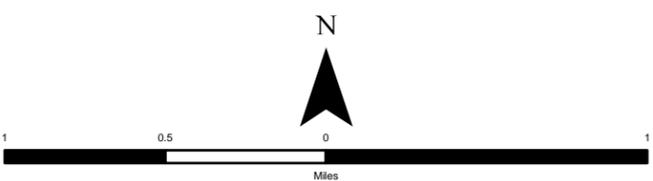


Figure 1
Arlington Drainage Basins and Subbasins



Figure 2

Old Town Arlington

0 125 250 500
Feet

Legend

Pipe_network_0401

Material

- Unspecified
- CMP Cor Metal
- Concrete
- Ductal Iron
- HDPE Corrugated
- HDPE Smooth
- PVC Smooth
- Perforated HDPE
- Perforated PVC
- Steel
- Contour_20
- Contour_10

Watersheds WS4

WS4

- - - Haller Park
- - - Seventh Day Adventist
- - - Talcott
- - - Utilities
- - - West
- - - West RR
- - - Butler DT
- - - Broadway
- - - Butler East
- - - Butler West
- - - Centennial Trail
- - - Division Main
- - - Edgecomb

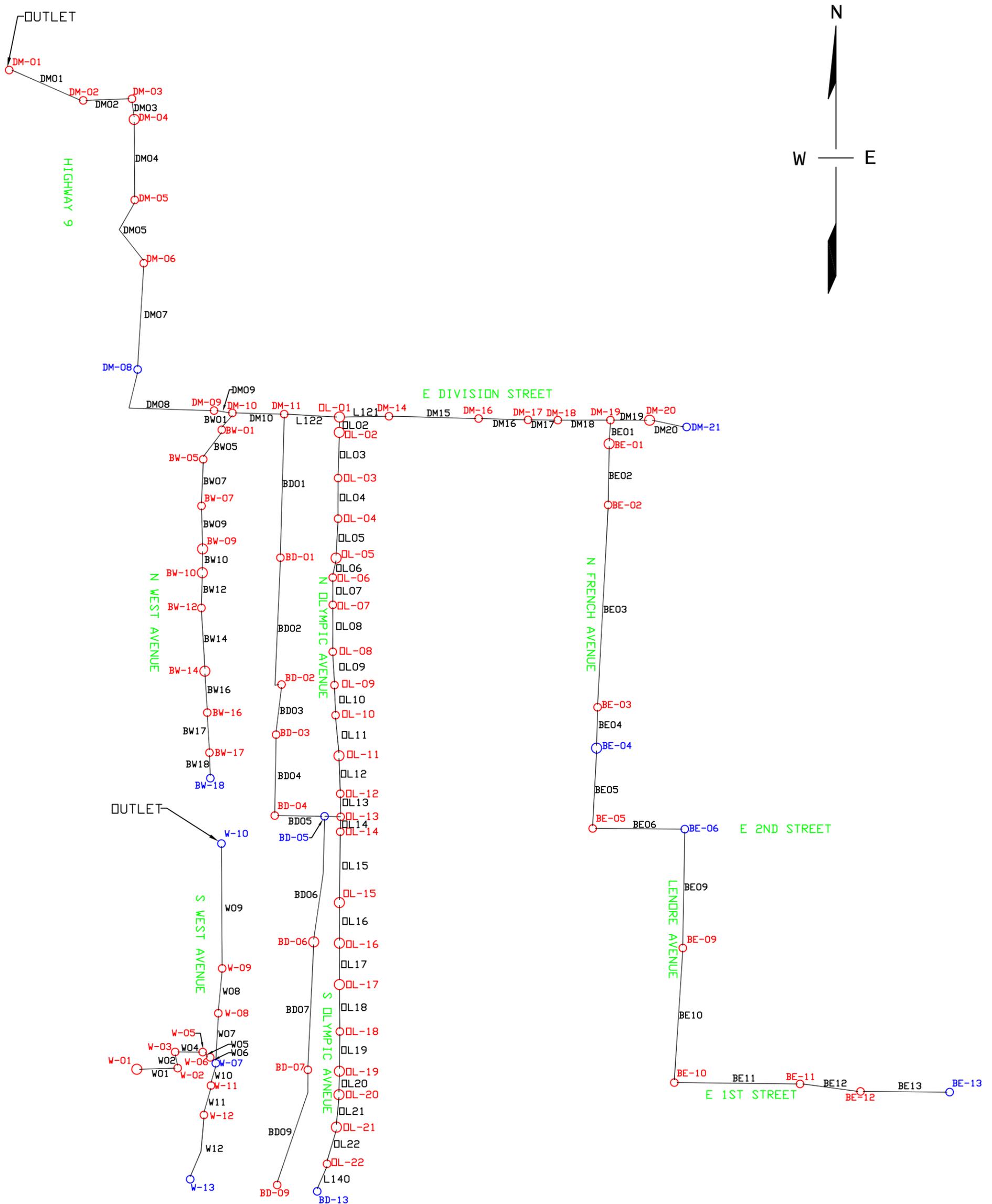


FIGURE 3
 OLD TOWN BASIN STORMWATER SYSTEM SCHEMATIC

- LEGEND:**
- X-# CONVEYANCE STRUCTURE WITHOUT INLET
 - X-# CONVEYANCE STRUCTURE WITH INLET
 - X# PIPELINE

ATTACHMENT 1
Isopluvials of 24-Hour Precipitation

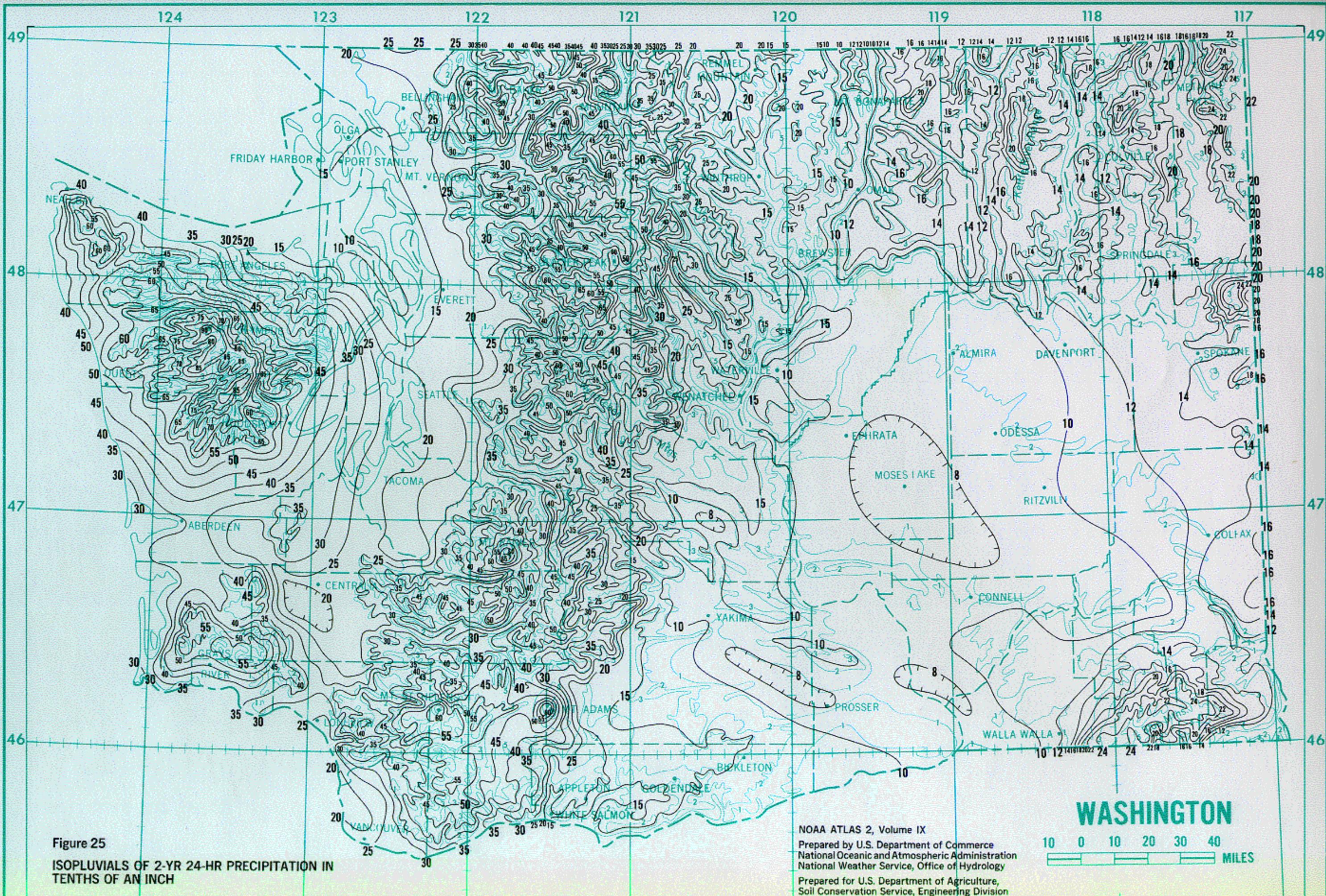


Figure 25
 ISOPLUVIALS OF 2-YR 24-HR PRECIPITATION IN
 TENTHS OF AN INCH

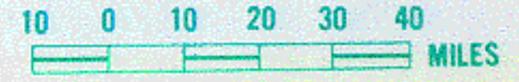
NOAA ATLAS 2, Volume IX
 Prepared by U.S. Department of Commerce
 National Oceanic and Atmospheric Administration
 National Weather Service, Office of Hydrology
 Prepared for U.S. Department of Agriculture,
 Soil Conservation Service, Engineering Division

10 0 10 20 30 40
 MILES



Figure 27
 ISOPLUVIALS OF 10-YR 24-HR PRECIPITATION
 IN TENTHS OF AN INCH

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 Prepared by U.S. Department of Commerce
 National Oceanic and Atmospheric Administration
 National Weather Service, Office of Hydrology
 Prepared for U.S. Department of Agriculture,
 Soil Conservation Service, Engineering Division



WASHINGTON

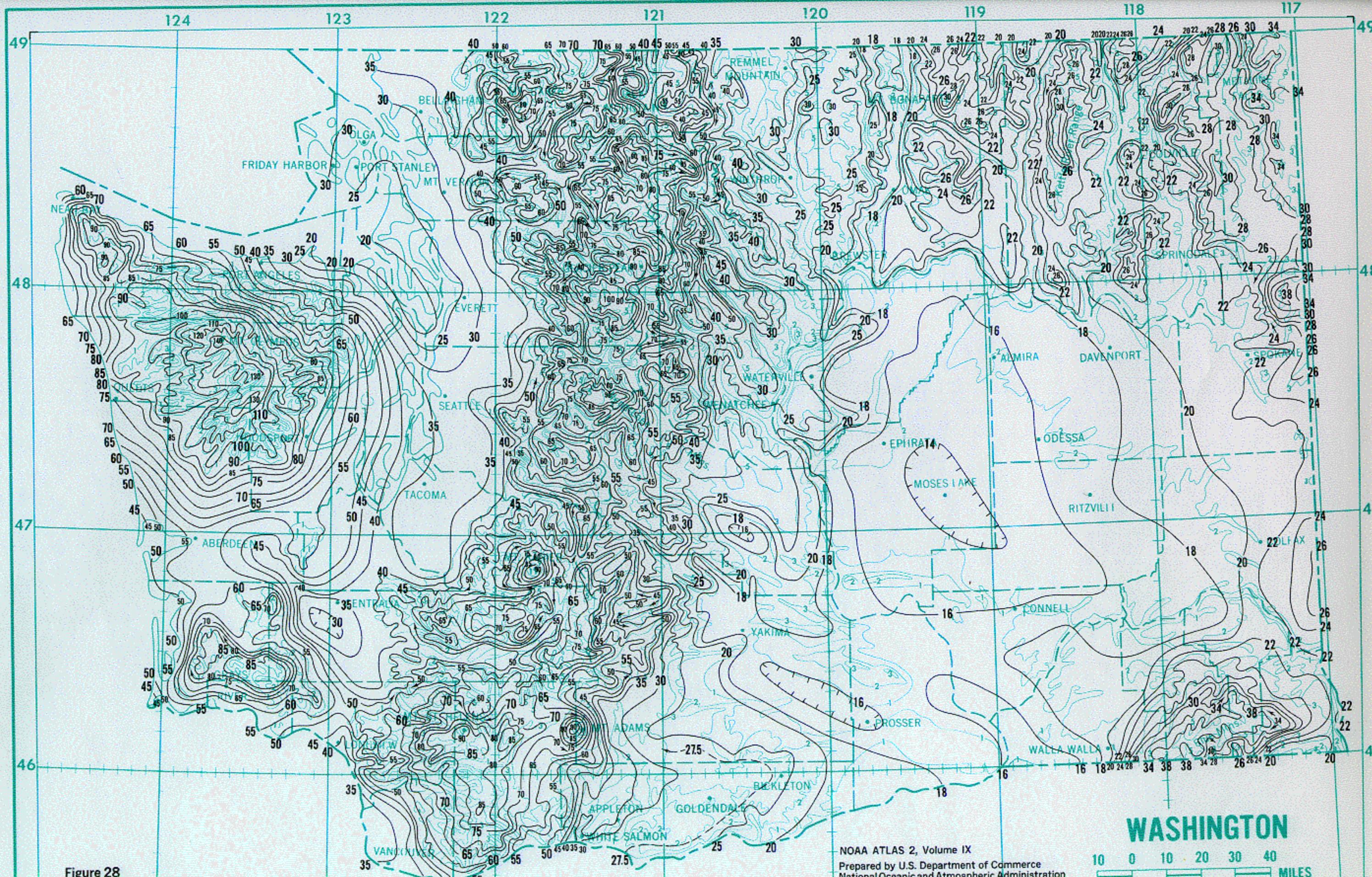


Figure 28
 ISOPLUVIALS OF 25-YR 24-HR PRECIPITATION
 IN TENTHS OF AN INCH

NOAA ATLAS 2, Volume IX
 Prepared by U.S. Department of Commerce
 National Oceanic and Atmospheric Administration
 National Weather Service, Office of Hydrology
 Prepared for U.S. Department of Agriculture,
 Soil Conservation Service, Engineering Division

WASHINGTON
 10 0 10 20 30 40
 MILES

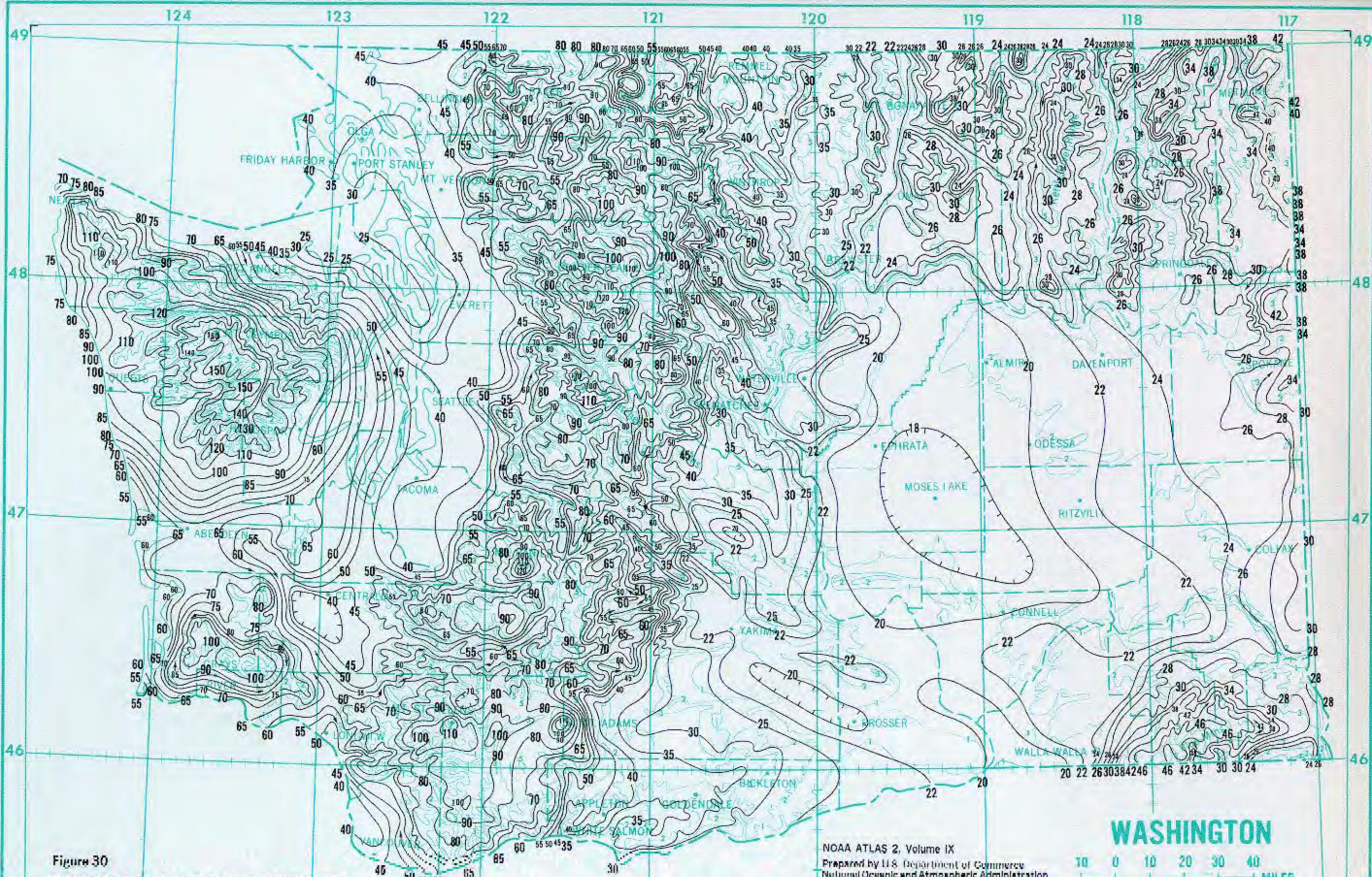


Figure 30
ISOPLUVIALS OF 100-YR 24-HR PRECIPITATION
IN TENTHS OF AN INCH

NOAA ATLAS 2, Volume IX
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National Oceanic and Atmospheric Administration
National Weather Service, Office of Hydrology
Prepared for U.S. Department of Agriculture,
Soil Conservation Service, Engineering Division



ATTACHMENT 2
Summary of XP-SWMM Input & Output

Downtown Basin - Summary of Impervious Areas

Node Name	Rainfall Reference	Width (ft)	Area (ac)	Impervious Percentage %	Slope
BW-18	SCS Type IA	1,140	5.7	65	0.004
BD-13	SCS Type IA	2,920	122.3	53.4	0.022
OL-13	SCS Type IA	1,850	30.5	53.4	0.048
BE-13	SCS Type IA	1,560	24.2	51.1	0.073
BE-06	SCS Type IA	1,500	34.8	51.1	0.019
BE-04	SCS Type IA	1,340	19.3	51.1	0.033
DM-21a	SCS Type IA	1,800	16.1	64.9	0.021
DM-21b	SCS Type IA	870	8.9	64.9	0.021
DM-08	SCS Type IA	1,260	19.9	64.9	0.029
W-13	SCS Type IA	180	1.7	66.8	0.032
W-10	SCS Type IA	840	2.9	66.8	0.015
W-07	SCS Type IA	490	7.6	65	0.003

2-Year Storm – Selected XP-SWMM Result

 | Table E1 - Conduit Data |

Inp Num	Conduit Name	Length (ft)	Conduit Class	Area (ft^2)	Manning Coef.	Max Width (ft)	Depth (ft)	Trapezoid Side Slopes
1	DM01	285.2000	Circular	7.0686	0.0240	3.0000	3.0000	
2	DM07	367.5000	Circular	7.0686	0.0140	3.0000	3.0000	
3	DM05	245.1000	Circular	7.0686	0.0140	3.0000	3.0000	
4	DM04	279.4000	Circular	19.6350	0.0240	5.0000	5.0000	
5	DM02	161.8000	Circular	7.0686	0.0240	3.0000	3.0000	
6	DM08	427.6100	Circular	7.0686	0.0140	3.0000	3.0000	
7	BW18	87.5000	Circular	1.0751	0.0120	1.1700	1.1700	
8	BW17	141.8000	Circular	1.0751	0.0120	1.1700	1.1700	
9	BW14	216.1000	Circular	1.0751	0.0120	1.1700	1.1700	
10	BW12	125.2000	Circular	1.0751	0.0120	1.1700	1.1700	
11	BW10	81.6000	Circular	1.0751	0.0120	1.1700	1.1700	
12	BW07	170.6000	Circular	1.0751	0.0120	1.1700	1.1700	
13	BW05	122.0000	Circular	1.3893	0.0120	1.3300	1.3300	
14	BW01	67.3000	Circular	1.7671	0.0120	1.5000	1.5000	
15	DM09	66.9000	Circular	7.0686	0.0140	3.0000	3.0000	
16	BE01	62.1000	Circular	2.4053	0.0140	1.7500	1.7500	
17	DM20	130.4000	Circular	1.2272	0.0140	1.2500	1.2500	
18	DM19	138.1000	Circular	0.7854	0.0140	1.0000	1.0000	
19	DM17	143.4000	Circular	3.1416	0.0140	2.0000	2.0000	
20	DM15	310.9000	Circular	2.4053	0.0140	1.7500	1.7500	
21	DM10	181.3000	Circular	7.0686	0.0140	3.0000	3.0000	
22	BD02	465.0000	Circular	6.2902	0.0140	2.8300	2.8300	
23	BD01	503.0000	Circular	7.0686	0.0140	3.0000	3.0000	
24	BD09	419.2000	Circular	3.1416	0.0140	2.0000	2.0000	
25	BD07	442.3000	Circular	3.1416	0.0140	2.0000	2.0000	
26	BD06	443.7000	Circular	3.9761	0.0140	2.2500	2.2500	
27	BD05	178.3000	Circular	3.9761	0.0140	2.2500	2.2500	
28	BD03	166.9000	Circular	4.9087	0.0140	2.5000	2.5000	
29	BE10	459.5000	Circular	1.7671	0.0140	1.5000	1.5000	
30	BE09	423.0000	Circular	2.4053	0.0140	1.7500	1.7500	
31	BE06	317.8000	Circular	1.7671	0.0140	1.5000	1.5000	
32	W09	434.8000	Circular	0.7854	0.0120	1.0000	1.0000	
33	W06	32.9000	Circular	0.7854	0.0120	1.0000	1.0000	
34	W04	99.2000	Circular	0.7854	0.0120	1.0000	1.0000	
35	W02	52.3000	Circular	0.7854	0.0120	1.0000	1.0000	
36	W01	138.1000	Circular	1.7671	0.0120	1.5000	1.5000	
37	W12	228.1000	Circular	0.7854	0.0120	1.0000	1.0000	
38	DM03	80.4000	Circular	4.9087	0.0240	2.5000	2.5000	
39	BW16	142.7000	Circular	1.0751	0.0120	1.1700	1.1700	
40	BW09	142.6000	Circular	1.0207	0.0120	1.1400	1.1400	
41	BD04	278.9000	Circular	3.9761	0.0140	2.2500	2.2500	
42	DM16	170.0000	Circular	3.1416	0.0140	2.0000	2.0000	
43	DM18	144.1000	Circular	3.1416	0.0140	2.0000	2.0000	
44	BE02	238.6000	Circular	2.4053	0.0140	1.7500	1.7500	
45	BE05	268.7000	Circular	2.4053	0.0140	1.7500	1.7500	
46	BE04	155.1000	Circular	2.4053	0.0140	1.7500	1.7500	
47	BE03	698.2000	Circular	2.4053	0.0140	1.7500	1.7500	
48	BE13	320.4000	Circular	1.2272	0.0140	1.2500	1.2500	
49	BE12	189.6000	Circular	1.2272	0.0140	1.2500	1.2500	
50	BE11	445.7000	Circular	1.7671	0.0140	1.5000	1.5000	
51	W11	111.1000	Circular	0.7854	0.0120	1.0000	1.0000	
52	W10	71.5000	Circular	0.7854	0.0120	1.0000	1.0000	
53	W08	160.3000	Circular	0.7854	0.0120	1.0000	1.0000	
54	W07	180.7000	Circular	0.7854	0.0120	1.0000	1.0000	
55	W05	26.1000	Circular	0.7854	0.0120	1.0000	1.0000	
56	OL05	149.0000	Circular	7.0686	0.0140	3.0000	3.0000	
57	OL04	120.0000	Circular	7.0686	0.0140	3.0000	3.0000	
58	OL03	149.0000	Circular	7.0686	0.0140	3.0000	3.0000	
59	OL02	69.0000	Circular	7.0686	0.0140	3.0000	3.0000	
60	L121	183.0500	Circular	7.0686	0.0140	3.0000	3.0000	
61	L122	183.0500	Circular	7.0686	0.0140	3.0000	3.0000	
62	OL09	77.0000	Circular	7.0686	0.0140	3.0000	3.0000	
63	OL08	149.0000	Circular	7.0686	0.0140	3.0000	3.0000	
64	OL07	145.0000	Circular	7.0686	0.0140	3.0000	3.0000	
65	OL06	71.0000	Circular	7.0686	0.0140	3.0000	3.0000	
66	OL13	57.0000	Circular	7.0686	0.0140	3.0000	3.0000	
67	OL12	124.0000	Circular	7.0686	0.0140	3.0000	3.0000	
68	OL11	108.0000	Circular	7.0686	0.0140	3.0000	3.0000	
69	OL10	122.0000	Circular	7.0686	0.0140	3.0000	3.0000	
70	OL14	49.0000	Circular	7.0686	0.0140	3.0000	3.0000	
71	OL16	91.0000	Circular	7.0686	0.0140	3.0000	3.0000	
72	OL15	267.0000	Circular	7.0686	0.0140	3.0000	3.0000	
73	OL19	49.0000	Circular	7.0686	0.0140	3.0000	3.0000	
74	OL18	163.0000	Circular	7.0686	0.0140	3.0000	3.0000	
75	OL17	212.0000	Circular	7.0686	0.0140	3.0000	3.0000	
76	OL20	91.0000	Circular	7.0686	0.0140	3.0000	3.0000	
77	OL22	113.0000	Circular	7.0686	0.0140	3.0000	3.0000	
78	OL21	146.0000	Circular	7.0686	0.0140	3.0000	3.0000	
79	L140	73.0000	Circular	7.0686	0.0140	3.0000	3.0000	
80	Overflow	200.0000	Circular	0.7854	0.0140	1.0000	1.0000	
Total length of all conduits		15600.7100 feet						

2-Year Storm – Selected XP-SWMM Result

 | Table E3a - Junction Data |

Inp Num	Junction Name	Ground Elevation	Crown Elevation	Invert Elevation	Qinst cfs	Initial Depth-ft	Interface Flow (%)
1	DM-02	65.4400	58.1200	55.1200	0.0000	0.0000	100.0000
2	DM-01	57.4200	54.7000	51.7000	0.0000	0.0000	100.0000
3	DM-03	67.6900	59.0200	56.0200	0.0000	0.0000	100.0000
4	DM-05	70.0000	63.9000	58.9000	0.0000	0.0000	100.0000
5	DM-06	97.5200	66.1000	63.1000	0.0000	0.0000	100.0000
6	DM-08	98.8200	88.6800	85.6800	0.0000	0.0000	100.0000
7	DM-09	103.1100	93.5000	90.5000	0.0000	0.0000	100.0000
8	DM-10	103.2100	94.7900	91.7900	0.0000	0.0000	100.0000
9	BW-01	103.2300	99.5100	98.0100	0.0000	0.0000	100.0000
10	BW-05	103.1700	100.2400	98.9100	0.0000	0.0000	100.0000
11	BW-07	104.4600	102.0900	100.9200	0.0000	0.0000	100.0000
12	BW-10	107.8900	104.0700	102.9000	0.0000	0.0000	100.0000
13	BW-12	111.0900	104.8900	103.7200	0.0000	0.0000	100.0000
14	BW-14	111.8500	105.9300	104.7600	0.0000	0.0000	100.0000
15	BW-17	110.4500	107.3400	106.1700	0.0000	0.0000	100.0000
16	BW-18	110.7700	107.9300	106.7600	0.0000	0.0000	100.0000
17	DM-11	101.3000	96.0700	93.0700	0.0000	0.0000	100.0000
18	DM-14	111.0800	99.6600	94.7000	0.0000	0.0000	100.0000
19	DM-17	113.2000	107.0300	105.0300	0.0000	0.0000	100.0000
20	DM-19	124.9500	122.1600	120.1600	0.0000	0.0000	100.0000
21	BE-05	168.2000	159.2400	157.4900	0.0000	0.0000	100.0000
22	DM-20	137.2200	130.7500	129.5000	0.0000	0.0000	100.0000
23	DM-21	139.6700	132.6000	131.3500	0.0000	0.0000	100.0000
24	BD-01	108.3790	101.1400	96.8200	0.0000	0.0000	100.0000
25	BD-02	111.3980	104.3000	101.4700	0.0000	0.0000	100.0000
26	BD-03	115.0550	105.3300	102.8300	0.0000	0.0000	100.0000
27	BD-05	110.9320	107.3900	103.7700	0.0000	0.0000	100.0000
28	BD-06	114.3850	107.2700	105.0100	0.0000	0.0000	100.0000
29	BD-07	115.1780	108.5600	106.5400	0.0000	0.0000	100.0000
30	BD-09	115.6970	109.1600	107.1000	0.0000	0.0000	100.0000
31	BD-13	114.5000	110.6700	107.6700	0.0000	0.0000	100.0000
32	BE-06	170.7200	163.9600	161.4600	0.0000	0.0000	100.0000
33	BE-09	166.9400	164.6600	162.9100	0.0000	0.0000	100.0000
34	BE-13	193.8900	189.8600	188.6100	0.0000	0.0000	100.0000
35	W-10	110.9800	108.7200	107.7200	0.0000	0.0000	100.0000
36	W-07	110.6480	101.4000	99.1500	0.0000	0.0000	100.0000
37	W-13	120.7010	116.3000	115.3000	0.0000	0.0000	100.0000
38	W-05	109.5400	100.2800	99.2300	0.0000	0.0000	100.0000
39	W-03	107.5690	101.1000	100.0600	0.0000	0.0000	100.0000
40	W-02	113.4460	103.8600	102.3600	0.0000	0.0000	100.0000
41	W-01	93.1500	93.1500	91.6500	0.0000	0.0000	100.0000
42	DM-04	68.4500	61.6000	56.5600	0.0000	0.0000	100.0000
43	BW-16	111.0800	106.7700	105.6000	0.0000	0.0000	100.0000
44	BW-09	105.7900	103.0600	101.8900	0.0000	0.0000	100.0000
45	BD-04	113.4460	105.7200	103.4700	0.0000	0.0000	100.0000
46	DM-16	112.2600	103.2600	101.2600	0.0000	0.0000	100.0000
47	DM-18	116.7400	112.1400	110.1400	0.0000	0.0000	100.0000
48	BE-01	140.4400	136.2700	134.5200	0.0000	0.0000	100.0000
49	BE-02	142.1700	138.6300	136.8800	0.0000	0.0000	100.0000
50	BE-03	164.3700	157.8700	156.1200	0.0000	0.0000	100.0000
51	BE-04	167.2500	158.5800	156.8300	0.0000	0.0000	100.0000
52	BE-10	170.2400	166.4200	164.9200	0.0000	0.0000	100.0000
53	BE-11	178.9700	172.9500	171.4500	0.0000	0.0000	100.0000
54	BE-12	183.9400	179.6200	178.3700	0.0000	0.0000	100.0000
55	W-12	108.9280	103.7500	102.7100	0.0000	0.0000	100.0000
56	W-11	111.1880	103.9400	102.5800	0.0000	0.0000	100.0000
57	W-09	108.4470	106.2800	105.2600	0.0000	0.0000	100.0000
58	W-08	112.3300	109.2600	108.1200	0.0000	0.0000	100.0000
59	W-06	110.0000	100.2500	99.2500	0.0000	0.0000	100.0000
60	OL-02	109.8200	105.5800	102.5800	0.0000	0.0000	100.0000
61	OL-03	109.8300	105.8900	102.8900	0.0000	0.0000	100.0000
62	OL-04	111.4300	106.1400	103.1400	0.0000	0.0000	100.0000
63	OL-05	114.5700	106.4900	103.4900	0.0000	0.0000	100.0000
64	OL-01	109.8300	105.4300	93.9300	0.0000	0.0000	100.0000
65	OL-09	114.9800	107.3200	104.3200	0.0000	0.0000	100.0000
66	OL-08	114.8000	107.1700	104.1700	0.0000	0.0000	100.0000
67	OL-07	114.7000	106.8700	103.8700	0.0000	0.0000	100.0000
68	OL-06	114.8100	106.5800	103.5800	0.0000	0.0000	100.0000
69	OL-13	114.6700	108.1500	105.1500	0.0000	0.0000	100.0000
70	OL-12	114.8700	108.0300	105.0300	0.0000	0.0000	100.0000
71	OL-11	114.7400	107.7800	104.7800	0.0000	0.0000	100.0000
72	OL-10	114.5700	107.5600	104.5600	0.0000	0.0000	100.0000
73	OL-14	114.5100	108.2400	105.2400	0.0000	0.0000	100.0000
74	OL-15	114.4300	108.7700	105.7700	0.0000	0.0000	100.0000
75	OL-16	114.6700	108.9500	105.9500	0.0000	0.0000	100.0000
76	OL-17	114.4500	109.3800	106.3800	0.0000	0.0000	100.0000
77	OL-18	114.4400	109.7000	106.7000	0.0000	0.0000	100.0000
78	OL-19	114.9000	109.8000	106.8000	0.0000	0.0000	100.0000
79	OL-20	114.3400	109.9700	106.9700	0.0000	0.0000	100.0000
80	OL-21	114.2400	110.2700	107.2700	0.0000	0.0000	100.0000
81	OL-22	113.7600	110.5000	107.5000	0.0000	0.0000	100.0000

2-Year Storm – Selected XP-SWMM Result

 | Table E4 - Conduit Connectivity |

Input Number	Conduit Name	Upstream Node	Downstream Node	Upstream Elevation	Downstream Elevation		
1	DM01	DM-02	DM-01	55.1200	51.7000	No	Design
2	DM07	DM-08	DM-06	85.6800	63.1000	No	Design
3	DM05	DM-06	DM-05	63.1000	58.9000	No	Design
4	DM04	DM-05	DM-04	58.9000	56.6000	No	Design
5	DM02	DM-03	DM-02	56.0200	55.1200	No	Design
6	DM08	DM-09	DM-08	90.5000	85.6800	No	Design
7	BW18	BW-18	BW-17	106.7600	106.1700	No	Design
8	BW17	BW-17	BW-16	106.1700	105.6000	No	Design
9	BW14	BW-14	BW-12	104.7600	103.7200	No	Design
10	BW12	BW-12	BW-10	103.7200	102.9000	No	Design
11	BW10	BW-10	BW-09	102.9000	101.8900	No	Design
12	BW07	BW-07	BW-05	100.9200	98.9100	No	Design
13	BW05	BW-05	BW-01	98.9100	98.0100	No	Design
14	BW01	BW-01	DM-10	98.0100	91.7900	No	Design
15	DM09	DM-10	DM-09	91.7900	90.5000	No	Design
16	BE01	BE-01	DM-19	134.5200	120.1600	No	Design
17	DM20	DM-21	DM-20	131.3500	129.5000	No	Design
18	DM19	DM-20	DM-19	129.5000	120.1600	No	Design
19	DM17	DM-18	DM-17	110.1400	105.0300	No	Design
20	DM15	DM-16	DM-14	101.2600	97.9100	No	Design
21	DM10	DM-11	DM-10	93.0700	91.7900	No	Design
22	BD02	BD-02	BD-01	101.4700	98.3100	No	Design
23	BD01	BD-01	DM-11	96.8200	93.0700	No	Design
24	BD09	BD-09	BD-07	107.1600	106.5400	No	Design
25	BD07	BD-07	BD-06	106.5600	105.2700	No	Design
26	BD06	BD-06	BD-05	105.0100	103.7700	No	Design
27	BD05	BD-05	BD-04	103.7700	103.4700	No	Design
28	BD03	BD-03	BD-02	102.8300	101.4700	No	Design
29	BE10	BE-10	BE-09	164.9200	162.9100	No	Design
30	BE09	BE-09	BE-06	162.9100	161.4600	No	Design
31	BE06	BE-06	BE-05	162.4600	157.4900	No	Design
32	W09	W-10	W-09	107.7200	105.2800	No	Design
33	W06	W-06	W-07	99.2500	99.1500	No	Design
34	W04	W-03	W-05	100.1000	99.2300	No	Design
35	W02	W-02	W-03	102.8000	100.0700	No	Design
36	W01	W-02	W-01	102.3600	91.6500	No	Design
37	W12	W-13	W-12	115.3000	102.7100	No	Design
38	DM03	DM-04	DM-03	56.5600	56.0200	No	Design
39	BW16	BW-16	BW-14	105.6000	104.7600	No	Design
40	BW09	BW-09	BW-07	101.8900	100.9200	No	Design
41	BD04	BD-04	BD-03	103.4700	102.8300	No	Design
42	DM16	DM-17	DM-16	105.0300	101.2600	No	Design
43	DM18	DM-19	DM-18	120.1600	110.1400	No	Design
44	BE02	BE-02	BE-01	136.8800	134.5200	No	Design
45	BE05	BE-05	BE-04	157.4900	156.8300	No	Design
46	BE04	BE-04	BE-03	156.8300	156.1200	No	Design
47	BE03	BE-03	BE-02	156.1200	136.8800	No	Design
48	BE13	BE-13	BE-12	188.6100	178.3700	No	Design
49	BE12	BE-12	BE-11	178.3700	171.4500	No	Design
50	BE11	BE-11	BE-10	171.4500	164.9200	No	Design
51	W11	W-11	W-12	102.9400	102.7500	No	Design
52	W10	W-11	W-07	102.5800	100.2900	No	Design
53	W08	W-08	W-09	108.2600	105.2600	No	Design
54	W07	W-08	W-07	108.1200	100.4000	No	Design
55	W05	W-05	W-06	99.2800	99.2500	No	Design
56	OL05	OL-05	OL-04	103.4900	103.1400	No	Design
57	OL04	OL-04	OL-03	103.1400	102.8900	No	Design
58	OL03	OL-03	OL-02	102.8900	102.5800	No	Design
59	OL02	OL-02	OL-01	102.5800	102.4300	No	Design
60	L121	DM-14	OL-01	94.7000	93.9300	No	Design
61	L122	OL-01	DM-11	93.9300	93.0700	No	Design
62	OL09	OL-09	OL-08	104.3200	104.1700	No	Design
63	OL08	OL-08	OL-07	104.1700	103.8700	No	Design
64	OL07	OL-07	OL-06	103.8700	103.5800	No	Design
65	OL06	OL-06	OL-05	103.5800	103.4900	No	Design
66	OL13	OL-13	OL-12	105.1500	105.0300	No	Design
67	OL12	OL-12	OL-11	105.0300	104.7800	No	Design
68	OL11	OL-11	OL-10	104.7800	104.5600	No	Design
69	OL10	OL-10	OL-09	104.5600	104.3200	No	Design
70	OL14	OL-14	OL-13	105.2400	105.1500	No	Design
71	OL16	OL-16	OL-15	105.9500	105.7700	No	Design
72	OL15	OL-15	OL-14	105.7700	105.2400	No	Design
73	OL19	OL-19	OL-18	106.8000	106.7000	No	Design
74	OL18	OL-18	OL-17	106.7000	106.3800	No	Design
75	OL17	OL-17	OL-16	106.3800	105.9500	No	Design
76	OL20	OL-20	OL-19	106.9700	106.8000	No	Design
77	OL22	OL-22	OL-21	107.5000	107.2700	No	Design
78	OL21	OL-21	OL-20	107.2700	106.9700	No	Design
79	L140	BD-13	OL-22	107.6700	107.5000	No	Design
80	Overflow	OL-13	BD-05	106.8000	106.3900	No	Design

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| Conduit Convergence Criteria |

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Conduit Name	Full Flow	Conduit Slope
DM01	39.5627	0.0120
DM07	153.5196	0.0614
DM05	81.0744	0.0171
DM04	127.9955	0.0082
DM02	26.9451	0.0056
DM08	65.7552	0.0113
BW18	4.8173	0.0067
BW17	3.7195	0.0040
BW14	4.0698	0.0048
BW12	4.7477	0.0065
BW10	6.5268	0.0124
BW07	6.3678	0.0118
BW05	7.0920	0.0074
BW01	34.5954	0.0924
DM09	86.0028	0.0193
BE01	70.7522	0.2312
DM20	7.1447	0.0142
DM19	8.6037	0.0676
DM17	39.6543	0.0356
DM15	15.2729	0.0108
DM10	52.0399	0.0071
BD02	43.7007	0.0068
BD01	53.4764	0.0075
BD09	8.0787	0.0015
BD07	11.3446	0.0029
BD06	15.2029	0.0028
BD05	11.7963	0.0017
BD03	34.3813	0.0081
BE10	6.4512	0.0044
BE09	8.6144	0.0034
BE06	12.1979	0.0156
W09	2.8914	0.0056
W06	2.1279	0.0030
W04	3.6146	0.0088
W02	8.8183	0.0522
W01	31.6905	0.0776
W12	9.0678	0.0552
DM03	18.2082	0.0067
BW16	4.5010	0.0059
BW09	4.5147	0.0068
BD04	13.7761	0.0023
DM16	31.2824	0.0222
DM18	55.3931	0.0695
BE02	14.6329	0.0099
BE05	7.2920	0.0025
BE04	9.9548	0.0046
BE03	24.4243	0.0276
BE13	10.7235	0.0320
BE12	11.4596	0.0365
BE11	11.8064	0.0147
W11	1.5962	0.0017
W10	6.9075	0.0320
W08	5.2802	0.0187
W07	7.9778	0.0427
W05	1.3086	0.0011
OL05	30.0173	0.0023
OL04	28.2690	0.0021
OL03	28.2500	0.0021
OL02	28.8770	0.0022
L121	40.1690	0.0042
L122	42.4517	0.0047
OL09	27.3357	0.0019
OL08	27.7906	0.0020
OL07	27.6978	0.0020
OL06	22.0507	0.0013
OL13	28.4174	0.0021
OL12	27.8093	0.0020
OL11	27.9531	0.0020
OL10	27.4699	0.0020
OL14	26.5432	0.0018
OL16	27.5452	0.0020
OL15	27.5939	0.0020
OL19	27.9790	0.0020
OL18	27.4418	0.0020
OL17	27.8931	0.0020
OL20	26.7691	0.0019
OL22	27.9419	0.0020
OL21	28.0747	0.0021
L140	29.8878	0.0023
Overflow	1.4979	0.0021

2-Year Storm – Selected XP-SWMM Result

 Table E15 - SPREADSHEET INFO LIST
 Conduit Flow and Junction Depth Information for use in
 spreadsheets. The maximum values in this table are the
 true maximum values because they sample every time step.
 The values in the review results may only be the
 maximum of a subset of all the time steps in the run.
 Note: These flows are only the flows in a single barrel.

Conduit Name	Maximum Flow (cfs)	Total Flow (ft^3)	Maximum Velocity (ft/s)	Maximum Volume (ft^3)	##	Junction Name	Invert Elevation (ft)	Maximum Elevation (ft)
DM01	53.8321	921455.2474	7.6971	1301.6345	##	DM-02	55.1200	60.3912
DM07	59.4382	926440.8684	18.1441	827.1731	##	DM-01	51.7000	54.0827
DM05	59.4403	926167.6118	10.5272	1758.4101	##	DM-03	56.0200	63.9184
DM04	59.4410	925887.8694	4.5321	5533.8533	##	DM-05	58.9000	68.9497
DM02	53.8245	921729.1583	7.5408	1186.6594	##	DM-06	63.1000	71.1419
DM08	54.9114	848509.5555	9.8144	514.6789	##	DM-08	85.6800	87.0073
BW18	1.5564	21993.4037	3.8295	35.2172	##	DM-09	90.5000	92.8282
BW17	1.5553	21981.5295	3.2637	52.7447	##	DM-10	91.7900	93.7238
BW14	1.5533	21959.0102	3.4842	42.8206	##	BW-01	98.0100	98.2259
BW12	1.5526	21945.4540	3.8523	21.4946	##	BW-05	98.9100	99.3776
BW10	1.5525	21937.7086	4.8044	14.0779	##	BW-07	100.9200	101.3134
BW07	1.5518	21918.4438	4.7601	14.2322	##	BW-10	102.9000	103.2891
BW05	1.5514	21905.2007	3.8090	8.1521	##	BW-12	103.7200	104.1980
BW01	1.5513	21900.7469	4.8677	21.5657	##	BW-14	104.7600	105.2698
DM09	54.9209	848753.6561	11.1581	357.2962	##	BW-17	106.1700	106.7094
BE01	16.2045	241068.6098	22.0589	3.6545	##	BW-18	106.7600	107.2247
DM20	6.8763	98148.4363	6.4382	42.2086	##	DM-11	93.0700	95.4295
DM19	6.8751	98136.4542	11.8411	8.5066	##	DM-14	94.7000	96.8898
DM17	22.7860	339116.1613	12.1735	295.1056	##	DM-17	105.0300	108.6866
DM15	22.7769	338878.9432	9.3796	364.9699	##	DM-19	120.1600	121.0549
DM10	53.4068	826944.5288	9.1367	975.0265	##	BE-05	157.4900	161.2659
BD02	1.7879	2769.0553	3.3878	29.8061	##	DM-20	129.5000	130.1760
BD01	1.7777	2766.4339	1.7101	859.2564	##	DM-21	131.3500	132.4140
BD09	0.0000	0.0000	0.0000	0.0000	##	BD-01	96.8200	97.1942
BD07	0.0000	0.0000	0.0000	0.0000	##	BD-02	101.4700	101.8611
BD06	0.0000	0.0000	0.0000	60.3624	##	BD-03	102.8300	103.2200
BD05	1.8175	2738.7139	2.1576	148.2528	##	BD-05	103.7700	104.3666
BD03	1.8126	2744.0653	3.7060	23.4612	##	BD-06	105.0100	105.0100
BE10	5.1976	75208.0394	3.8486	824.3614	##	BD-07	106.5400	106.5400
BE09	5.5003	75152.3194	2.5850	1028.2992	##	BD-09	107.1000	107.1000
BE06	12.1986	181762.0951	6.9669	430.7450	##	BD-13	107.6700	109.9245
W09	0.8315	11631.1301	1.3271	345.1237	##	BE-06	161.4600	166.1461
W06	-2.4043	-42995.6281	-2.9818	27.0881	##	BE-09	162.9100	166.6361
W04	-2.4011	-42901.1010	-2.9852	79.8526	##	BE-13	188.6100	189.2411
W02	-2.4014	-42809.6953	-4.1595	41.7258	##	W-10	107.7200	108.6335
W01	2.4014	42804.2957	10.5351	0.8141	##	W-07	99.1500	104.3630
W12	0.4833	6889.0408	4.1407	16.3759	##	W-13	115.3000	115.4567
DM03	53.8766	921951.0079	10.7302	411.2441	##	W-05	99.2300	104.1435
BW16	1.5543	21971.0564	3.7571	37.1898	##	W-03	100.0600	103.7761
BW09	1.5525	21927.3014	3.8948	20.7732	##	W-02	102.3600	102.6397
BD04	1.8094	2741.1031	2.3096	107.4803	##	W-01	91.6500	91.9279
DM16	22.7773	339055.0716	9.5606	542.0824	##	DM-04	56.5600	68.4500
DM18	22.8268	339167.1755	16.0350	32.5549	##	BW-16	105.6000	106.0750
BE02	16.2049	241089.4598	7.2157	84.0977	##	BW-09	101.8900	102.3700
BE05	12.2104	181649.2666	5.0417	668.9489	##	BD-04	103.4700	104.0534
BE04	16.2249	241380.5392	6.9864	301.4034	##	DM-16	101.2600	106.7172
BE03	16.2139	241263.5268	10.2402	269.5387	##	DM-18	110.1400	111.4386
BE13	5.2775	75356.3021	8.5613	11.1670	##	BE-01	134.5200	135.0895
BE12	5.2765	75329.3081	8.9368	12.8540	##	BE-02	136.8800	140.2923
BE11	5.2748	75275.8143	6.0900	304.8639	##	BE-03	156.1200	157.1714
W11	-0.4753	-6812.0952	-0.8570	91.0299	##	BE-04	156.8300	159.4374
W10	0.4778	6831.1722	2.6336	57.2540	##	BE-10	164.9200	167.8494
W08	-0.0638	-4926.4412	-0.5440	107.8410	##	BE-11	171.4500	172.1755
W07	0.1227	4934.3587	1.5420	61.7970	##	BE-12	178.3700	178.9658
W05	-2.4019	-43015.7346	-2.9827	21.4894	##	W-12	102.7100	104.3865
OL05	29.3720	485829.2380	5.0159	866.7627	##	W-11	102.5800	104.3706
OL04	29.3679	485686.4805	5.1132	676.8995	##	W-09	105.2600	108.4470
OL03	29.3693	485540.9568	5.3869	775.4821	##	W-08	108.1200	108.1243
OL02	29.3709	485397.6102	6.0677	316.9249	##	W-06	99.2500	104.2398
L121	22.7274	338807.5557	4.3108	1124.6544	##	OL-02	102.5800	104.5439
L122	51.8426	824010.0959	7.7351	1165.4548	##	OL-03	102.8900	105.0748
OL09	29.4240	486342.3042	4.6480	486.2503	##	OL-04	103.1400	105.4222
OL08	29.4070	486228.4697	4.6809	931.5218	##	OL-05	103.4900	105.8110
OL07	29.3886	486068.5087	4.7393	892.9928	##	OL-01	93.9300	96.6795
OL06	29.3772	485945.0426	4.8314	424.9280	##	OL-09	104.3200	106.8400
OL13	29.5320	486707.2476	4.5989	367.1550	##	OL-08	104.1700	106.6697
OL12	29.5043	486605.1387	4.5884	796.8924	##	OL-07	103.8700	106.3353
OL11	29.4714	486479.7547	4.6000	691.2720	##	OL-06	103.5800	105.9987
OL10	29.4459	486358.1753	4.6138	776.3718	##	OL-13	105.1500	107.7198
OL14	25.2944	390532.8977	3.9580	314.8309	##	OL-12	105.0300	107.5995
OL16	25.1776	390840.8820	4.1124	559.7434	##	OL-11	104.7800	107.3356
OL15	25.2296	390676.0443	4.0746	1680.6788	##	OL-10	104.5600	107.1045
OL19	25.2678	391244.4792	4.3432	288.3158	##	OL-14	105.2400	107.7936
OL18	25.2315	391144.5544	4.3042	966.6842	##	OL-15	105.7700	108.2234
OL17	25.1749	390981.4591	4.2265	1280.6765	##	OL-16	105.9500	108.3740
OL20	25.2943	391315.3035	4.3463	534.0430	##	OL-17	106.3800	108.7413
OL22	25.3866	391546.7305	4.4307	653.9760	##	OL-18	106.7000	109.0344
OL21	25.3385	391423.8357	4.3951	851.9043	##	OL-19	106.8000	109.1228
L140	25.4217	391627.8598	4.4939	418.5360	##	OL-20	106.9700	109.2920
Overflow	1.8297	2734.6641	2.5173	121.2457	##	OL-21	107.2700	109.5664
FREE # 1	53.8579	921451.8971	0.0000	0.0000	##	OL-22	107.5000	109.7837
FREE # 2	2.4014	42804.2065	0.0000	0.0000	##			

2-Year Storm – Selected XP-SWMM Result

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*****
Table E15a - SPREADSHEET REACH LIST
Peak flow and Total Flow listed by Reach or those
conduits or diversions having the same
upstream and downstream nodes.
*****
    
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Upstream Node	Downstream Node	Maximum Flow (cfs)	Total Flow (ft^3)
DM-02	DM-01	53.8321	921455.247
DM-08	DM-06	59.4382	926440.868
DM-06	DM-05	59.4403	926167.612
DM-05	DM-04	59.4410	925887.869
DM-03	DM-02	53.8245	921729.158
DM-09	DM-08	54.9114	848509.555
BW-18	BW-17	1.5564	21993.4037
BW-17	BW-16	1.5553	21981.5295
BW-14	BW-12	1.5533	21959.0102
BW-12	BW-10	1.5526	21945.4540
BW-10	BW-09	1.5525	21937.7086
BW-07	BW-05	1.5518	21918.4438
BW-05	BW-01	1.5514	21905.2007
BW-01	DM-10	1.5513	21900.7469
DM-10	DM-09	54.9209	848753.656
BE-01	DM-19	16.2045	241068.610
DM-21	DM-20	6.8763	98148.4363
DM-20	DM-19	6.8751	98136.4542
DM-18	DM-17	22.7860	339116.161
DM-16	DM-14	22.7769	338878.943
DM-11	DM-10	53.4068	826944.529
BD-02	BD-01	1.7879	2769.0553
BD-01	DM-11	1.7777	2766.4339
BD-05	BD-04	1.8175	2738.7139
BD-03	BD-02	1.8126	2744.0653
BE-10	BE-09	5.1976	75208.0394
BE-09	BE-06	5.5003	75152.3194
BE-06	BE-05	12.1986	181762.095
W-10	W-09	0.8315	11631.1301
W-06	W-07	2.4043	42995.6281
W-03	W-05	2.4011	42901.1010
W-02	W-03	2.4014	42809.6953
W-02	W-01	2.4014	42804.2957
W-13	W-12	0.4833	6889.0408
DM-04	DM-03	53.8766	921951.008
BW-16	BW-14	1.5543	21971.0564
BW-09	BW-07	1.5525	21927.3014
BD-04	BD-03	1.8094	2741.1031
DM-17	DM-16	22.7773	339055.072
DM-19	DM-18	22.8268	339167.176
BE-02	BE-01	16.2049	241089.460
BE-05	BE-04	12.2104	181649.267
BE-04	BE-03	16.2249	241380.539
BE-03	BE-02	16.2139	241263.527
BE-13	BE-12	5.2775	75356.3021
BE-12	BE-11	5.2765	75329.3081
BE-11	BE-10	5.2748	75275.8143
W-11	W-12	0.4753	6812.0952
W-11	W-07	0.4778	6831.1722
W-08	W-09	0.0638	4926.4412
W-08	W-07	0.1227	4934.3587
W-05	W-06	2.4019	43015.7346
OL-05	OL-04	29.3720	485829.238
OL-04	OL-03	29.3679	485686.480
OL-03	OL-02	29.3693	485540.957
OL-02	OL-01	29.3709	485397.610
DM-14	OL-01	22.7274	338807.556
OL-01	DM-11	51.8426	824010.096
OL-09	OL-08	29.4240	486342.304
OL-08	OL-07	29.4070	486228.470
OL-07	OL-06	29.3886	486068.509
OL-06	OL-05	29.3772	485945.043
OL-13	OL-12	29.5320	486707.248
OL-12	OL-11	29.5043	486605.139
OL-11	OL-10	29.4714	486479.755
OL-10	OL-09	29.4459	486358.175
OL-14	OL-13	25.2944	390532.898
OL-16	OL-15	25.1776	390840.882
OL-15	OL-14	25.2296	390676.044
OL-19	OL-18	25.2678	391244.479
OL-18	OL-17	25.2315	391144.554
OL-17	OL-16	25.1749	390981.459
OL-20	OL-19	25.2943	391315.304
OL-22	OL-21	25.3866	391546.731
OL-21	OL-20	25.3385	391423.836
BD-13	OL-22	25.4217	391627.860
OL-13	BD-05	1.8297	2734.6641

2-Year Storm – Selected XP-SWMM Result

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#####
# Table E16. New Conduit Information Section #
# Conduit Invert (IE) Elevation and Conduit #
# Maximum Water Surface (WS) Elevations #
#####
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Conduit Name	Upstream Node	Downstream Node	IE Up	IE Dn	WS Up	WS Dn	Conduit Type
DM01	DM-02	DM-01	55.1200	51.7000	60.3901	54.0827	Circular
DM07	DM-08	DM-06	85.6800	63.1000	87.0073	71.1419	Circular
DM05	DM-06	DM-05	63.1000	58.9000	71.1419	68.9448	Circular
DM04	DM-05	DM-04	58.9000	56.6000	68.9448	68.4500	Circular
DM02	DM-03	DM-02	56.0200	55.1200	63.9186	60.3901	Circular
DM08	DM-09	DM-08	90.5000	85.6800	92.8282	87.0073	Circular
BW18	BW-18	BW-17	106.7600	106.1700	107.2247	106.7094	Circular
BW17	BW-17	BW-16	106.1700	105.6000	106.7094	106.0750	Circular
BW14	BW-14	BW-12	104.7600	103.7200	105.2698	104.1980	Circular
BW12	BW-12	BW-10	103.7200	102.9000	104.1980	103.2891	Circular
BW10	BW-10	BW-09	102.9000	101.8900	103.2891	102.3700	Circular
BW07	BW-07	BW-05	100.9200	98.9100	101.3134	99.3776	Circular
BW05	BW-05	BW-01	98.9100	98.0100	99.3776	98.2259	Circular
BW01	BW-01	DM-10	98.0100	91.7900	98.2259	93.7238	Circular
DM09	DM-10	DM-09	91.7900	90.5000	93.7238	92.8282	Circular
BE01	BE-01	DM-19	134.5200	120.1600	135.0895	121.0549	Circular
DM20	DM-21	DM-20	131.3500	129.5000	132.4140	130.1760	Circular
DM19	DM-20	DM-19	129.5000	120.1600	130.1760	121.0549	Circular
DM17	DM-18	DM-17	110.1400	105.0300	111.4386	108.6866	Circular
DM15	DM-16	DM-14	101.2600	97.9100	106.7172	99.5549	Circular
DM10	DM-11	DM-10	93.0700	91.7900	95.4295	93.7238	Circular
BD02	BD-02	BD-01	101.4700	98.3100	101.8611	98.6964	Circular
BD01	BD-01	DM-11	96.8200	93.0700	97.1942	95.4295	Circular
BD09	BD-09	BD-07	107.1600	106.5400	106.5400	106.5400	Circular
BD07	BD-07	BD-06	106.5600	105.2700	105.0100	105.0100	Circular
BD06	BD-06	BD-05	105.0100	103.7700	105.0100	104.3666	Circular
BD05	BD-05	BD-04	103.7700	103.4700	104.3666	104.0534	Circular
BD03	BD-03	BD-02	102.8300	101.4700	103.2200	101.8611	Circular
BE10	BE-10	BE-09	164.9200	162.9100	167.8494	166.6361	Circular
BE09	BE-09	BE-06	162.9100	161.4600	166.6361	166.1461	Circular
BE06	BE-06	BE-05	162.4600	157.4900	166.1460	161.2659	Circular
W09	W-10	W-09	107.7200	105.2800	108.6335	108.4470	Circular
W06	W-06	W-07	99.2500	99.1500	104.2398	104.3630	Circular
W04	W-03	W-05	100.1000	99.2300	103.7761	104.1435	Circular
W02	W-02	W-03	102.8000	100.0700	103.4631	103.7761	Circular
W01	W-02	W-01	102.3600	91.6500	102.6397	91.9279	Circular
W12	W-13	W-12	115.3000	102.7100	115.4567	104.3865	Circular
DM03	DM-04	DM-03	56.5600	56.0200	68.4500	63.9184	Circular
BW16	BW-16	BW-14	105.6000	104.7600	106.0750	105.2698	Circular
BW09	BW-09	BW-07	101.8900	100.9200	102.3700	101.3134	Circular
BD04	BD-04	BD-03	103.4700	102.8300	104.0534	103.2200	Circular
DM16	DM-17	DM-16	105.0300	101.2600	108.6866	106.7172	Circular
DM18	DM-19	DM-18	120.1600	110.1400	121.0549	111.4386	Circular
BE02	BE-02	BE-01	136.8800	134.5200	140.2923	135.0895	Circular
BE05	BE-05	BE-04	157.4900	156.8300	161.2659	159.4374	Circular
BE04	BE-04	BE-03	156.8300	156.1200	159.4374	157.1714	Circular
BE03	BE-03	BE-02	156.1200	136.8800	157.1714	140.2923	Circular
BE13	BE-13	BE-12	188.6100	178.3700	189.2411	178.9658	Circular
BE12	BE-12	BE-11	178.3700	171.4500	178.9658	172.1755	Circular
BE11	BE-11	BE-10	171.4500	164.9200	172.1755	167.8494	Circular
W11	W-11	W-12	102.9400	102.7500	104.3706	104.3865	Circular
W10	W-11	W-07	102.5800	100.2900	104.3706	104.3630	Circular
W08	W-08	W-09	108.2600	105.2600	108.3608	108.4470	Circular
W07	W-08	W-07	108.1200	100.4000	108.1238	104.3630	Circular
W05	W-05	W-06	99.2800	99.2500	104.1435	104.2398	Circular
OL05	OL-05	OL-04	103.4900	103.1400	105.8110	105.4222	Circular
OL04	OL-04	OL-03	103.1400	102.8900	105.4222	105.0748	Circular
OL03	OL-03	OL-02	102.8900	102.5800	105.0748	104.5439	Circular
OL02	OL-02	OL-01	102.5800	102.4300	104.5439	104.1837	Circular
L121	DM-14	OL-01	94.7000	93.9300	96.8898	96.6795	Circular
L122	OL-01	DM-11	93.9300	93.0700	96.6795	95.4295	Circular
OL09	OL-09	OL-08	104.3200	104.1700	106.8400	106.6697	Circular
OL08	OL-08	OL-07	104.1700	103.8700	106.6697	106.3353	Circular
OL07	OL-07	OL-06	103.8700	103.5800	106.3353	105.9987	Circular
OL06	OL-06	OL-05	103.5800	103.4900	105.9987	105.8110	Circular
OL13	OL-13	OL-12	105.1500	105.0300	107.7198	107.5995	Circular
OL12	OL-12	OL-11	105.0300	104.7800	107.5995	107.3356	Circular
OL11	OL-11	OL-10	104.7800	104.5600	107.3356	107.1045	Circular
OL10	OL-10	OL-09	104.5600	104.3200	107.1045	106.8400	Circular
OL14	OL-14	OL-13	105.2400	105.1500	107.7936	107.7198	Circular
OL16	OL-16	OL-15	105.9500	105.7700	108.3740	108.2234	Circular
OL15	OL-15	OL-14	105.7700	105.2400	108.2234	107.7936	Circular
OL19	OL-19	OL-18	106.8000	106.7000	109.1228	109.0344	Circular
OL18	OL-18	OL-17	106.7000	106.3800	109.0344	108.7413	Circular
OL17	OL-17	OL-16	106.3800	105.9500	108.7413	108.3740	Circular
OL20	OL-20	OL-19	106.9700	106.8000	109.2920	109.1228	Circular
OL22	OL-22	OL-21	107.5000	107.2700	109.7837	109.5664	Circular
OL21	OL-21	OL-20	107.2700	106.9700	109.5664	109.2920	Circular
L140	BD-13	OL-22	107.6700	107.5000	109.9245	109.7837	Circular
Overflow	OL-13	BD-05	106.8000	106.3900	107.7198	106.9657	Circular

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*-----*
| Table E20 - Junction Flooding and Volume Listing. |
| The maximum volume is the total volume         |
| in the node including the volume in the        |
| flooded storage area. This is the max         |
| volume at any time. The volume in the        |
| flooded storage area is the total volume      |
| above the ground elevation, where the        |
| flooded pond storage area starts.             |
*-----*
    
```

Junction Name	Surcharged Time (min)	Flooded Time(min)	Out of 1D-System (Flooded Volume)	Maximum Volume	Passed to 2D cell OR Volume Stored in allowed Flood Pond of 1D-System
DM-02	33.4722	0.0000	0.0000	66.2265	0.0000
DM-01	0.0000	0.0000	0.0000	29.9405	0.0000
DM-03	48.4444	0.0000	0.0000	99.2534	0.0000
DM-05	32.7222	0.0000	0.0000	126.2234	0.0000
DM-06	28.6111	0.0000	0.0000	101.0541	0.0000
DM-08	0.0000	0.0000	0.0000	16.6787	0.0000
DM-09	0.0000	0.0000	0.0000	29.2564	0.0000
DM-10	0.0000	0.0000	0.0000	24.2999	0.0000
BW-01	0.0000	0.0000	0.0000	2.7125	0.0000
BW-05	0.0000	0.0000	0.0000	5.8759	0.0000
BW-07	0.0000	0.0000	0.0000	4.9429	0.0000
BW-10	0.0000	0.0000	0.0000	4.8892	0.0000
BW-12	0.0000	0.0000	0.0000	6.0063	0.0000
BW-14	0.0000	0.0000	0.0000	6.4056	0.0000
BW-17	0.0000	0.0000	0.0000	6.7784	0.0000
BW-18	0.0000	0.0000	0.0000	5.8393	0.0000
DM-11	0.0000	0.0000	0.0000	29.6500	0.0000
DM-14	0.0000	0.0000	0.0000	27.5166	0.0000
DM-17	15.1944	0.0000	0.0000	45.9488	0.0000
DM-19	0.0000	0.0000	0.0000	11.2448	0.0000
BE-05	34.1667	0.0000	0.0000	47.4485	0.0000
DM-20	0.0000	0.0000	0.0000	8.4946	0.0000
DM-21	0.0000	0.0000	0.0000	13.3702	0.0000
BD-01	0.0000	0.0000	0.0000	4.7022	0.0000
BD-02	0.0000	0.0000	0.0000	4.9151	0.0000
BD-03	0.0000	0.0000	0.0000	4.9008	0.0000
BD-05	0.0000	0.0000	0.0000	7.4969	0.0000
BD-06	0.0000	0.0000	0.0000	0.0000	0.0000
BD-07	0.0000	0.0000	0.0000	0.0000	0.0000
BD-09	0.0000	0.0000	0.0000	0.0000	0.0000
BD-13	0.0000	0.0000	0.0000	28.3295	0.0000
BE-06	22.9778	0.0000	0.0000	58.8852	0.0000
BE-09	19.4583	0.0000	0.0000	46.8218	0.0000
BE-13	0.0000	0.0000	0.0000	7.9309	0.0000
W-10	0.0000	0.0000	0.0000	11.4788	0.0000
W-07	1307.8333	0.0000	0.0000	65.5068	0.0000
W-13	0.0000	0.0000	0.0000	1.9693	0.0000
W-05	1323.7083	0.0000	0.0000	61.7430	0.0000
W-03	1310.7500	0.0000	0.0000	46.6961	0.0000
W-02	0.0000	0.0000	0.0000	3.5150	0.0000
W-01	0.0000	0.0000	0.0000	3.4927	0.0000
DM-04	43.6667	15.1111	3613.6406	149.4097	0.0000
BW-16	0.0000	0.0000	0.0000	5.9695	0.0000
BW-09	0.0000	0.0000	0.0000	6.0322	0.0000
BD-04	0.0000	0.0000	0.0000	7.3316	0.0000
DM-16	33.8611	0.0000	0.0000	68.5756	0.0000
DM-18	0.0000	0.0000	0.0000	16.3178	0.0000
BE-01	0.0000	0.0000	0.0000	7.1569	0.0000
BE-02	26.2500	0.0000	0.0000	42.8791	0.0000
BE-03	0.0000	0.0000	0.0000	13.2118	0.0000
BE-04	29.5000	0.0000	0.0000	32.7648	0.0000
BE-10	12.6500	0.0000	0.0000	36.8113	0.0000
BE-11	0.0000	0.0000	0.0000	9.1167	0.0000
BE-12	0.0000	0.0000	0.0000	7.4870	0.0000
W-12	48.6667	0.0000	0.0000	21.0668	0.0000
W-11	33.7222	0.0000	0.0000	22.5002	0.0000
W-09	1331.0417	1225.9722	6273.7379	40.0478	0.0000
W-08	0.0000	0.0000	0.0000	0.0476	0.0000
W-06	1324.2917	0.0000	0.0000	62.7014	0.0000
OL-02	0.0000	0.0000	0.0000	24.6784	0.0000
OL-03	0.0000	0.0000	0.0000	27.4537	0.0000
OL-04	0.0000	0.0000	0.0000	28.6782	0.0000
OL-05	0.0000	0.0000	0.0000	29.1652	0.0000
OL-01	0.0000	0.0000	0.0000	34.5499	0.0000
OL-09	0.0000	0.0000	0.0000	31.6665	0.0000
OL-08	0.0000	0.0000	0.0000	31.4112	0.0000
OL-07	0.0000	0.0000	0.0000	30.9793	0.0000
OL-06	0.0000	0.0000	0.0000	30.3936	0.0000
OL-13	0.0000	0.0000	0.0000	32.2924	0.0000
OL-12	0.0000	0.0000	0.0000	32.2883	0.0000
OL-11	0.0000	0.0000	0.0000	32.1133	0.0000
OL-10	0.0000	0.0000	0.0000	31.9744	0.0000
OL-14	0.0000	0.0000	0.0000	32.0886	0.0000
OL-15	0.0000	0.0000	0.0000	30.8297	0.0000
OL-16	0.0000	0.0000	0.0000	30.4600	0.0000
OL-17	0.0000	0.0000	0.0000	29.6717	0.0000
OL-18	0.0000	0.0000	0.0000	29.3337	0.0000
OL-19	0.0000	0.0000	0.0000	29.1885	0.0000
OL-20	0.0000	0.0000	0.0000	29.1786	0.0000
OL-21	0.0000	0.0000	0.0000	28.8568	0.0000
OL-22	0.0000	0.0000	0.0000	28.6968	0.0000

10-Year Storm – Selected XP-SWMM Results

```

*=====
|   Conduit Convergence Criteria   |
*=====
    
```

Conduit Name	Full Flow	Conduit Slope
DM01	39.5627	0.0120
DM07	153.5196	0.0614
DM05	81.0744	0.0171
DM04	20.1580	0.0082
DM02	26.9451	0.0056
DM08	65.7552	0.0113
BW18	4.8173	0.0067
BW17	3.7195	0.0040
BW14	4.0698	0.0048
BW12	4.7477	0.0065
BW10	6.5268	0.0124
BW07	6.3678	0.0118
BW05	7.0920	0.0074
BW01	34.5954	0.0924
DM09	86.0028	0.0193
BE01	70.7522	0.2312
DM20	7.1447	0.0142
DM19	8.6037	0.0676
DM17	39.6543	0.0356
DM15	15.2729	0.0108
DM10	52.0399	0.0071
BD02	43.7007	0.0068
BD01	53.4764	0.0075
BD09	8.0787	0.0015
BD07	11.3446	0.0029
BD06	15.2029	0.0028
BD05	11.7963	0.0017
BD03	34.3813	0.0081
BE10	3.9672	0.0044
BE09	5.7108	0.0034
BE06	12.1979	0.0156
W09	2.8914	0.0056
W06	2.1279	0.0030
W04	3.6146	0.0088
W02	8.8183	0.0522
W01	31.6905	0.0776
W12	9.0678	0.0552
DM03	18.2082	0.0067
BW16	4.5010	0.0059
BW09	4.5147	0.0068
BD04	13.7761	0.0023
DM16	21.9106	0.0222
DM18	55.3931	0.0695
BE02	14.6329	0.0099
BE05	7.2920	0.0025
BE04	9.9548	0.0046
BE03	24.4243	0.0276
BE13	10.7235	0.0320
BE12	11.4596	0.0365
BE11	7.2605	0.0147
W11	1.5962	0.0017
W10	6.9075	0.0320
W08	5.2802	0.0187
W07	7.9778	0.0427
W05	1.3086	0.0011
OL05	30.0173	0.0023
OL04	28.2690	0.0021
OL03	28.2500	0.0021
OL02	28.8770	0.0022
L121	40.1690	0.0042
L122	42.4517	0.0047
OL09	27.3357	0.0019
OL08	27.7906	0.0020
OL07	27.6978	0.0020
OL06	22.0507	0.0013
OL13	28.4174	0.0021
OL12	27.8093	0.0020
OL11	27.9531	0.0020
OL10	27.4699	0.0020
OL14	26.5432	0.0018
OL16	27.5452	0.0020
OL15	27.5939	0.0020
OL19	27.9790	0.0020
OL18	27.4418	0.0020
OL17	27.8931	0.0020
OL20	26.7691	0.0019
OL22	27.9419	0.0020
OL21	28.0747	0.0021
L140	29.8878	0.0023
Overflow	1.4979	0.0021

10-Year Storm – Selected XP-SWMM Results

Table E15 - SPREADSHEET INFO LIST
 Conduit Flow and Junction Depth Information for use in spreadsheets. The maximum values in this table are the true maximum values because they sample every time step. The values in the review results may only be the maximum of a subset of all the time steps in the run.
 Note: These flows are only the flows in a single barrel.

Conduit Name	Maximum Flow (cfs)	Total Flow (ft^3)	Maximum Velocity (ft/s)	Maximum Volume (ft^3)	##	Junction Name	Invert Elevation (ft)	Maximum Elevation (ft)
DM01	38.7388	1334908.595	6.3575	951.2262	##	DM-02	55.1200	57.5831
DM07	79.3886	1435457.570	17.6658	1186.5141	##	DM-01	51.7000	53.7258
DM05	79.3909	1435099.895	11.0480	1762.2894	##	DM-03	56.0200	59.3550
DM04	38.8086	1335955.152	7.7481	1413.2626	##	DM-05	58.9000	70.0000
DM02	38.7404	1335310.010	5.3425	1098.2168	##	DM-06	63.1000	73.8961
DM08	71.2234	1313290.041	10.4245	873.4746	##	DM-08	85.6800	87.4036
BW18	2.4098	34706.8226	4.2577	53.2762	##	DM-09	90.5000	94.4806
BW17	2.4087	34692.0754	3.6813	87.3688	##	DM-10	91.7900	95.3717
BW14	2.4066	34659.8277	3.8974	74.6475	##	BW-01	98.0100	98.2753
BW12	2.4060	34641.4178	4.3789	37.7777	##	BW-05	98.9100	99.4985
BW10	2.4059	34631.2801	5.3603	24.7727	##	BW-07	100.9200	101.4185
BW07	2.4052	34605.8291	5.3775	24.3880	##	BW-10	102.9000	103.3974
BW05	2.4050	34588.3832	4.3957	13.3831	##	BW-12	103.7200	104.3261
BW01	2.9926	34579.2522	8.6965	53.2068	##	BW-14	104.7600	105.4191
DM09	71.6544	1313597.496	11.3138	490.0487	##	BW-17	106.1700	106.8639
BE01	18.8377	368378.2249	22.3671	6.4966	##	BW-18	106.7600	107.3602
DM20	10.5975	153853.8941	8.3858	163.3163	##	DM-11	93.0700	97.5671
DM19	10.5921	153839.6097	13.1270	14.5197	##	DM-14	94.7000	99.8316
DM17	29.4299	522107.8716	12.0015	459.3522	##	DM-17	105.0300	113.2000
DM15	25.2804	517134.9621	10.3352	442.5009	##	DM-19	120.1600	121.3766
DM10	69.3741	1279136.310	9.7785	1324.5795	##	BE-05	157.4900	161.8951
BD02	4.1159	9922.3158	4.3593	81.2042	##	DM-20	129.5000	134.7886
BD01	4.5803	9933.8899	2.8399	2593.4458	##	DM-21	131.3500	138.6524
BD09	0.0000	0.0000	0.0000	0.0000	##	BD-01	96.8200	97.5719
BD07	0.0000	0.0000	0.0000	0.0000	##	BD-02	101.4700	102.0571
BD06	0.0000	0.0000	0.0000	163.7568	##	BD-03	102.8300	103.4152
BD05	4.1251	9852.1318	2.7537	261.3888	##	BD-05	103.7700	104.6793
BD03	4.1233	9879.5421	4.7203	63.0944	##	BD-06	105.0100	105.0100
BE10	5.1765	113652.4288	4.1352	571.8751	##	BD-07	106.5400	106.5400
BE09	4.9783	107797.1125	2.7885	759.9993	##	BD-09	107.1000	107.1000
BE06	12.4110	275370.2713	6.9707	503.9120	##	BD-13	107.6700	114.0600
W09	1.2794	18288.7329	1.5641	346.8600	##	BE-06	161.4600	166.8991
W06	-3.7399	-63821.2022	-4.6052	27.0875	##	BE-09	162.9100	166.9400
W04	-3.7381	-63721.8511	-4.6239	80.2838	##	BE-13	188.6100	189.4786
W02	-3.7382	-63630.3091	-5.3261	41.8767	##	W-10	107.7200	108.8978
W01	3.7382	63624.0357	12.0164	1.3902	##	W-07	99.1500	105.5342
W12	0.9767	10787.7221	6.4607	33.5381	##	W-13	115.3000	115.4787
DM03	38.7914	1335601.348	7.8526	411.2459	##	W-05	99.2300	105.0129
BW16	2.4077	34677.1407	4.1886	66.7752	##	W-03	100.0600	104.1307
BW09	2.4058	34618.1956	4.4120	36.0705	##	W-02	102.3600	102.7081
BD04	4.1223	9867.5837	3.0173	282.7949	##	W-01	91.6500	91.9971
DM16	25.3919	517346.2122	10.3198	417.0220	##	DM-04	56.5600	61.7770
DM18	29.4292	522164.1985	16.0753	247.2578	##	BW-16	105.6000	106.2123
BE02	18.8377	368403.6957	8.3138	94.4977	##	BW-09	101.8900	102.5022
BE05	12.4389	275207.0558	5.1258	668.8828	##	BD-04	103.4700	104.3431
BE04	18.8406	368782.2101	7.9958	326.1165	##	DM-16	101.2600	108.4716
BE03	18.8381	368618.9573	10.5262	418.3706	##	DM-18	110.1400	115.9326
BE13	8.1084	117845.1502	9.3468	109.6299	##	BE-01	134.5200	135.1365
BE12	8.1075	117807.1861	9.1254	225.5579	##	BE-02	136.8800	141.8503
BE11	8.1069	117732.0751	6.4366	455.1686	##	BE-03	156.1200	157.3479
W11	-0.7363	-10698.4398	-1.0267	91.0294	##	BE-04	156.8300	160.0180
W10	0.7322	10716.8040	2.9612	57.6724	##	BE-10	164.9200	170.2400
W08	-0.0638	-5105.1437	-0.5440	107.8410	##	BE-11	171.4500	177.9933
W07	0.1214	5135.8305	1.5247	84.3550	##	BE-12	178.3700	181.3700
W05	-3.7386	-63838.0682	-4.6142	21.4895	##	W-12	102.7100	105.5917
OL05	40.4465	752931.0963	5.5679	1067.9326	##	W-11	102.5800	105.5552
OL04	40.4457	752761.8493	5.7810	823.2386	##	W-09	105.2600	108.4470
OL03	40.4455	752586.1913	6.1140	939.4317	##	W-08	108.1200	108.1242
OL02	40.4454	752407.7178	6.9101	383.5864	##	W-06	99.2500	105.2431
L121	25.3704	517041.3884	4.3171	1342.8027	##	OL-02	102.5800	104.9252
L122	65.6790	1269184.572	9.2490	1342.7964	##	OL-03	102.8900	105.5877
OL09	41.2568	753557.5807	5.8244	570.5786	##	OL-04	103.1400	106.0913
OL08	41.0352	753406.6389	5.7692	1104.1140	##	OL-05	103.4900	106.7102
OL07	40.6113	753220.7991	5.5943	1074.2887	##	OL-01	93.9300	99.5348
OL06	40.4759	753071.0679	5.4763	518.6864	##	OL-09	104.3200	108.5132
OL13	41.5342	754091.5576	5.8520	422.3792	##	OL-08	104.1700	108.1861
OL12	41.3620	753973.1438	5.8287	918.8602	##	OL-07	103.8700	107.5552
OL11	41.3553	753828.9562	5.8314	800.2974	##	OL-06	103.5800	106.9939
OL10	41.3071	753686.4729	5.8279	904.0374	##	OL-13	105.1500	110.2550
OL14	35.3799	609107.4101	4.9857	363.0972	##	OL-12	105.0300	110.0139
OL16	35.3445	609486.5125	4.9760	674.3248	##	OL-11	104.7800	109.4888
OL15	35.3642	609288.4208	4.9802	1978.4376	##	OL-10	104.5600	109.0310
OL19	35.6041	609982.5244	5.0068	363.0959	##	OL-14	105.2400	110.4008
OL18	35.4869	609851.7892	4.9907	1207.8563	##	OL-15	105.7700	111.1945
OL17	35.3297	609653.5920	4.9714	1570.9548	##	OL-16	105.9500	111.4649
OL20	35.7307	610065.9922	5.0231	674.3219	##	OL-17	106.3800	112.0942
OL22	36.0755	610331.8352	5.0696	837.3388	##	OL-18	106.7000	112.5778
OL21	35.9016	610194.0972	5.0460	1081.8815	##	OL-19	106.8000	112.7234
L140	39.9547	614796.6471	5.6122	540.9391	##	OL-20	106.9700	112.9936
Overflow	4.1344	9843.5290	5.2364	151.3494	##	OL-21	107.2700	113.4264
FREE # 1	38.7593	1334906.653	0.0000	0.0000	##	OL-22	107.5000	113.7600
FREE # 2	3.7382	63624.0266	0.0000	0.0000	##			

10-Year Storm – Selected XP-SWMM Results

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*****
Table E15a - SPREADSHEET REACH LIST
Peak flow and Total Flow listed by Reach or those
conduits or diversions having the same
upstream and downstream nodes.
*****
    
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Upstream Node	Downstream Node	Maximum Flow (cfs)	Total Flow (ft^3)
DM-02	DM-01	38.7388	1334908.59
DM-08	DM-06	79.3886	1435457.57
DM-06	DM-05	79.3909	1435099.89
DM-05	DM-04	38.8086	1335955.15
DM-03	DM-02	38.7404	1335310.01
DM-09	DM-08	71.2234	1313290.04
BW-18	BW-17	2.4098	34706.8226
BW-17	BW-16	2.4087	34692.0754
BW-14	BW-12	2.4066	34659.8277
BW-12	BW-10	2.4060	34641.4178
BW-10	BW-09	2.4059	34631.2801
BW-07	BW-05	2.4052	34605.8291
BW-05	BW-01	2.4050	34588.3832
BW-01	DM-10	2.9926	34579.2522
DM-10	DM-09	71.6544	1313597.50
BE-01	DM-19	18.8377	368378.225
DM-21	DM-20	10.5975	153853.894
DM-20	DM-19	10.5921	153839.610
DM-18	DM-17	29.4299	522107.872
DM-16	DM-14	25.2804	517134.962
DM-11	DM-10	69.3741	1279136.31
BD-02	BD-01	4.1159	9922.3158
BD-01	DM-11	4.5803	9933.8899
BD-05	BD-04	4.1251	9852.1318
BD-03	BD-02	4.1233	9879.5421
BE-10	BE-09	5.1765	113652.429
BE-09	BE-06	4.9783	107797.113
BE-06	BE-05	12.4110	275370.271
W-10	W-09	1.2794	18288.7329
W-06	W-07	3.7399	63821.2022
W-03	W-05	3.7381	63721.8511
W-02	W-03	3.7382	63630.3091
W-02	W-01	3.7382	63624.0357
W-13	W-12	0.9767	10787.7221
DM-04	DM-03	38.7914	1335601.35
BW-16	BW-14	2.4077	34677.1407
BW-09	BW-07	2.4058	34618.1956
BD-04	BD-03	4.1223	9867.5837
DM-17	DM-16	25.3919	517346.212
DM-19	DM-18	29.4292	522164.199
BE-02	BE-01	18.8377	368403.696
BE-05	BE-04	12.4389	275207.056
BE-04	BE-03	18.8406	368782.210
BE-03	BE-02	18.8381	368618.957
BE-13	BE-12	8.1084	117845.150
BE-12	BE-11	8.1075	117807.186
BE-11	BE-10	8.1069	117732.075
W-11	W-12	0.7363	10698.4398
W-11	W-07	0.7322	10716.8040
W-08	W-09	0.0638	5105.1437
W-08	W-07	0.1214	5135.8305
W-05	W-06	3.7386	63838.0682
OL-05	OL-04	40.4465	752931.096
OL-04	OL-03	40.4457	752761.849
OL-03	OL-02	40.4455	752586.191
OL-02	OL-01	40.4454	752407.718
DM-14	OL-01	25.3704	517041.388
OL-01	DM-11	65.6790	1269184.57
OL-09	OL-08	41.2568	753557.581
OL-08	OL-07	41.0352	753406.639
OL-07	OL-06	40.6113	753220.799
OL-06	OL-05	40.4759	753071.068
OL-13	OL-12	41.5342	754091.558
OL-12	OL-11	41.3620	753973.144
OL-11	OL-10	41.3553	753828.956
OL-10	OL-09	41.3071	753686.473
OL-14	OL-13	35.3799	609107.410
OL-16	OL-15	35.3445	609486.512
OL-15	OL-14	35.3642	609288.421
OL-19	OL-18	35.6041	609982.524
OL-18	OL-17	35.4869	609851.789
OL-17	OL-16	35.3297	609653.592
OL-20	OL-19	35.7307	610065.992
OL-22	OL-21	36.0755	610331.835
OL-21	OL-20	35.9016	610194.097
BD-13	OL-22	39.9547	614796.647
OL-13	BD-05	4.1344	9843.5290

10-Year Storm – Selected XP-SWMM Results

 # Table E16. New Conduit Information Section #
 # Conduit Invert (IE) Elevation and Conduit #
 # Maximum Water Surface (WS) Elevations #
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Conduit Name	Upstream Node	Downstream Node	IE Up	IE Dn	WS Up	WS Dn	Conduit Type
DM01	DM-02	DM-01	55.1200	51.7000	57.5831	53.7258	Circular
DM07	DM-08	DM-06	85.6800	63.1000	87.4036	73.8961	Circular
DM05	DM-06	DM-05	63.1000	58.9000	73.8961	70.0000	Circular
DM04	DM-05	DM-04	58.9000	56.6000	70.0000	61.7770	Circular
DM02	DM-03	DM-02	56.0200	55.1200	59.3550	57.5831	Circular
DM08	DM-09	DM-08	90.5000	85.6800	94.4806	87.4036	Circular
BW18	BW-18	BW-17	106.7600	106.1700	107.3602	106.8639	Circular
BW17	BW-17	BW-16	106.1700	105.6000	106.8639	106.2123	Circular
BW14	BW-14	BW-12	104.7600	103.7200	105.4191	104.3261	Circular
BW12	BW-12	BW-10	103.7200	102.9000	104.3261	103.3974	Circular
BW10	BW-10	BW-09	102.9000	101.8900	103.3974	102.5022	Circular
BW07	BW-07	BW-05	100.9200	98.9100	101.4185	99.4985	Circular
BW05	BW-05	BW-01	98.9100	98.0100	99.4985	98.2753	Circular
BW01	BW-01	DM-10	98.0100	91.7900	98.2753	95.3717	Circular
DM09	DM-10	DM-09	91.7900	90.5000	95.3717	94.4806	Circular
BE01	BE-01	DM-19	134.5200	120.1600	135.1365	121.3766	Circular
DM20	DM-21	DM-20	131.3500	129.5000	138.6524	134.7886	Circular
DM19	DM-20	DM-19	129.5000	120.1600	134.7886	121.3766	Circular
DM17	DM-18	DM-17	110.1400	105.0300	115.9326	113.2000	Circular
DM15	DM-16	DM-14	101.2600	97.9100	108.4716	99.8316	Circular
DM10	DM-11	DM-10	93.0700	91.7900	97.5671	95.3717	Circular
BD02	BD-02	BD-01	101.4700	98.3100	102.0571	98.8954	Circular
BD01	BD-01	DM-11	96.8200	93.0700	97.5719	97.5671	Circular
BD09	BD-09	BD-07	107.1600	106.5400	106.5400	106.5400	Circular
BD07	BD-07	BD-06	106.5600	105.2700	105.0100	105.0100	Circular
BD06	BD-06	BD-05	105.0100	103.7700	105.0100	104.6793	Circular
BD05	BD-05	BD-04	103.7700	103.4700	104.6793	104.3431	Circular
BD03	BD-03	BD-02	102.8300	101.4700	103.4152	102.0571	Circular
BE10	BE-10	BE-09	164.9200	162.9100	170.2400	166.9400	Circular
BE09	BE-09	BE-06	162.9100	161.4600	166.9400	166.8991	Circular
BE06	BE-06	BE-05	162.4600	157.4900	166.8991	161.8951	Circular
W09	W-10	W-09	107.7200	105.2800	108.8978	108.4470	Circular
W06	W-06	W-07	99.2500	99.1500	105.2431	105.5342	Circular
W04	W-03	W-05	100.1000	99.2300	104.1307	105.0129	Circular
W02	W-02	W-03	102.8000	100.0700	103.6226	104.1307	Circular
W01	W-02	W-01	102.3600	91.6500	102.7081	91.9971	Circular
W12	W-13	W-12	115.3000	102.7100	115.4787	105.5917	Circular
DM03	DM-04	DM-03	56.5600	56.0200	61.7770	59.3550	Circular
BW16	BW-16	BW-14	105.6000	104.7600	106.2123	105.4191	Circular
BW09	BW-09	BW-07	101.8900	100.9200	102.5022	101.4185	Circular
BD04	BD-04	BD-03	103.4700	102.8300	104.3431	103.4152	Circular
DM16	DM-17	DM-16	105.0300	101.2600	113.2000	108.4716	Circular
DM18	DM-19	DM-18	120.1600	110.1400	121.3766	115.9326	Circular
BE02	BE-02	BE-01	136.8800	134.5200	141.8503	135.1365	Circular
BE05	BE-05	BE-04	157.4900	156.8300	161.8951	160.0180	Circular
BE04	BE-04	BE-03	156.8300	156.1200	160.0180	157.3479	Circular
BE03	BE-03	BE-02	156.1200	136.8800	157.3479	141.8503	Circular
BE13	BE-13	BE-12	188.6100	178.3700	189.4786	181.3700	Circular
BE12	BE-12	BE-11	178.3700	171.4500	181.3700	177.9933	Circular
BE11	BE-11	BE-10	171.4500	164.9200	177.9933	170.2400	Circular
W11	W-11	W-12	102.9400	102.7500	105.5552	105.5917	Circular
W10	W-11	W-07	102.5800	100.2900	105.5552	105.5342	Circular
W08	W-08	W-09	108.2600	105.2600	108.3608	108.4470	Circular
W07	W-08	W-07	108.1200	100.4000	108.1238	105.5342	Circular
W05	W-05	W-06	99.2800	99.2500	105.0129	105.2431	Circular
OL05	OL-05	OL-04	103.4900	103.1400	106.7102	106.0913	Circular
OL04	OL-04	OL-03	103.1400	102.8900	106.0913	105.5877	Circular
OL03	OL-03	OL-02	102.8900	102.5800	105.5877	104.9252	Circular
OL02	OL-02	OL-01	102.5800	102.4300	104.9252	104.4999	Circular
L121	DM-14	OL-01	94.7000	93.9300	99.8316	99.5348	Circular
L122	OL-01	DM-11	93.9300	93.0700	99.5348	97.5671	Circular
OL09	OL-09	OL-08	104.3200	104.1700	108.5132	108.1861	Circular
OL08	OL-08	OL-07	104.1700	103.8700	108.1861	107.5552	Circular
OL07	OL-07	OL-06	103.8700	103.5800	107.5552	106.9939	Circular
OL06	OL-06	OL-05	103.5800	103.4900	106.9939	106.7102	Circular
OL13	OL-13	OL-12	105.1500	105.0300	110.2550	110.0139	Circular
OL12	OL-12	OL-11	105.0300	104.7800	110.0139	109.4888	Circular
OL11	OL-11	OL-10	104.7800	104.5600	109.4888	109.0310	Circular
OL10	OL-10	OL-09	104.5600	104.3200	109.0310	108.5132	Circular
OL14	OL-14	OL-13	105.2400	105.1500	110.4008	110.2550	Circular
OL16	OL-16	OL-15	105.9500	105.7700	111.4649	111.1945	Circular
OL15	OL-15	OL-14	105.7700	105.2400	111.1945	110.4008	Circular
OL19	OL-19	OL-18	106.8000	106.7000	112.7234	112.5778	Circular
OL18	OL-18	OL-17	106.7000	106.3800	112.5778	112.0942	Circular
OL17	OL-17	OL-16	106.3800	105.9500	112.0942	111.4649	Circular
OL20	OL-20	OL-19	106.9700	106.8000	112.9937	112.7234	Circular
OL22	OL-22	OL-21	107.5000	107.2700	113.7600	113.4264	Circular
OL21	OL-21	OL-20	107.2700	106.9700	113.4265	112.9936	Circular
L140	BD-13	OL-22	107.6700	107.5000	114.0600	113.7600	Circular
Overflow	OL-13	BD-05	106.8000	106.3900	110.2550	107.2477	Circular

10-Year Storm – Selected XP-SWMM Results

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| Table E20 - Junction Flooding and Volume Listing. |
| The maximum volume is the total volume         |
| in the node including the volume in the        |
| flooded storage area. This is the max         |
| volume at any time. The volume in the        |
| flooded storage area is the total volume     |
| above the ground elevation, where the        |
| flooded pond storage area starts.           |
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Junction Name	Surcharged Time (min)	Flooded Time(min)	Out of 1D-System (Flooded Volume)	Maximum Volume	Passed to 2D cell OR Volume Stored in allowed Flood Pond of 1D-System
DM-02	0.0000	0.0000	0.0000	30.9513	0.0000
DM-01	0.0000	0.0000	0.0000	25.4563	0.0000
DM-03	86.8889	0.0000	0.0000	41.9081	0.0000
DM-05	218.6667	73.9444	98537.7676	139.4826	0.0000
DM-06	107.3056	0.0000	0.0000	135.6639	0.0000
DM-08	0.0000	0.0000	0.0000	21.6593	0.0000
DM-09	23.8833	0.0000	0.0000	50.0208	0.0000
DM-10	14.8167	0.0000	0.0000	45.0076	0.0000
BW-01	0.0000	0.0000	0.0000	3.3332	0.0000
BW-05	0.0000	0.0000	0.0000	7.3950	0.0000
BW-07	0.0000	0.0000	0.0000	6.2643	0.0000
BW-10	0.0000	0.0000	0.0000	6.2499	0.0000
BW-12	0.0000	0.0000	0.0000	7.6160	0.0000
BW-14	0.0000	0.0000	0.0000	8.2818	0.0000
BW-17	0.0000	0.0000	0.0000	8.7199	0.0000
BW-18	0.0000	0.0000	0.0000	7.5416	0.0000
DM-11	23.7333	0.0000	0.0000	56.5107	0.0000
DM-14	6.4833	0.0000	0.0000	64.4833	0.0000
DM-17	46.9750	23.7250	4687.1518	102.6642	0.0000
DM-19	0.0000	0.0000	0.0000	15.2883	0.0000
BE-05	55.3611	0.0000	0.0000	55.3542	0.0000
DM-20	22.3500	0.0000	0.0000	66.4568	0.0000
DM-21	31.1583	0.0000	0.0000	91.7621	0.0000
BD-01	0.0000	0.0000	0.0000	9.4480	0.0000
BD-02	0.0000	0.0000	0.0000	7.3776	0.0000
BD-03	0.0000	0.0000	0.0000	7.3532	0.0000
BD-05	0.0000	0.0000	0.0000	11.4261	0.0000
BD-06	0.0000	0.0000	0.0000	0.0000	0.0000
BD-07	0.0000	0.0000	0.0000	0.0000	0.0000
BD-09	0.0000	0.0000	0.0000	0.0000	0.0000
BD-13	32.0167	0.0000	0.0000	80.2967	0.0000
BE-06	41.9333	0.0000	0.0000	68.3478	0.0000
BE-09	44.4083	35.0583	5769.2928	50.6410	0.0000
BE-13	0.0000	0.0000	0.0000	10.9147	0.0000
W-10	27.4778	0.0000	0.0000	14.8004	0.0000
W-07	1343.2083	0.0000	0.0000	80.2231	0.0000
W-13	0.0000	0.0000	0.0000	2.2461	0.0000
W-05	1354.0000	0.0000	0.0000	72.6679	0.0000
W-03	1345.2500	0.0000	0.0000	51.1530	0.0000
W-02	0.0000	0.0000	0.0000	4.3743	0.0000
W-01	0.0000	0.0000	0.0000	4.3616	0.0000
DM-04	203.4444	0.0000	0.0000	65.5563	0.0000
BW-16	0.0000	0.0000	0.0000	7.6938	0.0000
BW-09	0.0000	0.0000	0.0000	7.6923	0.0000
BD-04	0.0000	0.0000	0.0000	10.9718	0.0000
DM-16	56.7056	0.0000	0.0000	90.6211	0.0000
DM-18	32.3667	0.0000	0.0000	72.7903	0.0000
BE-01	0.0000	0.0000	0.0000	7.7468	0.0000
BE-02	46.6500	0.0000	0.0000	62.4565	0.0000
BE-03	0.0000	0.0000	0.0000	15.4295	0.0000
BE-04	48.6861	0.0000	0.0000	40.0599	0.0000
BE-10	48.2139	30.4500	4000.1931	66.8511	0.0000
BE-11	34.7333	0.0000	0.0000	82.2226	0.0000
BE-12	18.3333	0.0000	0.0000	37.6974	0.0000
W-12	93.3056	0.0000	0.0000	36.2115	0.0000
W-11	62.3806	0.0000	0.0000	37.3866	0.0000
W-09	1360.1667	1331.6111	12761.0559	40.0478	0.0000
W-08	0.0000	0.0000	0.0000	0.0476	0.0000
W-06	1354.3750	0.0000	0.0000	75.3087	0.0000
OL-02	0.0000	0.0000	0.0000	29.4692	0.0000
OL-03	0.0000	0.0000	0.0000	33.8993	0.0000
OL-04	0.0000	0.0000	0.0000	37.0858	0.0000
OL-05	18.9250	0.0000	0.0000	40.4651	0.0000
OL-01	0.0000	0.0000	0.0000	70.4297	0.0000
OL-09	28.0667	0.0000	0.0000	52.6921	0.0000
OL-08	26.5417	0.0000	0.0000	50.4668	0.0000
OL-07	23.8000	0.0000	0.0000	46.3083	0.0000
OL-06	21.1833	0.0000	0.0000	42.8986	0.0000
OL-13	33.7083	0.0000	0.0000	64.1498	0.0000
OL-12	33.1167	0.0000	0.0000	62.6271	0.0000
OL-11	31.4083	0.0000	0.0000	59.1710	0.0000
OL-10	29.9583	0.0000	0.0000	56.1825	0.0000
OL-14	33.7917	0.0000	0.0000	64.8508	0.0000
OL-15	33.5667	0.0000	0.0000	68.1644	0.0000
OL-16	33.5000	0.0000	0.0000	69.2997	0.0000
OL-17	32.9750	0.0000	0.0000	71.8046	0.0000
OL-18	32.8750	0.0000	0.0000	73.8609	0.0000
OL-19	32.7750	0.0000	0.0000	74.4335	0.0000
OL-20	32.7667	0.0000	0.0000	75.6932	0.0000
OL-21	32.4417	0.0000	0.0000	77.3621	0.0000
OL-22	32.3083	16.7000	4359.1474	78.6632	0.0000

25-Year Storm – Selected XP-SWMM Results

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|  Conduit Convergence Criteria  |
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Conduit Name	Full Flow	Conduit Slope
DM01	39.5627	0.0120
DM07	153.5196	0.0614
DM05	81.0744	0.0171
DM04	20.1580	0.0082
DM02	26.9451	0.0056
DM08	65.7552	0.0113
BW18	4.8173	0.0067
BW17	3.7195	0.0040
BW14	4.0698	0.0048
BW12	4.7477	0.0065
BW10	6.5268	0.0124
BW07	6.3678	0.0118
BW05	7.0920	0.0074
BW01	34.5954	0.0924
DM09	86.0028	0.0193
BE01	70.7522	0.2312
DM20	7.1447	0.0142
DM19	8.6037	0.0676
DM17	39.6543	0.0356
DM15	15.2729	0.0108
DM10	52.0399	0.0071
BD02	43.7007	0.0068
BD01	53.4764	0.0075
BD09	8.0787	0.0015
BD07	11.3446	0.0029
BD06	15.2029	0.0028
BD05	11.7963	0.0017
BD03	34.3813	0.0081
BE10	3.9672	0.0044
BE09	5.7108	0.0034
BE06	12.1979	0.0156
W09	2.8914	0.0056
W06	2.1279	0.0030
W04	3.6146	0.0088
W02	8.8183	0.0522
W01	31.6905	0.0776
W12	9.0678	0.0552
DM03	18.2082	0.0067
BW16	4.5010	0.0059
BW09	4.5147	0.0068
BD04	13.7761	0.0023
DM16	21.9106	0.0222
DM18	55.3931	0.0695
BE02	14.6329	0.0099
BE05	7.2920	0.0025
BE04	9.9548	0.0046
BE03	24.4243	0.0276
BE13	10.7235	0.0320
BE12	11.4596	0.0365
BE11	7.2605	0.0147
W11	1.5962	0.0017
W10	6.9075	0.0320
W08	5.2802	0.0187
W07	7.9778	0.0427
W05	1.3086	0.0011
OL05	30.0173	0.0023
OL04	28.2690	0.0021
OL03	28.2500	0.0021
OL02	28.8770	0.0022
L121	40.1690	0.0042
L122	42.4517	0.0047
OL09	27.3357	0.0019
OL08	27.7906	0.0020
OL07	27.6978	0.0020
OL06	22.0507	0.0013
OL13	28.4174	0.0021
OL12	27.8093	0.0020
OL11	27.9531	0.0020
OL10	27.4699	0.0020
OL14	26.5432	0.0018
OL16	27.5452	0.0020
OL15	27.5939	0.0020
OL19	27.9790	0.0020
OL18	27.4418	0.0020
OL17	27.8931	0.0020
OL20	26.7691	0.0019
OL22	27.9419	0.0020
OL21	28.0747	0.0021
L140	29.8878	0.0023
Overflow	1.4979	0.0021

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Table E15 - SPREADSHEET INFO LIST
 Conduit Flow and Junction Depth Information for use in spreadsheets. The maximum values in this table are the true maximum values because they sample every time step. The values in the review results may only be the maximum of a subset of all the time steps in the run.
 Note: These flows are only the flows in a single barrel.

Conduit Name	Maximum Flow (cfs)	Total Flow (ft^3)	Maximum Velocity (ft/s)	Maximum Volume (ft^3)	##	Junction Name	Invert Elevation (ft)	Maximum Elevation (ft)
DM01	38.7388	1532511.326	6.3576	951.2243	##	DM-02	55.1200	57.5831
DM07	81.9164	1663357.440	17.7022	1220.1678	##	DM-01	51.7000	53.7258
DM05	81.9169	1662950.325	11.3942	1762.2894	##	DM-03	56.0200	59.3550
DM04	38.7575	1533691.613	7.7379	1413.2557	##	DM-05	58.9000	70.0000
DM02	38.7405	1532970.741	5.3334	1098.2151	##	DM-06	63.1000	74.1448
DM08	72.4769	1520193.044	10.6404	902.3380	##	DM-08	85.6800	87.4541
BW18	2.8132	40731.9314	4.4026	59.9564	##	DM-09	90.5000	94.6205
BW17	2.8122	40715.0843	3.8314	98.2488	##	DM-10	91.7900	95.5296
BW14	2.8101	40679.8847	4.0441	91.5022	##	BW-01	98.0100	98.2892
BW12	2.8096	40660.5261	4.5757	46.4582	##	BW-05	98.9100	99.5464
BW10	2.8094	40649.6977	5.5553	30.5010	##	BW-07	100.9200	101.4642
BW07	2.8088	40622.0024	5.6111	29.3213	##	BW-10	102.9000	103.4453
BW05	2.8086	40603.8864	4.6113	15.1508	##	BW-12	103.7200	104.3824
BW01	3.3250	40593.9387	9.3890	57.0729	##	BW-14	104.7600	105.4878
DM09	72.4825	1520500.156	11.3401	490.3863	##	BW-17	106.1700	106.9351
BE01	19.3574	426412.6103	22.4710	6.8723	##	BW-18	106.7600	107.4221
DM20	11.0208	178867.3207	8.6784	164.0111	##	DM-11	93.0700	97.8030
DM19	10.8889	178851.5942	13.4570	14.7475	##	DM-14	94.7000	100.1157
DM17	30.2287	605153.8858	12.0215	459.3522	##	DM-17	105.0300	113.2000
DM15	25.3053	596849.8603	10.3438	496.8267	##	DM-19	120.1600	121.4124
DM10	69.8183	1480022.502	9.8336	1324.5971	##	BE-05	157.4900	162.0794
BD02	4.2688	12887.8297	4.4040	84.7777	##	DM-20	129.5000	135.6411
BD01	4.6164	12914.8862	2.7915	2799.5876	##	DM-21	131.3500	139.6700
BD09	0.0000	0.0000	0.0000	0.0000	##	BD-01	96.8200	97.7804
BD07	0.0000	0.0000	0.0000	0.0000	##	BD-02	101.4700	102.0679
BD06	0.0000	0.0000	0.0000	171.0871	##	BD-03	102.8300	103.4250
BD05	4.2729	12809.2484	2.7821	267.8844	##	BD-05	103.7700	104.6965
BD03	4.2718	12843.0698	4.7679	65.8996	##	BD-06	105.0100	105.0100
BE10	5.1765	131161.7380	4.1352	571.8751	##	BD-07	106.5400	106.5400
BE09	4.9519	121304.8264	2.7750	760.2888	##	BD-09	107.1000	107.1000
BE06	12.4420	317752.2606	6.9758	525.3678	##	BD-13	107.6700	114.1726
W09	1.4918	21444.2077	1.8927	346.8600	##	BE-06	161.4600	166.9903
W06	-4.3806	-73647.5215	-5.3712	27.0876	##	BE-09	162.9100	166.9400
W04	-4.3763	-73546.3798	-5.3972	80.5393	##	BE-13	188.6100	191.2989
W02	-4.3763	-73455.2881	-5.9353	41.9276	##	W-10	107.7200	109.0914
W01	4.3763	73448.7722	12.5772	1.6835	##	W-07	99.1500	106.2384
W12	0.9843	12634.7695	6.4813	44.4062	##	W-13	115.3000	115.4800
DM03	38.7534	1533289.118	7.8446	411.2358	##	W-05	99.2300	105.5280
BW16	2.8113	40698.7218	4.3359	83.1213	##	W-03	100.0600	104.3252
BW09	2.8094	40635.3858	4.6048	44.0856	##	W-02	102.3600	102.7368
BD04	4.2721	12827.7987	3.0532	294.9807	##	W-01	91.6500	92.0258
DM16	25.5571	597081.5863	10.3873	417.2374	##	DM-04	56.5600	61.7770
DM18	30.2271	605209.0863	16.1634	255.2825	##	BW-16	105.6000	106.2751
BE02	19.3574	426440.6415	8.5280	96.5430	##	BW-09	101.8900	102.5609
BE05	12.4562	317568.7664	5.1345	668.8811	##	BD-04	103.4700	104.3589
BE04	19.8014	427177.5737	8.3583	335.2094	##	DM-16	101.2600	108.6164
BE03	19.7996	426998.0883	10.5693	450.7242	##	DM-18	110.1400	116.0822
BE13	9.4573	137975.8353	9.4526	199.7139	##	BE-01	134.5200	135.1455
BE12	9.4572	137940.0772	9.2907	239.9698	##	BE-02	136.8800	142.1700
BE11	8.6295	137153.1890	6.8217	455.4287	##	BE-03	156.1200	157.4183
W11	-0.8568	-12537.2889	-1.0999	91.0294	##	BE-04	156.8300	160.2380
W10	0.8523	12555.7556	3.0339	57.9243	##	BE-10	164.9200	170.2400
W08	-0.0638	-5150.5163	-0.5440	107.8410	##	BE-11	171.4500	178.9700
W07	0.1219	5185.8784	1.5282	98.0569	##	BE-12	178.3700	183.4785
W05	-4.3776	-73662.8051	-5.3833	21.4894	##	W-12	102.7100	106.3141
OL05	41.0754	871263.8020	5.6012	1076.1237	##	W-11	102.5800	106.2651
OL04	41.0752	871080.8350	5.8206	830.3081	##	W-09	105.2600	108.4470
OL03	41.0751	870894.2432	6.1568	947.4887	##	W-08	108.1200	108.1242
OL02	41.0750	870693.6734	6.9575	386.8809	##	W-06	99.2500	105.8416
L121	25.2624	596752.7963	4.3066	1342.5622	##	OL-02	102.5800	104.9458
L122	66.2873	1467155.398	9.3338	1342.8009	##	OL-03	102.8900	105.6193
OL09	42.8449	871929.9568	5.9979	570.5825	##	OL-04	103.1400	106.1591
OL08	42.4199	871773.1579	5.9636	1104.1133	##	OL-05	103.4900	106.7967
OL07	41.6033	871574.4787	5.7828	1074.4737	##	OL-01	93.9300	99.8280
OL06	41.0756	871408.9014	5.5668	523.2431	##	OL-09	104.3200	108.6652
OL13	43.2187	872508.2189	6.0936	422.3783	##	OL-08	104.1700	108.3281
OL12	42.9428	872380.6102	6.0522	918.8578	##	OL-07	103.8700	107.6751
OL11	42.9956	872229.9276	6.0643	800.2976	##	OL-06	103.5800	107.0808
OL10	42.8444	872069.0956	6.0471	904.0400	##	OL-13	105.1500	110.4602
OL14	35.7116	703974.4156	5.0315	363.0981	##	OL-12	105.0300	110.2117
OL16	36.0011	704364.8385	5.0676	674.3248	##	OL-11	104.7800	109.6706
OL15	35.9726	704155.5308	5.0651	1978.4303	##	OL-10	104.5600	109.1988
OL19	36.4654	704863.9568	5.1277	363.0980	##	OL-14	105.2400	110.5974
OL18	36.4618	704732.5110	5.1281	1207.8548	##	OL-15	105.7700	111.3444
OL17	36.1920	704530.9352	5.0933	1570.9535	##	OL-16	105.9500	111.5989
OL20	36.6678	704948.3648	5.1556	674.3228	##	OL-17	106.3800	112.1913
OL22	37.2508	705219.6504	5.2338	837.3462	##	OL-18	106.7000	112.6466
OL21	36.9764	705075.1586	5.1977	1081.8838	##	OL-19	106.8000	112.7834
L140	46.8297	720616.5979	6.5765	540.9403	##	OL-20	106.9700	113.0375
Overflow	4.2745	12798.7776	5.4021	152.2544	##	OL-21	107.2700	113.4448
FREE # 1	38.7592	1532512.801	0.0000	0.0000	##	OL-22	107.5000	113.7600
FREE # 2	4.3763	73448.9474	0.0000	0.0000	##			

Table E15a - SPREADSHEET REACH LIST
 Peak flow and Total Flow listed by Reach or those
 conduits or diversions having the same
 upstream and downstream nodes.

Upstream Node	Downstream Node	Maximum Flow (cfs)	Total Flow (ft^3)
DM-02	DM-01	38.7388	1532511.33
DM-08	DM-06	81.9164	1663357.44
DM-06	DM-05	81.9169	1662950.32
DM-05	DM-04	38.7575	1533691.61
DM-03	DM-02	38.7405	1532970.74
DM-09	DM-08	72.4769	1520193.04
BW-18	BW-17	2.8132	40731.9314
BW-17	BW-16	2.8122	40715.0843
BW-14	BW-12	2.8101	40679.8847
BW-12	BW-10	2.8096	40660.5261
BW-10	BW-09	2.8094	40649.6977
BW-07	BW-05	2.8088	40622.0024
BW-05	BW-01	2.8086	40603.8864
BW-01	DM-10	3.3250	40593.9387
DM-10	DM-09	72.4825	1520500.16
BE-01	DM-19	19.3574	426412.610
DM-21	DM-20	11.0208	178867.321
DM-20	DM-19	10.8889	178851.594
DM-18	DM-17	30.2287	605153.886
DM-16	DM-14	25.3053	596849.860
DM-11	DM-10	69.8183	1480022.50
BD-02	BD-01	4.2688	12887.8297
BD-01	DM-11	4.6164	12914.8862
BD-05	BD-04	4.2729	12809.2484
BD-03	BD-02	4.2718	12843.0698
BE-10	BE-09	5.1765	131161.738
BE-09	BE-06	4.9519	121304.826
BE-06	BE-05	12.4420	317752.261
W-10	W-09	1.4918	21444.2077
W-06	W-07	4.3806	73647.5215
W-03	W-05	4.3763	73546.3798
W-02	W-03	4.3763	73455.2881
W-02	W-01	4.3763	73448.7722
W-13	W-12	0.9843	12634.7695
DM-04	DM-03	38.7534	1533289.12
BW-16	BW-14	2.8113	40698.7218
BW-09	BW-07	2.8094	40635.3858
BD-04	BD-03	4.2721	12827.7987
DM-17	DM-16	25.5571	597081.586
DM-19	DM-18	30.2271	605209.086
BE-02	BE-01	19.3574	426440.641
BE-05	BE-04	12.4562	317568.766
BE-04	BE-03	19.8014	427177.574
BE-03	BE-02	19.7996	426998.088
BE-13	BE-12	9.4573	137975.835
BE-12	BE-11	9.4572	137940.077
BE-11	BE-10	8.6295	137153.189
W-11	W-12	0.8568	12537.2889
W-11	W-07	0.8523	12555.7556
W-08	W-09	0.0638	5150.5163
W-08	W-07	0.1219	5185.8784
W-05	W-06	4.3776	73662.8051
OL-05	OL-04	41.0754	871263.802
OL-04	OL-03	41.0752	871080.835
OL-03	OL-02	41.0751	870894.243
OL-02	OL-01	41.0750	870693.673
DM-14	OL-01	25.2624	596752.796
OL-01	DM-11	66.2873	1467155.40
OL-09	OL-08	42.8449	871929.957
OL-08	OL-07	42.4199	871773.158
OL-07	OL-06	41.6033	871574.479
OL-06	OL-05	41.0756	871408.901
OL-13	OL-12	43.2187	872508.219
OL-12	OL-11	42.9428	872380.610
OL-11	OL-10	42.9956	872229.928
OL-10	OL-09	42.8444	872069.096
OL-14	OL-13	35.7116	703974.416
OL-16	OL-15	36.0011	704364.838
OL-15	OL-14	35.9726	704155.531
OL-19	OL-18	36.4654	704863.957
OL-18	OL-17	36.4618	704732.511
OL-17	OL-16	36.1920	704530.935
OL-20	OL-19	36.6678	704948.365
OL-22	OL-21	37.2508	705219.650
OL-21	OL-20	36.9764	705075.159
BD-13	OL-22	46.8297	720616.598
OL-13	BD-05	4.2745	12798.7776

25-Year Storm – Selected XP-SWMM Results

 # Table E16. New Conduit Information Section #
 # Conduit Invert (IE) Elevation and Conduit #
 # Maximum Water Surface (WS) Elevations #
 #####

Conduit Name	Upstream Node	Downstream Node	IE Up	IE Dn	WS Up	WS Dn	Conduit Type
DM01	DM-02	DM-01	55.1200	51.7000	57.5831	53.7258	Circular
DM07	DM-08	DM-06	85.6800	63.1000	87.4541	74.1448	Circular
DM05	DM-06	DM-05	63.1000	58.9000	74.1448	70.0000	Circular
DM04	DM-05	DM-04	58.9000	56.6000	70.0000	61.7770	Circular
DM02	DM-03	DM-02	56.0200	55.1200	59.3550	57.5831	Circular
DM08	DM-09	DM-08	90.5000	85.6800	94.6205	87.4541	Circular
BW18	BW-18	BW-17	106.7600	106.1700	107.4221	106.9351	Circular
BW17	BW-17	BW-16	106.1700	105.6000	106.9351	106.2751	Circular
BW14	BW-14	BW-12	104.7600	103.7200	105.4878	104.3824	Circular
BW12	BW-12	BW-10	103.7200	102.9000	104.3824	103.4453	Circular
BW10	BW-10	BW-09	102.9000	101.8900	103.4453	102.5609	Circular
BW07	BW-07	BW-05	100.9200	98.9100	101.4642	99.5464	Circular
BW05	BW-05	BW-01	98.9100	98.0100	99.5464	98.2892	Circular
BW01	BW-01	DM-10	98.0100	91.7900	98.2892	95.5296	Circular
DM09	DM-10	DM-09	91.7900	90.5000	95.5296	94.6205	Circular
BE01	BE-01	DM-19	134.5200	120.1600	135.1455	121.4124	Circular
DM20	DM-21	DM-20	131.3500	129.5000	139.6700	135.6411	Circular
DM19	DM-20	DM-19	129.5000	120.1600	135.6411	121.4124	Circular
DM17	DM-18	DM-17	110.1400	105.0300	116.0822	113.2000	Circular
DM15	DM-16	DM-14	101.2600	97.9100	108.6164	100.1157	Circular
DM10	DM-11	DM-10	93.0700	91.7900	97.8030	95.5296	Circular
BD02	BD-02	BD-01	101.4700	98.3100	102.0679	98.9056	Circular
BD01	BD-01	DM-11	96.8200	93.0700	97.7804	97.8030	Circular
BD09	BD-09	BD-07	107.1600	106.5400	106.5400	106.5400	Circular
BD07	BD-07	BD-06	106.5600	105.2700	105.0100	105.0100	Circular
BD06	BD-06	BD-05	105.0100	103.7700	105.0100	104.6965	Circular
BD05	BD-05	BD-04	103.7700	103.4700	104.6965	104.3589	Circular
BD03	BD-03	BD-02	102.8300	101.4700	103.4250	102.0679	Circular
BE10	BE-10	BE-09	164.9200	162.9100	170.2400	166.9400	Circular
BE09	BE-09	BE-06	162.9100	161.4600	166.9400	166.9903	Circular
BE06	BE-06	BE-05	162.4600	157.4900	166.9903	162.0794	Circular
W09	W-10	W-09	107.7200	105.2800	109.0914	108.4470	Circular
W06	W-06	W-07	99.2500	99.1500	105.8416	106.2384	Circular
W04	W-03	W-05	100.1000	99.2300	104.3252	105.5280	Circular
W02	W-02	W-03	102.8000	100.0700	103.6774	104.3252	Circular
W01	W-02	W-01	102.3600	91.6500	102.7368	92.0258	Circular
W12	W-13	W-12	115.3000	102.7100	115.4800	106.3141	Circular
DM03	DM-04	DM-03	56.5600	56.0200	61.7770	59.3550	Circular
BW16	BW-16	BW-14	105.6000	104.7600	106.2751	105.4878	Circular
BW09	BW-09	BW-07	101.8900	100.9200	102.5609	101.4642	Circular
BD04	BD-04	BD-03	103.4700	102.8300	104.3589	103.4250	Circular
DM16	DM-17	DM-16	105.0300	101.2600	113.2000	108.6164	Circular
DM18	DM-19	DM-18	120.1600	110.1400	121.4124	116.0822	Circular
BE02	BE-02	BE-01	136.8800	134.5200	142.1700	135.1455	Circular
BE05	BE-05	BE-04	157.4900	156.8300	162.0794	160.2380	Circular
BE04	BE-04	BE-03	156.8300	156.1200	160.2380	157.4183	Circular
BE03	BE-03	BE-02	156.1200	136.8800	157.4183	142.1700	Circular
BE13	BE-13	BE-12	188.6100	178.3700	191.2989	183.4785	Circular
BE12	BE-12	BE-11	178.3700	171.4500	183.4785	178.9700	Circular
BE11	BE-11	BE-10	171.4500	164.9200	178.9700	170.2400	Circular
W11	W-11	W-12	102.9400	102.7500	106.2651	106.3141	Circular
W10	W-11	W-07	102.5800	100.2900	106.2651	106.2384	Circular
W08	W-08	W-09	108.2600	105.2600	108.3608	108.4470	Circular
W07	W-08	W-07	108.1200	100.4000	108.1238	106.2384	Circular
W05	W-05	W-06	99.2800	99.2500	105.5280	105.8416	Circular
OL05	OL-05	OL-04	103.4900	103.1400	106.7967	106.1591	Circular
OL04	OL-04	OL-03	103.1400	102.8900	106.1591	105.6193	Circular
OL03	OL-03	OL-02	102.8900	102.5800	105.6193	104.9458	Circular
OL02	OL-02	OL-01	102.5800	102.4300	104.9458	104.5160	Circular
L121	DM-14	OL-01	94.7000	93.9300	100.1157	99.8280	Circular
L122	OL-01	DM-11	93.9300	93.0700	99.8280	97.8030	Circular
OL09	OL-09	OL-08	104.3200	104.1700	108.6652	108.3281	Circular
OL08	OL-08	OL-07	104.1700	103.8700	108.3281	107.6751	Circular
OL07	OL-07	OL-06	103.8700	103.5800	107.6751	107.0808	Circular
OL06	OL-06	OL-05	103.5800	103.4900	107.0808	106.7967	Circular
OL13	OL-13	OL-12	105.1500	105.0300	110.4602	110.2117	Circular
OL12	OL-12	OL-11	105.0300	104.7800	110.2117	109.6706	Circular
OL11	OL-11	OL-10	104.7800	104.5600	109.6706	109.1988	Circular
OL10	OL-10	OL-09	104.5600	104.3200	109.1988	108.6652	Circular
OL14	OL-14	OL-13	105.2400	105.1500	110.5974	110.4602	Circular
OL16	OL-16	OL-15	105.9500	105.7700	111.5989	111.3444	Circular
OL15	OL-15	OL-14	105.7700	105.2400	111.3444	110.5974	Circular
OL19	OL-19	OL-18	106.8000	106.7000	112.7834	112.6466	Circular
OL18	OL-18	OL-17	106.7000	106.3800	112.6466	112.1913	Circular
OL17	OL-17	OL-16	106.3800	105.9500	112.1913	111.5989	Circular
OL20	OL-20	OL-19	106.9700	106.8000	113.0375	112.7834	Circular
OL22	OL-22	OL-21	107.5000	107.2700	113.7600	113.4448	Circular
OL21	OL-21	OL-20	107.2700	106.9700	113.4448	113.0375	Circular
L140	BD-13	OL-22	107.6700	107.5000	114.1726	113.7600	Circular
Overflow	OL-13	BD-05	106.8000	106.3900	110.4602	107.2591	Circular

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| Table E20 - Junction Flooding and Volume Listing. |
| The maximum volume is the total volume         |
| in the node including the volume in the        |
| flooded storage area. This is the max         |
| volume at any time. The volume in the        |
| flooded storage area is the total volume      |
| above the ground elevation, where the        |
| flooded pond storage area starts.             |
*-----*
    
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Junction Name	Surcharged Time (min)	Flooded Time(min)	Out of 1D-System (Flooded Volume)	Maximum Volume	Passed to 2D cell OR Volume Stored in allowed Flood Pond of 1D-System
DM-02	0.0000	0.0000	0.0000	30.9512	0.0000
DM-01	0.0000	0.0000	0.0000	25.4563	0.0000
DM-03	113.5000	0.0000	0.0000	41.9082	0.0000
DM-05	292.7222	96.4444	128581.5784	139.4826	0.0000
DM-06	140.6111	0.0000	0.0000	138.7892	0.0000
DM-08	0.0000	0.0000	0.0000	22.2930	0.0000
DM-09	33.5667	0.0000	0.0000	51.7777	0.0000
DM-10	25.2500	0.0000	0.0000	46.9922	0.0000
BW-01	0.0000	0.0000	0.0000	3.5084	0.0000
BW-05	0.0000	0.0000	0.0000	7.9972	0.0000
BW-07	0.0000	0.0000	0.0000	6.8378	0.0000
BW-10	0.0000	0.0000	0.0000	6.8519	0.0000
BW-12	0.0000	0.0000	0.0000	8.3233	0.0000
BW-14	0.0000	0.0000	0.0000	9.1460	0.0000
BW-17	0.0000	0.0000	0.0000	9.6142	0.0000
BW-18	0.0000	0.0000	0.0000	8.3203	0.0000
DM-11	33.3250	0.0000	0.0000	59.4754	0.0000
DM-14	18.7333	0.0000	0.0000	68.0532	0.0000
DM-17	55.5250	30.1000	7999.7534	102.6642	0.0000
DM-19	0.0000	0.0000	0.0000	15.7377	0.0000
BE-05	71.0905	0.0000	0.0000	57.6702	0.0000
DM-20	29.6250	0.0000	0.0000	77.1692	0.0000
DM-21	37.6583	20.0000	1389.7469	104.5491	0.0000
BD-01	0.0000	0.0000	0.0000	12.0685	0.0000
BD-02	0.0000	0.0000	0.0000	7.5130	0.0000
BD-03	0.0000	0.0000	0.0000	7.4774	0.0000
BD-05	0.0000	0.0000	0.0000	11.6428	0.0000
BD-06	0.0000	0.0000	0.0000	0.0000	0.0000
BD-07	0.0000	0.0000	0.0000	0.0000	0.0000
BD-09	0.0000	0.0000	0.0000	0.0000	0.0000
BD-13	41.7417	0.0000	0.0000	81.7118	0.0000
BE-06	49.8750	0.0000	0.0000	69.4943	0.0000
BE-09	52.4000	42.1500	9756.0033	50.6410	0.0000
BE-13	16.9750	0.0000	0.0000	33.7889	0.0000
W-10	32.8583	0.0000	0.0000	17.2330	0.0000
W-07	1352.6250	0.0000	0.0000	89.0730	0.0000
W-13	0.0000	0.0000	0.0000	2.2614	0.0000
W-05	1362.4583	0.0000	0.0000	79.1407	0.0000
W-03	1354.4167	0.0000	0.0000	53.5968	0.0000
W-02	0.0000	0.0000	0.0000	4.7347	0.0000
W-01	0.0000	0.0000	0.0000	4.7218	0.0000
DM-04	272.6389	0.0000	0.0000	65.5563	0.0000
BW-16	0.0000	0.0000	0.0000	8.4830	0.0000
BW-09	0.0000	0.0000	0.0000	8.4301	0.0000
BD-04	0.0000	0.0000	0.0000	11.1695	0.0000
DM-16	72.3167	0.0000	0.0000	92.4400	0.0000
DM-18	39.6083	0.0000	0.0000	74.6698	0.0000
BE-01	0.0000	0.0000	0.0000	7.8596	0.0000
BE-02	55.0583	17.5583	325.1577	66.4741	0.0000
BE-03	0.0000	0.0000	0.0000	16.3142	0.0000
BE-04	57.6417	0.0000	0.0000	42.8251	0.0000
BE-10	56.2167	37.4083	5909.4623	66.8511	0.0000
BE-11	41.8833	19.1167	707.8601	94.4963	0.0000
BE-12	26.2583	0.0000	0.0000	64.1940	0.0000
W-12	122.3333	0.0000	0.0000	45.2888	0.0000
W-11	79.8583	0.0000	0.0000	46.3070	0.0000
W-09	1368.1667	1343.8889	15873.6390	40.0478	0.0000
W-08	0.0000	0.0000	0.0000	0.0476	0.0000
W-06	1362.8333	0.0000	0.0000	82.8295	0.0000
OL-02	0.0000	0.0000	0.0000	29.7285	0.0000
OL-03	0.0000	0.0000	0.0000	34.2959	0.0000
OL-04	16.2667	0.0000	0.0000	37.9386	0.0000
OL-05	28.5250	0.0000	0.0000	41.5516	0.0000
OL-01	0.0000	0.0000	0.0000	74.1139	0.0000
OL-09	37.7000	0.0000	0.0000	54.6018	0.0000
OL-08	36.1750	0.0000	0.0000	52.2502	0.0000
OL-07	33.4833	0.0000	0.0000	47.8143	0.0000
OL-06	30.9083	0.0000	0.0000	43.9909	0.0000
OL-13	43.0833	0.0000	0.0000	66.7279	0.0000
OL-12	42.5417	0.0000	0.0000	65.1128	0.0000
OL-11	41.0583	0.0000	0.0000	61.4550	0.0000
OL-10	39.6167	0.0000	0.0000	58.2908	0.0000
OL-14	43.1667	0.0000	0.0000	67.3210	0.0000
OL-15	43.0083	0.0000	0.0000	70.0478	0.0000
OL-16	42.9667	0.0000	0.0000	70.9837	0.0000
OL-17	42.4583	0.0000	0.0000	73.0254	0.0000
OL-18	42.4083	0.0000	0.0000	74.7253	0.0000
OL-19	42.3750	0.0000	0.0000	75.1880	0.0000
OL-20	42.3333	0.0000	0.0000	76.2439	0.0000
OL-21	42.1000	0.0000	0.0000	77.5930	0.0000
OL-22	42.0250	26.8583	15290.1553	78.6632	0.0000

100-Year Storm – Selected XP-SWMM Results

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Conduit Convergence Criteria		
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Conduit Name	Full Flow	Conduit Slope

DM01	39.5627	0.0120
DM07	153.5196	0.0614
DM05	81.0744	0.0171
DM04	20.1580	0.0082
DM02	26.9451	0.0056
DM08	65.7552	0.0113
BW18	4.8173	0.0067
BW17	3.7195	0.0040
BW14	4.0698	0.0048
BW12	4.7477	0.0065
BW10	6.5268	0.0124
BW07	6.3678	0.0118
BW05	7.0920	0.0074
BW01	34.5954	0.0924
DM09	86.0028	0.0193
BE01	70.7522	0.2312
DM20	7.1447	0.0142
DM19	8.6037	0.0676
DM17	39.6543	0.0356
DM15	15.2729	0.0108
DM10	52.0399	0.0071
BD02	43.7007	0.0068
BD01	53.4764	0.0075
BD09	8.0787	0.0015
BD07	11.3446	0.0029
BD06	15.2029	0.0028
BD05	11.7963	0.0017
BD03	34.3813	0.0081
BE10	3.9672	0.0044
BE09	5.7108	0.0034
BE06	12.1979	0.0156
W09	2.8914	0.0056
W06	2.1279	0.0030
W04	3.6146	0.0088
W02	8.8183	0.0522
W01	31.6905	0.0776
W12	9.0678	0.0552
DM03	18.2082	0.0067
BW16	4.5010	0.0059
BW09	4.5147	0.0068
BD04	13.7761	0.0023
DM16	21.9106	0.0222
DM18	55.3931	0.0695
BE02	14.6329	0.0099
BE05	7.2920	0.0025
BE04	9.9548	0.0046
BE03	24.4243	0.0276
BE13	10.7235	0.0320
BE12	11.4596	0.0365
BE11	7.2605	0.0147
W11	1.5962	0.0017
W10	6.9075	0.0320
W08	5.2802	0.0187
W07	7.9778	0.0427
W05	1.3086	0.0011
OL05	30.0173	0.0023
OL04	28.2690	0.0021
OL03	28.2500	0.0021
OL02	28.8770	0.0022
L121	40.1690	0.0042
L122	42.4517	0.0047
OL09	27.3357	0.0019
OL08	27.7906	0.0020
OL07	27.6978	0.0020
OL06	22.0507	0.0013
OL13	28.4174	0.0021
OL12	27.8093	0.0020
OL11	27.9531	0.0020
OL10	27.4699	0.0020
OL14	26.5432	0.0018
OL16	27.5452	0.0020
OL15	27.5939	0.0020
OL19	27.9790	0.0020
OL18	27.4418	0.0020
OL17	27.8931	0.0020
OL20	26.7691	0.0019
OL22	27.9419	0.0020
OL21	28.0747	0.0021
L140	29.8878	0.0023
Overflow	1.4979	0.0021

100-Year Storm – Selected XP-SWMM Results

Table E15 - SPREADSHEET INFO LIST
 Conduit Flow and Junction Depth Information for use in spreadsheets. The maximum values in this table are the true maximum values because they sample every time step. The values in the review results may only be the maximum of a subset of all the time steps in the run.
 Note: These flows are only the flows in a single barrel.

Conduit Name	Maximum Flow (cfs)	Total Flow (ft^3)	Maximum Velocity (ft/s)	Maximum Volume (ft^3)	##	Junction Name	Invert Elevation (ft)	Maximum Elevation (ft)
DM01	38.7388	1763369.211	6.3575	951.2262	##	DM-02	55.1200	57.5831
DM07	85.1342	1935329.171	17.7244	1264.8624	##	DM-01	51.7000	53.7258
DM05	85.1371	1934897.224	11.8348	1762.2894	##	DM-03	56.0200	59.3550
DM04	38.7528	1764706.060	7.7369	1413.2395	##	DM-05	58.9000	70.0000
DM02	38.7404	1763884.120	5.3328	1098.2168	##	DM-06	63.1000	74.4754
DM08	73.8449	1766523.249	10.8092	942.4657	##	DM-08	85.6800	87.5211
BW18	3.3058	48098.1487	4.5369	68.0700	##	DM-09	90.5000	94.7256
BW17	3.3048	48079.4012	3.9832	111.3943	##	DM-10	91.7900	95.6649
BW14	3.3028	48039.6953	4.1924	113.8851	##	BW-01	98.0100	98.3025
BW12	3.3023	48018.0302	4.7831	58.0084	##	BW-05	98.9100	99.6023
BW10	3.3021	48006.0831	5.7490	38.1844	##	BW-07	100.9200	101.5175
BW07	3.3018	47975.9040	5.8631	35.6340	##	BW-10	102.9000	103.5027
BW05	3.3016	47957.7289	4.8510	18.0452	##	BW-12	103.7200	104.4500
BW01	3.5435	47947.0846	9.6446	60.3124	##	BW-14	104.7600	105.5733
DM09	73.8524	1766848.363	11.3581	490.7403	##	BW-17	106.1700	107.0245
BE01	19.3574	495093.7845	22.4876	6.8737	##	BW-18	106.7600	107.5000
DM20	11.1630	208047.4523	8.7910	164.0145	##	DM-11	93.0700	97.9950
DM19	10.8853	208028.0988	13.4534	14.7495	##	DM-14	94.7000	100.3436
DM17	30.2316	703022.1136	11.7491	459.3522	##	DM-17	105.0300	113.2000
DM15	25.3168	692210.2286	10.3478	548.1480	##	DM-19	120.1600	121.4126
DM10	70.5753	1719024.194	9.9460	1324.5987	##	BE-05	157.4900	162.5236
BD02	4.4307	16767.4467	4.4509	88.6194	##	DM-20	129.5000	135.6453
BD01	4.7291	16793.2195	2.7712	2968.6463	##	DM-21	131.3500	139.6700
BD09	0.0000	0.0000	0.0000	0.0000	##	BD-01	96.8200	97.9931
BD07	0.0000	0.0000	0.0000	0.0000	##	BD-02	101.4700	102.0791
BD06	0.0000	0.0000	0.0000	179.0174	##	BD-03	102.8300	103.4362
BD05	4.4328	16689.2609	2.8130	274.7369	##	BD-05	103.7700	104.7148
BD03	4.4322	16724.4296	4.8186	68.9374	##	BD-06	105.0100	105.0100
BE10	5.1765	152155.8889	4.1352	571.8751	##	BD-07	106.5400	106.5400
BE09	4.9621	136805.1040	2.7797	761.6954	##	BD-09	107.1000	107.1000
BE06	12.4586	368552.6510	6.9667	570.0003	##	BD-13	107.6700	114.3393
W09	1.7509	25302.4407	2.2185	346.8599	##	BE-06	161.4600	167.4339
W06	-5.1692	-85644.6615	-6.3004	27.0883	##	BE-09	162.9100	166.9400
W04	-5.1623	-85540.5301	-6.3399	80.8996	##	BE-13	188.6100	193.8900
W02	-5.1622	-85449.4865	-6.7726	41.9977	##	W-10	107.7200	109.3323
W01	5.1623	85442.3848	13.1938	2.0610	##	W-07	99.1500	107.2270
W12	1.0169	14894.5118	6.5005	59.9654	##	W-13	115.3000	115.4865
DM03	38.7504	1764239.515	7.8439	411.2367	##	W-05	99.2300	106.2544
BW16	3.3039	48061.6517	4.4775	105.5691	##	W-03	100.0600	104.5933
BW09	3.3020	47990.5587	4.8069	54.5152	##	W-02	102.3600	102.7697
BD04	4.4324	16708.4870	3.0905	308.1345	##	W-01	91.6500	92.0589
DM16	25.6281	692485.2661	10.4162	417.3534	##	DM-04	56.5600	61.7770
DM18	30.2288	703061.1719	15.9434	255.3558	##	BW-16	105.6000	106.3532
BE02	19.3574	495125.0655	8.5280	96.5430	##	BW-09	101.8900	102.6321
BE05	12.4724	368336.8053	5.1412	668.8769	##	BD-04	103.4700	104.3755
BE04	21.1289	497547.7059	8.8445	348.6213	##	DM-16	101.2600	108.6943
BE03	21.1248	497348.9623	10.6068	456.1026	##	DM-18	110.1400	116.0837
BE13	10.7902	162409.6741	9.5832	218.5439	##	BE-01	134.5200	135.1455
BE12	9.9471	161438.3122	9.2869	239.9955	##	BE-02	136.8800	142.1700
BE11	8.6295	159495.6152	6.8217	455.4287	##	BE-03	156.1200	157.5301
W11	-1.0115	-14788.6971	-1.2599	91.0283	##	BE-04	156.8300	160.6813
W10	1.0111	14805.8456	3.0710	58.2783	##	BE-10	164.9200	170.2400
W08	-0.0638	-5190.8673	-0.5441	107.8412	##	BE-11	171.4500	178.9700
W07	0.1294	5250.8089	1.6349	117.7194	##	BE-12	178.3700	183.9400
W05	-5.1689	-85657.2049	-6.3240	21.4892	##	W-12	102.7100	107.3383
OL05	41.8636	1011229.484	5.6757	1080.4744	##	W-11	102.5800	107.2697
OL04	41.8634	1011020.822	5.8785	838.2568	##	W-09	105.2600	108.4470
OL03	41.8633	1010809.723	6.2117	957.3385	##	W-08	108.1200	108.1263
OL02	41.8633	1010586.303	7.0161	390.9975	##	W-06	99.2500	106.6858
L121	25.4153	692108.0870	4.2507	1342.4036	##	OL-02	102.5800	104.9715
L122	66.9674	1702366.374	9.4285	1342.8314	##	OL-03	102.8900	105.6591
OL09	43.9551	1011989.549	6.1946	570.5826	##	OL-04	103.1400	106.2144
OL08	43.1760	1011810.130	6.0551	1104.1139	##	OL-05	103.4900	106.8643
OL07	42.2700	1011581.857	5.8882	1074.4739	##	OL-01	93.9300	100.0612
OL06	41.8637	1011395.059	5.6896	525.4323	##	OL-09	104.3200	108.8373
OL13	45.1160	1012661.064	6.3568	422.3794	##	OL-08	104.1700	108.4873
OL12	44.5992	1012512.579	6.2865	918.8592	##	OL-07	103.8700	107.8094
OL11	44.4205	1012334.478	6.2650	800.2963	##	OL-06	103.5800	107.1540
OL10	43.8234	1012151.794	6.1799	904.0326	##	OL-13	105.1500	110.7002
OL14	36.1063	815608.4474	5.0864	363.0981	##	OL-12	105.0300	110.4423
OL16	36.8451	816055.9287	5.2012	674.3235	##	OL-11	104.7800	109.8808
OL15	36.1115	815818.8708	5.0850	1978.4252	##	OL-10	104.5600	109.3911
OL19	36.8684	816623.6450	5.1982	363.0981	##	OL-14	105.2400	110.8274
OL18	36.4854	816478.6226	5.1426	1207.8557	##	OL-15	105.7700	111.5198
OL17	36.2366	816244.2075	5.0984	1570.9546	##	OL-16	105.9500	111.7557
OL20	37.1136	816719.7397	5.2346	674.3243	##	OL-17	106.3800	112.3051
OL22	38.7854	817027.7120	5.4537	837.3452	##	OL-18	106.7000	112.7273
OL21	37.5273	816867.8754	5.2809	1081.8828	##	OL-19	106.8000	112.8542
L140	55.4272	850023.0857	7.7815	540.9403	##	OL-20	106.9700	113.0898
Overflow	4.4335	16676.4420	5.5893	153.2327	##	OL-21	107.2700	113.4706
FREE # 1	38.7593	1763371.765	0.0000	0.0000	##	OL-22	107.5000	113.7600
FREE # 2	5.1623	85442.5716	0.0000	0.0000	##			

Table E15a - SPREADSHEET REACH LIST
 Peak flow and Total Flow listed by Reach or those
 conduits or diversions having the same
 upstream and downstream nodes.

Upstream Node	Downstream Node	Maximum Flow (cfs)	Total Flow (ft^3)
DM-02	DM-01	38.7388	1763369.21
DM-08	DM-06	85.1342	1935329.17
DM-06	DM-05	85.1371	1934897.22
DM-05	DM-04	38.7528	1764706.06
DM-03	DM-02	38.7404	1763884.12
DM-09	DM-08	73.8449	1766523.25
BW-18	BW-17	3.3058	48098.1487
BW-17	BW-16	3.3048	48079.4012
BW-14	BW-12	3.3028	48039.6953
BW-12	BW-10	3.3023	48018.0302
BW-10	BW-09	3.3021	48006.0831
BW-07	BW-05	3.3018	47975.9040
BW-05	BW-01	3.3016	47957.7289
BW-01	DM-10	3.5435	47947.0846
DM-10	DM-09	73.8524	1766848.36
BE-01	DM-19	19.3574	495093.785
DM-21	DM-20	11.1630	208047.452
DM-20	DM-19	10.8853	208028.099
DM-18	DM-17	30.2316	703022.114
DM-16	DM-14	25.3168	692210.229
DM-11	DM-10	70.5753	1719024.19
BD-02	BD-01	4.4307	16767.4467
BD-01	DM-11	4.7291	16793.2195
BD-05	BD-04	4.4328	16689.2609
BD-03	BD-02	4.4322	16724.4296
BE-10	BE-09	5.1765	152155.889
BE-09	BE-06	4.9621	136805.104
BE-06	BE-05	12.4586	368552.651
W-10	W-09	1.7509	25302.4407
W-06	W-07	5.1692	85644.6615
W-03	W-05	5.1623	85540.5301
W-02	W-03	5.1622	85449.4865
W-02	W-01	5.1623	85442.3848
W-13	W-12	1.0169	14894.5118
DM-04	DM-03	38.7504	1764239.51
BW-16	BW-14	3.3039	48061.6517
BW-09	BW-07	3.3020	47990.5587
BD-04	BD-03	4.4324	16708.4870
DM-17	DM-16	25.6281	692485.266
DM-19	DM-18	30.2288	703061.172
BE-02	BE-01	19.3574	495125.065
BE-05	BE-04	12.4724	368336.805
BE-04	BE-03	21.1289	497547.706
BE-03	BE-02	21.1248	497348.962
BE-13	BE-12	10.7902	162409.674
BE-12	BE-11	9.9471	161438.312
BE-11	BE-10	8.6295	159495.615
W-11	W-12	1.0115	14788.6971
W-11	W-07	1.0111	14805.8456
W-08	W-09	0.0638	5190.8673
W-08	W-07	0.1294	5250.8089
W-05	W-06	5.1689	85657.2049
OL-05	OL-04	41.8636	1011229.48
OL-04	OL-03	41.8634	1011020.82
OL-03	OL-02	41.8633	1010809.72
OL-02	OL-01	41.8633	1010586.30
DM-14	OL-01	25.4153	692108.087
OL-01	DM-11	66.9674	1702366.37
OL-09	OL-08	43.9551	1011989.55
OL-08	OL-07	43.1760	1011810.13
OL-07	OL-06	42.2700	1011581.86
OL-06	OL-05	41.8637	1011395.06
OL-13	OL-12	45.1160	1012661.06
OL-12	OL-11	44.5992	1012512.58
OL-11	OL-10	44.4205	1012334.48
OL-10	OL-09	43.8234	1012151.79
OL-14	OL-13	36.1063	815608.447
OL-16	OL-15	36.8451	816055.929
OL-15	OL-14	36.1115	815818.871
OL-19	OL-18	36.8684	816623.645
OL-18	OL-17	36.4854	816478.623
OL-17	OL-16	36.2366	816244.207
OL-20	OL-19	37.1136	816719.740
OL-22	OL-21	38.7854	817027.712
OL-21	OL-20	37.5273	816867.875
BD-13	OL-22	55.4272	850023.086
OL-13	BD-05	4.4335	16676.4420

100-Year Storm – Selected XP-SWMM Results

```
#####
# Table E16. New Conduit Information Section #
# Conduit Invert (IE) Elevation and Conduit #
# Maximum Water Surface (WS) Elevations #
#####
```

Conduit Name	Upstream Node	Downstream Node	IE Up	IE Dn	WS Up	WS Dn	Conduit Type
DM01	DM-02	DM-01	55.1200	51.7000	57.5831	53.7258	Circular
DM07	DM-08	DM-06	85.6800	63.1000	87.5211	74.4754	Circular
DM05	DM-06	DM-05	63.1000	58.9000	74.4754	70.0000	Circular
DM04	DM-05	DM-04	58.9000	56.6000	70.0000	61.7770	Circular
DM02	DM-03	DM-02	56.0200	55.1200	59.3550	57.5831	Circular
DM08	DM-09	DM-08	90.5000	85.6800	94.7256	87.5211	Circular
BW18	BW-18	BW-17	106.7600	106.1700	107.5000	107.0245	Circular
BW17	BW-17	BW-16	106.1700	105.6000	107.0245	106.3532	Circular
BW14	BW-14	BW-12	104.7600	103.7200	105.5733	104.4500	Circular
BW12	BW-12	BW-10	103.7200	102.9000	104.4500	103.5027	Circular
BW10	BW-10	BW-09	102.9000	101.8900	103.5027	102.6321	Circular
BW07	BW-07	BW-05	100.9200	98.9100	101.5175	99.6023	Circular
BW05	BW-05	BW-01	98.9100	98.0100	99.6023	98.3025	Circular
BW01	BW-01	DM-10	98.0100	91.7900	98.3025	95.6649	Circular
DM09	DM-10	DM-09	91.7900	90.5000	95.6649	94.7256	Circular
BE01	BE-01	DM-19	134.5200	120.1600	135.1455	121.4126	Circular
DM20	DM-21	DM-20	131.3500	129.5000	139.6700	135.6453	Circular
DM19	DM-20	DM-19	129.5000	120.1600	135.6453	121.4126	Circular
DM17	DM-18	DM-17	110.1400	105.0300	116.0837	113.2000	Circular
DM15	DM-16	DM-14	101.2600	97.9100	108.6943	100.3436	Circular
DM10	DM-11	DM-10	93.0700	91.7900	97.9950	95.6649	Circular
BD02	BD-02	BD-01	101.4700	98.3100	102.0791	98.9165	Circular
BD01	BD-01	DM-11	96.8200	93.0700	97.9931	97.9950	Circular
BD09	BD-09	BD-07	107.1600	106.5400	106.5400	106.5400	Circular
BD07	BD-07	BD-06	106.5600	105.2700	105.0100	105.0100	Circular
BD06	BD-06	BD-05	105.0100	103.7700	105.0100	104.7148	Circular
BD05	BD-05	BD-04	103.7700	103.4700	104.7148	104.3755	Circular
BD03	BD-03	BD-02	102.8300	101.4700	103.4362	102.0791	Circular
BE10	BE-10	BE-09	164.9200	162.9100	170.2400	166.9400	Circular
BE09	BE-09	BE-06	162.9100	161.4600	166.9400	167.4339	Circular
BE06	BE-06	BE-05	162.4600	157.4900	167.4339	162.5236	Circular
W09	W-10	W-09	107.7200	105.2800	109.3323	108.4470	Circular
W06	W-06	W-07	99.2500	99.1500	106.6858	107.2270	Circular
W04	W-03	W-05	100.1000	99.2300	104.5933	106.2544	Circular
W02	W-02	W-03	102.8000	100.0700	103.7235	104.5933	Circular
W01	W-02	W-01	102.3600	91.6500	102.7697	92.0589	Circular
W12	W-13	W-12	115.3000	102.7100	115.4865	107.3383	Circular
DM03	DM-04	DM-03	56.5600	56.0200	61.7770	59.3550	Circular
BW16	BW-16	BW-14	105.6000	104.7600	106.3532	105.5733	Circular
BW09	BW-09	BW-07	101.8900	100.9200	102.6321	101.5175	Circular
BD04	BD-04	BD-03	103.4700	102.8300	104.3755	103.4362	Circular
DM16	DM-17	DM-16	105.0300	101.2600	113.2000	108.6943	Circular
DM18	DM-19	DM-18	120.1600	110.1400	121.4126	116.0837	Circular
BE02	BE-02	BE-01	136.8800	134.5200	142.1700	135.1455	Circular
BE05	BE-05	BE-04	157.4900	156.8300	162.5236	160.6813	Circular
BE04	BE-04	BE-03	156.8300	156.1200	160.6813	157.5301	Circular
BE03	BE-03	BE-02	156.1200	136.8800	157.5301	142.1700	Circular
BE13	BE-13	BE-12	188.6100	178.3700	193.8900	183.9400	Circular
BE12	BE-12	BE-11	178.3700	171.4500	183.9400	178.9700	Circular
BE11	BE-11	BE-10	171.4500	164.9200	178.9700	170.2400	Circular
W11	W-11	W-12	102.9400	102.7500	107.2697	107.3383	Circular
W10	W-11	W-07	102.5800	100.2900	107.2697	107.2270	Circular
W08	W-08	W-09	108.2600	105.2600	108.3609	108.4470	Circular
W07	W-08	W-07	108.1200	100.4000	108.1263	107.2270	Circular
W05	W-05	W-06	99.2800	99.2500	106.2544	106.6858	Circular
OL05	OL-05	OL-04	103.4900	103.1400	106.8643	106.2144	Circular
OL04	OL-04	OL-03	103.1400	102.8900	106.2144	105.6591	Circular
OL03	OL-03	OL-02	102.8900	102.5800	105.6591	104.9715	Circular
OL02	OL-02	OL-01	102.5800	102.4300	104.9715	104.5362	Circular
L121	DM-14	OL-01	94.7000	93.9300	100.3436	100.0612	Circular
L122	OL-01	DM-11	93.9300	93.0700	100.0612	97.9950	Circular
OL09	OL-09	OL-08	104.3200	104.1700	108.8373	108.4873	Circular
OL08	OL-08	OL-07	104.1700	103.8700	108.4873	107.8094	Circular
OL07	OL-07	OL-06	103.8700	103.5800	107.8094	107.1540	Circular
OL06	OL-06	OL-05	103.5800	103.4900	107.1540	106.8643	Circular
OL13	OL-13	OL-12	105.1500	105.0300	110.7002	110.4423	Circular
OL12	OL-12	OL-11	105.0300	104.7800	110.4423	109.8808	Circular
OL11	OL-11	OL-10	104.7800	104.5600	109.8808	109.3911	Circular
OL10	OL-10	OL-09	104.5600	104.3200	109.3911	108.8373	Circular
OL14	OL-14	OL-13	105.2400	105.1500	110.8274	110.7002	Circular
OL16	OL-16	OL-15	105.9500	105.7700	111.7557	111.5198	Circular
OL15	OL-15	OL-14	105.7700	105.2400	111.5198	110.8274	Circular
OL19	OL-19	OL-18	106.8000	106.7000	112.8542	112.7273	Circular
OL18	OL-18	OL-17	106.7000	106.3800	112.7273	112.3051	Circular
OL17	OL-17	OL-16	106.3800	105.9500	112.3051	111.7557	Circular
OL20	OL-20	OL-19	106.9700	106.8000	113.0898	112.8542	Circular
OL22	OL-22	OL-21	107.5000	107.2700	113.7600	113.4706	Circular
OL21	OL-21	OL-20	107.2700	106.9700	113.4706	113.0898	Circular
L140	BD-13	OL-22	107.6700	107.5000	114.3393	113.7600	Circular
Overflow	OL-13	BD-05	106.8000	106.3900	110.7002	107.2715	Circular

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*-----*
| Table E20 - Junction Flooding and Volume Listing. |
| The maximum volume is the total volume         |
| in the node including the volume in the        |
| flooded storage area. This is the max         |
| volume at any time. The volume in the        |
| flooded storage area is the total volume      |
| above the ground elevation, where the        |
| flooded pond storage area starts.             |
*-----*
    
```

Junction Name	Surcharged Time (min)	Flooded Time(min)	Out of 1D-System (Flooded Volume)	Maximum Volume	Passed to 2D cell OR Volume Stored in allowed Flood Pond of 1D-System
DM-02	0.0000	0.0000	0.0000	30.9513	0.0000
DM-01	0.0000	0.0000	0.0000	25.4563	0.0000
DM-03	149.5556	0.0000	0.0000	41.9081	0.0000
DM-05	398.6111	127.4722	169407.7024	139.4826	0.0000
DM-06	231.3333	0.0000	0.0000	142.9435	0.0000
DM-08	0.0000	0.0000	0.0000	23.1357	0.0000
DM-09	42.9417	0.0000	0.0000	53.0990	0.0000
DM-10	34.8583	0.0000	0.0000	48.6922	0.0000
BW-01	0.0000	0.0000	0.0000	3.6761	0.0000
BW-05	0.0000	0.0000	0.0000	8.7000	0.0000
BW-07	0.0000	0.0000	0.0000	7.5078	0.0000
BW-10	0.0000	0.0000	0.0000	7.5741	0.0000
BW-12	0.0000	0.0000	0.0000	9.1733	0.0000
BW-14	0.0000	0.0000	0.0000	10.2199	0.0000
BW-17	0.0000	0.0000	0.0000	10.7376	0.0000
BW-18	0.0000	0.0000	0.0000	9.2983	0.0000
DM-11	42.5667	0.0000	0.0000	61.8873	0.0000
DM-14	29.3333	0.0000	0.0000	70.9180	0.0000
DM-17	73.3500	38.1000	10460.4053	102.6642	0.0000
DM-19	0.0000	0.0000	0.0000	15.7399	0.0000
BE-05	99.3250	0.0000	0.0000	63.2524	0.0000
DM-20	36.3000	0.0000	0.0000	77.2215	0.0000
DM-21	44.3500	26.7417	4488.9534	104.5491	0.0000
BD-01	0.0000	0.0000	0.0000	14.7411	0.0000
BD-02	0.0000	0.0000	0.0000	7.6536	0.0000
BD-03	0.0000	0.0000	0.0000	7.6171	0.0000
BD-05	0.0000	0.0000	0.0000	11.8723	0.0000
BD-06	0.0000	0.0000	0.0000	0.0000	0.0000
BD-07	0.0000	0.0000	0.0000	0.0000	0.0000
BD-09	0.0000	0.0000	0.0000	0.0000	0.0000
BD-13	51.3333	0.0000	0.0000	83.8062	0.0000
BE-06	59.7167	0.0000	0.0000	75.0677	0.0000
BE-09	68.4917	50.4667	15207.3600	50.6410	0.0000
BE-13	24.9167	12.0250	181.0354	66.3485	0.0000
W-10	37.4167	0.0000	0.0000	20.2604	0.0000
W-07	1361.4583	0.0000	0.0000	101.4952	0.0000
W-13	0.0000	0.0000	0.0000	2.3432	0.0000
W-05	1370.6667	0.0000	0.0000	88.2686	0.0000
W-03	1363.1250	0.0000	0.0000	56.9649	0.0000
W-02	0.0000	0.0000	0.0000	5.1477	0.0000
W-01	0.0000	0.0000	0.0000	5.1382	0.0000
DM-04	384.0000	0.0000	0.0000	65.5563	0.0000
BW-16	0.0000	0.0000	0.0000	9.4644	0.0000
BW-09	0.0000	0.0000	0.0000	9.3246	0.0000
BD-04	0.0000	0.0000	0.0000	11.3782	0.0000
DM-16	97.5667	0.0000	0.0000	93.4199	0.0000
DM-18	48.1417	0.0000	0.0000	74.6886	0.0000
BE-01	0.0000	0.0000	0.0000	7.8596	0.0000
BE-02	73.6750	25.4750	1970.1441	66.4741	0.0000
BE-03	0.0000	0.0000	0.0000	17.7188	0.0000
BE-04	81.0250	0.0000	0.0000	48.3957	0.0000
BE-10	78.9000	45.2667	7229.2162	66.8511	0.0000
BE-11	50.2333	25.6917	1871.6298	94.4963	0.0000
BE-12	32.5750	21.3333	932.1201	69.9926	0.0000
W-12	192.1111	0.0000	0.0000	58.1590	0.0000
W-11	107.1881	0.0000	0.0000	58.9310	0.0000
W-09	1376.0417	1355.0833	19695.8888	40.0478	0.0000
W-08	0.0000	0.0000	0.0000	0.0792	0.0000
W-06	1371.0417	0.0000	0.0000	93.4385	0.0000
OL-02	0.0000	0.0000	0.0000	30.0519	0.0000
OL-03	0.0000	0.0000	0.0000	34.7963	0.0000
OL-04	22.4500	0.0000	0.0000	38.6335	0.0000
OL-05	37.5500	0.0000	0.0000	42.4021	0.0000
OL-01	0.0000	0.0000	0.0000	77.0448	0.0000
OL-09	47.0833	0.0000	0.0000	56.7639	0.0000
OL-08	45.5083	0.0000	0.0000	54.2512	0.0000
OL-07	42.6833	0.0000	0.0000	49.5024	0.0000
OL-06	40.0000	0.0000	0.0000	44.9106	0.0000
OL-13	52.5000	0.0000	0.0000	69.7444	0.0000
OL-12	51.9750	0.0000	0.0000	68.0114	0.0000
OL-11	50.5167	0.0000	0.0000	64.0966	0.0000
OL-10	49.0917	0.0000	0.0000	60.7076	0.0000
OL-14	52.6000	0.0000	0.0000	70.2112	0.0000
OL-15	52.4667	0.0000	0.0000	72.2522	0.0000
OL-16	52.4667	0.0000	0.0000	72.9548	0.0000
OL-17	52.0833	0.0000	0.0000	74.4546	0.0000
OL-18	51.9000	0.0000	0.0000	75.7387	0.0000
OL-19	51.8167	0.0000	0.0000	76.0766	0.0000
OL-20	51.9250	0.0000	0.0000	76.9010	0.0000
OL-21	51.7083	0.0000	0.0000	77.9184	0.0000
OL-22	51.6083	35.9167	32876.8057	78.6632	0.0000

ATTACHMENT 3
City of Arlington Stormwater Projects

Problem No.:	1	Basin ID:	DT-B-1
Primary Issue(s)	Local flooding		
Problem Description	Apparent surcharging along First Street between McLeod and Lenore Avenues, Lenore Avenue between First and Second Streets; and along French Avenue between First and Third Streets (variable pipe diameters in French Avenue);		
Information Sources	1995, 1999, and 2003 assessments; residual areas of Project 11, Downtown Drainage System Improvements, not addressed by the Olympic Avenue renovation in 2007; also Project 201		

Problem No.:	2	Basin ID:	DT-B-2
Primary Issue(s)	Conveyance limitations		
Problem Description	Trunk line along SR9 surcharges at manholes north of Burke Avenue; downstream-most pipe segment to outfall is old and outlet is crushed; requires realignment in conjunction with separate wetland treatment project		
Information Sources	1995, 1999, and 2003 assessments; residual areas of Project 12, Downtown Outfall Trunk Line		

Upper Mainstem Stillaguamish Basin

Problem Identified:

This project is intended to address capacity issues along this stretch of the drainage system

Proposed Project:

Downtown Drainage System Improvements, provides for collection of stormwater along Olympic Avenue from Division to East First Street, then up E First Street to Lenore Avenue and within a two block section along French Avenue from E First Street to Third Street.

ITEM NO.	ITEM	Quantity	Units	Unit Price	Total Price	2006 Unit Price	2006 Total Price
11 Downtown Drainage System Improvements							
1	Traffic Safety Control Devices	1	LS	\$20,000	\$20,000	\$22,510	\$22,510
2	Remove Manhole	5	EA	\$500	\$2,500	\$563	\$2,814
3	Remove Drainage Pipe	500	LF	\$10	\$5,000	\$11	\$5,628
4	Sawcut Existing Pavement (RSM)	6,580	LF	\$2.00	\$13,160	\$2	\$14,812
5	Remove & Dispose of AC Paving*	1,760	SY	\$3.50	\$6,160	\$4	\$6,933
6	Roadway Excavation, w/ Haul*	6,743	CY	\$8.50	\$57,316	\$10	\$64,509
7	Temporary Patch	301	Ton	\$45	\$13,545	\$51	\$15,245
8	AC Paving Class A*	301	Ton	\$38	\$11,438	\$43	\$12,874
9	Controlled Density Fill*	2,117	CY	\$65	\$137,605	\$73	\$154,876
10	Storm Sewer Pipe (21-inch)*	880	LF	\$30	\$26,400	\$34	\$29,713
11	Storm Sewer Pipe (24-inch)*	320	LF	\$40	\$12,800	\$45	\$14,407
12	Storm Sewer Pipe (27-inch)	990	LF	\$48	\$47,520	\$54	\$53,484
13	Storm Sewer Pipe (30-inch)*	0	LF	\$52	\$0	\$59	\$0
14	Storm Sewer Pipe (36-inch)*	2,470	LF	\$50	\$123,500	\$56	\$139,000
15	Storm Sewer Pipe (48-inch)	0	LF	\$85	\$0	\$96	\$0
16	Reconnect Existing Catch Basins	20	EA	\$500	\$10,000	\$563	\$11,255
17	Type 2 - 54" Catch Basins*	5	EA	\$2,500	\$12,500	\$2,814	\$14,069
18	Type 2 - 60" Catch Basins*	0	EA	\$3,200	\$0	\$3,602	\$0
19	Type 2 - 72" Catch Basins*	9	EA	\$3,800	\$34,200	\$4,277	\$38,492
20	Type 2 - 96" Catch Basins	0	EA	\$7,500	\$0	\$8,441	\$0
21	Relocate Existing Utilities	1	LS	\$10,000	\$10,000	\$11,255	\$11,255
22	Trench Safety	4,660	LF	\$2	\$9,320	\$2	\$10,490
New Items							
23	SPCC Plan, Silt Fence, & TESC Measures	4,660	LF	\$5	\$23,300	\$6	\$28
24	Traffic Control Labor	800	Hours	\$35	\$28,000	\$39	\$1,379
Subtotal					\$604,264		\$623,772
25	Mobilization/Demob/Overhead & Profit	1	LS	10%	\$60,426	10%	\$62,377
26	Contingency	1	LS	30%	\$199,407	30%	\$205,845
27	Sales Tax	1	LS	8.5%	\$73,448	8.5%	\$75,820
Estimated Construction Cost					\$937,545		\$967,814
28	Eng'r, Permitting, Legal, Admin (25%)	1	LS	25%	\$234,386	25%	\$241,953
29	Surveying (Design, Const. & As-built)	1	LS	4%	\$37,502	4%	\$38,713
30	Construction Management (4%)	1	LS	4%	\$37,502	4%	\$38,713
CITY'S ESTIMATE (2004)					\$1,294,000		
TOTAL 2002 DOLLARS					\$1,246,935		
TOTAL 2006 DOLLARS							\$1,287,192

Notes:

- 1 Quantities based on previous take-off in 1995 Comprehensive Plan with New Items.
- 2 (*) Indicates Unit Prices from 67th Ave. NE Bid Tab opened March 15, 2001.
- 3 (RSM) Indicates Unit Prices from RS Means.
- 4 Assumes a single contractor to perform all projects in the area under one contract.
- 5 See accompanying text for more detailed description.

Upper Mainstem Stillaguamish Basin

Problem Identified:

Listed in the 1995 Stormwater Master Plan, this project is intended to addresses capacity issues along this stretch of the drainage system.

Proposed Project:

The Downtown Outfall Trunk Line, includes an existing outfall to the Stillaguamish River. This outfall trunkline would be relocated (the outfall location to the river would probably not change but would be improved). The new trunk line would proceed down Division Street from Olympic due west across SR-9 to a new stormwater treatment facility.

ITEM NO.	ITEM	Quantity	Units	Unit Price	Total Price	2006 Unit Price	2006 Total Price
12 Outfall Trunk Line Improvements							
1	Traffic Safety Control Devices	1	LS	\$7,500	\$7,500	\$8,441	\$8,441
2	Remove Manhole	6	EA	\$500	\$3,000	\$563	\$3,377
3	Remove Drainage Pipe	550	LF	\$10	\$5,500	\$11	\$6,190
4	Sawcut Existing Pavement (RSM)	1,100	LF	\$2.00	\$2,200	\$2	\$2,476
5	Remove & Dispose of AC Paving*	336	SY	\$3.50	\$1,176	\$4	\$1,324
6	Roadway Excavation, w/ Haul*	896	CY	\$8.50	\$7,616	\$10	\$8,572
7	Temporary Patch	57	Ton	\$45	\$2,565	\$51	\$2,887
8	AC Paving Class A*	57	Ton	\$38	\$2,166	\$43	\$2,438
9	Controlled Density Fill*	894	CY	\$65	\$58,110	\$73	\$65,403
10	Storm Sewer Pipe (21-inch)*	0	LF	\$30	\$0	\$34	\$0
11	Storm Sewer Pipe (24-inch)*	0	LF	\$40	\$0	\$45	\$0
12	Storm Sewer Pipe (27-inch)	0	LF	\$48	\$0	\$54	\$0
13	Storm Sewer Pipe (30-inch)*	0	LF	\$52	\$0	\$59	\$0
14	Storm Sewer Pipe (36-inch)*	0	LF	\$50	\$0	\$56	\$0
15	Storm Sewer Pipe (48-inch)	550	LF	\$85	\$46,750	\$96	\$52,618
16	Reconnect Existing Catch Basins	2	EA	\$500	\$1,000	\$563	\$1,126
17	Type 2 - 54" Catch Basins*	5	EA	\$2,500	\$12,500	\$2,814	\$14,069
18	Type 2 - 60" Catch Basins*	0	EA	\$3,200	\$0	\$3,602	\$0
19	Type 2 - 72" Catch Basins*	2	EA	\$3,800	\$7,600	\$4,277	\$8,554
20	Type 2 - 96" Catch Basins	0	EA	\$7,500	\$0	\$8,441	\$0
21	Relocate Existing Utilities	1	LS	\$2,500	\$2,500	\$2,814	\$2,814
22	Trench Safety	550	LF	\$2	\$1,100	\$2	\$1,238
New Items							
23	SPCC Plan, Silt Fence, & TESC Measures	550	LF	\$5	\$2,750	\$6	\$3,095
24	Traffic Control Labor	200	Hours	\$35	\$7,000	\$39	\$7,879
Subtotal					\$171,033		\$192,499
25	Mobilization/Demob/Overhead & Profit	1	LS	10%	\$17,103	10%	\$19,250
26	Contingency	1	LS	30%	\$56,441	30%	\$63,525
27	Sales Tax	1	LS	8.5%	\$20,789	8.5%	\$23,398
Estimated Construction Cost					\$265,366		\$298,672
28	Eng'r, Permitting, Legal, Admin (25%)	1	LS	25%	\$66,342	25%	\$74,668
29	Surveying (Design, Const. & As-built)	1	LS	4%	\$10,615	4%	\$11,947
30	Construction Management (4%)	1	LS	4%	\$10,615	4%	\$11,947
CITY'S ESTIMATE (2004)					\$290,000		
TOTAL 2002 DOLLARS					\$352,937		
TOTAL 2006 DOLLARS							\$397,234

Notes:

- 1 Quantities based on previous take-off in 1995 Comprehensive Plan with New Items.
- 2 (*) Indicates Unit Prices from 67th Ave. NE Bid Tab opened March 15, 2001.
- 3 (RSM) Indicates Unit Prices from RS Means.
- 4 Assumes a single contractor to perform all projects in the area under one contract.
- 5 See accompanying text for more detailed description.

Upper Mainstem Stillaguamish Basin

Problem Identified:

The storm drain system experiences surcharging in several locations during intense rainfall, resulting in localized flooding, where flows surface from catchbasins to flow down slope across roads and properties.

Proposed Project:

The project will upgrade pipe size and slopes through the basin.

ITEM NO.	ITEM	Quantity	Units	Unit Price	Total Price	2006 Unit Price	2006 Total Price
201 Downtown Storm Drain Facility (E Maple St, E 1st St, N Lenore Ave, N French Ave, E Division St)							
1	Traffic Safety Control Devices	12	LS	\$20,000	\$240,000	\$22,510	\$270,122
2	Remove Manhole	5	EA	\$500	\$2,500	\$563	\$2,814
3	Remove Drainage Pipe	200	LF	\$10	\$2,000	\$11	\$2,251
4	Sawcut Existing Pavement (RSM)	2750	LF	\$2.00	\$5,500	\$2	\$6,190
5	Remove & Dispose of AC Paving*	900	SY	\$3.50	\$3,150	\$4	\$3,545
6	Roadway Excavation, w/ Haul*	4000	CY	\$8.50	\$34,000	\$10	\$38,267
7	Temporary Patch	180	Ton	\$45	\$8,100	\$51	\$9,117
8	AC Paving Class A*	180	Ton	\$38	\$6,840	\$43	\$7,698
9	Controlled Density Fill*	1500	CY	\$65	\$97,500	\$73	\$109,737
10	Storm Sewer Pipe (21-inch)*	0	LF	\$30	\$0	\$34	\$0
11	Storm Sewer Pipe (24-inch)*	0	LF	\$40	\$0	\$45	\$0
12	Storm Sewer Pipe (27-inch)	2250	LF	\$48	\$108,000	\$54	\$121,555
13	Storm Sewer Pipe (30-inch)*	0	LF	\$52	\$0	\$59	\$0
14	Storm Sewer Pipe (36-inch)*	500	LF	\$50	\$25,000	\$56	\$28,138
15	Storm Sewer Pipe (48-inch)	0	LF	\$85	\$0	\$96	\$0
16	Reconnect Existing Catch Basins	9	EA	\$500	\$4,500	\$563	\$5,065
17	Type 2 - 54" Catch Basins*	0	EA	\$2,500	\$0	\$2,814	\$0
18	Type 2 - 60" Catch Basins*	0	EA	\$3,200	\$0	\$3,602	\$0
19	Type 2 - 72" Catch Basins*	0	EA	\$3,800	\$0	\$4,277	\$0
20	Type 2 - 96" Catch Basins	0	EA	\$7,500	\$0	\$8,441	\$0
21	Relocate Existing Utilities	1	LS	\$10,000	\$10,000	\$11,255	\$11,255
22	Trench Safety	2,750	LF	\$2	\$5,500	\$2	\$6,190
23	SPCC Plan, Silt Fence, & TESC Measures	3,000	LF	\$5	\$15,000	\$6	\$16,883
24	Traffic Control Labor	350	Hours	\$35	\$12,250	\$39	\$13,787
Subtotal					\$579,840		\$652,615
25	Mobilization/Demob/Overhead & Profit	1	LS	10%	\$57,984	10%	\$65,262
26	Contingency	1	LS	30%	\$191,347	30%	\$215,363
27	Sales Tax	1	LS	8.5%	\$70,480	8.5%	\$79,325
Estimated Construction Cost					\$899,651		\$1,012,565
28	Eng'r, Permitting, Legal, Admin (15%)	1	LS	15%	\$134,948	15%	\$151,885
29	Surveying (Property, Design & Const.)	1	LS	4%	\$35,986	4%	\$40,503
30	Construction Management (4%)	1	LS	4%	\$35,986	4%	\$40,503
CITY'S ESTIMATE (2003)							
TOTAL 2002 DOLLARS					\$1,106,570		
TOTAL 2006 DOLLARS							\$1,245,455

Notes:

- 1 Quantities based on discussions with City of Arlington staff 12/10/2002.
- 2 (*) Indicates Unit Prices from 67th Ave. NE Bid Tab opened March 15, 2001.
- 3 (RSM) Indicates Unit Prices from RS Means.
- 4 Assumes a single contractor to perform all projects in the area under one contract.
- 5 See accompanying text for more detailed description.

Hydraulic Model: Existing Conditions

**APPENDIX D
EXISTING STORMWATER MODEL RESULTS FOR ARLINGTON
CONSTRUCTED WETLAND**

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Stormwater Model

Below is a diagram of the stormwater model showing the labels of various pipes, catch basins, and wetland cells. Note that DM-01 is the Butler outfall. This figure applies to Figures D-2 through D-6.

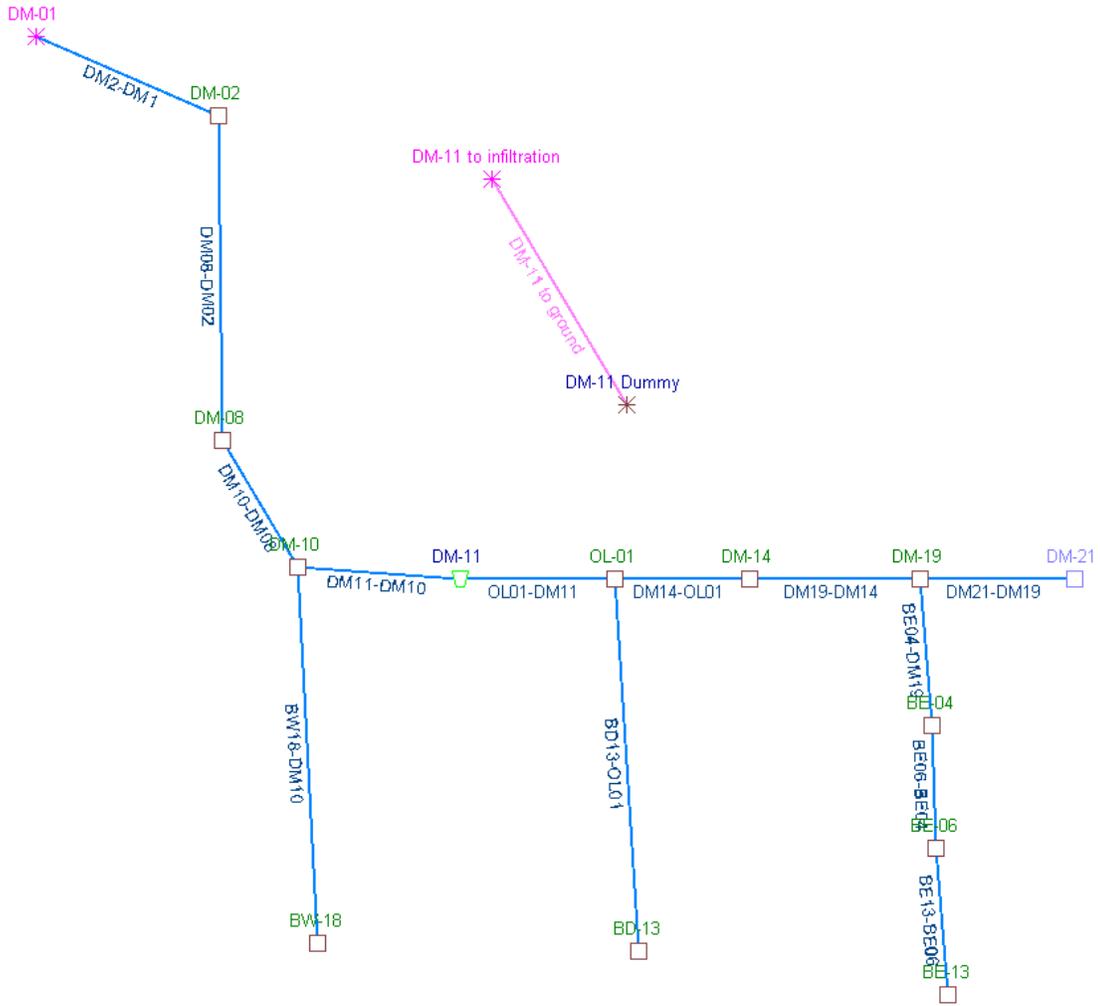


Figure D-1. Stormwater Model

Weekly Event – 0.38 inches of Rain

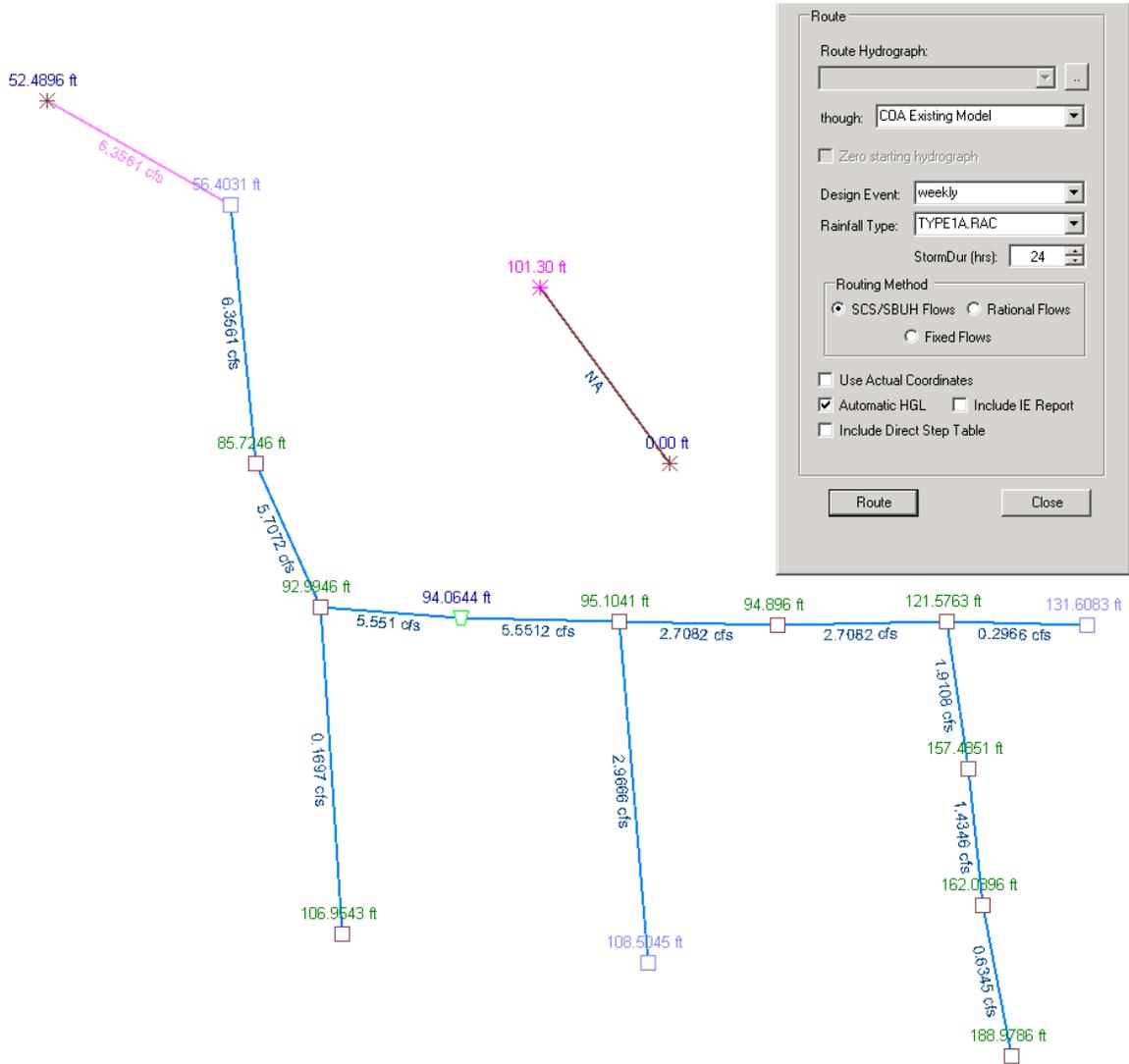


Figure D-2. Peak Flow Rate Results from Weekly Event Model

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ROUTEHYD [] THRU [COA Existing Model] USING [weekly] AND [TYPE1A.RAC] NOTZERO RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	0.6345	15.7042	0.0404	0.2401	0.1372	21 in Diam	3.193	6.529	0.00	BE-13
BE06-BE04	57.50	1.4346	11.4736	0.125	0.4179	0.2388	21 in Diam	3.2553	4.7701	0.00	BE-06
BE04-DM19	76.80	1.9108	23.0196	0.083	0.3404	0.1945	21 in Diam	5.8066	9.5704	0.00	BE-04
DM21-DM19	8.90	0.2966	10.7468	0.0276	0.143	0.1144	15 in Diam	3.8137	8.7572	0.00	DM-21b
DM19-DM14	101.80	2.7082	31.3855	0.0863	0.3969	0.1984	24 in Diam	6.1228	9.9903	0.00	DM-21a
DM14-OL01	101.80	2.7082	35.2153	0.0769	0.5619	0.1873	36 in Diam	2.9564	4.9819	0.00	
BD13-OL01	122.30	2.9666	24.3008	0.1221	0.7084	0.2361	36 in Diam	2.3269	3.4379	0.00	BD-13
OL01-DM11	224.10	5.5512	37.2525	0.149	0.7804	0.2601	36 in Diam	3.7984	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
weekly	5.5512	5.551	0.5129	6.4475	0.0001	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	5.551	45.7863	0.1212	0.7062	0.2354	36 in Diam	4.3736	6.4774	0.00	
BW18-DM10	5.70	0.1697	12.2362	0.0139	0.124	0.0827	18 in Diam	2.4405	6.9243	0.00	BW-18

DM10-DM08	5.70	5.7072	60.5086	0.0943	0.6225	0.2075	36 in Diam	5.3804	8.5602	0.00	
DM08-DM02	25.60	6.3561	232.00	0.0274	0.57	0.114	60 in Diam	5.1336	11.8157	0.00	DM-08
DM2-DM1	25.60	6.3561	86.5813	0.0734	0.5498	0.1833	36 in Diam	7.1588	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							55.912
No approach losses at node DM-08 because inverts and/or crowns are offset.							
DM-02	DM-01	52.7597	-----	0.2210	-----	52.9807	65.4400
DM-08	DM-02	86.5381	-----	0.0446	-----	86.5827	98.8200
DM-10	DM-08	92.7913	-----	0.1957	0.0076	92.9946	103.2100
DM-11	DM-10	94.0644	--na--	--na--	--na--	94.0644	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	94.2225	0.0841	0.0980	0.0437	94.2801	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	94.6965	0.5821	0.0066	-----	94.1210	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	98.6553	-----	0.5969	0.0636	99.3158	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	120.8204	-----	0.0018	-----	120.8221	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	158.4034	-----	0.0031	-----	158.4066	170.7200
BE-13	BE-06	161.8098	-----	-----	-----	161.8098	193.8900
DM-21	DM-19	120.4063	-----	-----	-----	120.4063	139.6700
BD-13	OL-01	103.2645	-----	-----	-----	103.2645	114.5000
BW-18	DM-10	91.9823	-----	-----	-----	91.9823	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	1.0621	0.3540	6.36	0.7920	0.7920	0.5498	SuperCrit flow, Inlet end controls
DM08-DM02	0.8581	0.1716	6.36	0.6781	0.6781	0.5700	SuperCrit flow, Inlet end controls
DM10-DM08	1.0013	0.3338	5.71	0.9047	0.7495	0.6225	SuperCrit flow, Inlet end controls
DM11-DM10	0.9944	0.3315	5.55	1.2028	0.7387	0.7062	SuperCrit flow, Inlet end controls
OL01-DM11	1.1165	0.3722	5.55	0.7804	0.7387	0.7804	Outlet Control M1 Backwater
DM14-OL01	0.7655	0.2552	2.71	0.5619	0.5119	0.5619	Outlet Control M2 Backwater
DM19-DM14	0.7558	0.3779	2.71	0.5736	0.5736	0.3969	SuperCrit flow, Inlet end controls
BE04-DM19	0.6534	0.3734	1.91	0.4980	0.4980	0.3404	SuperCrit flow, Inlet end controls
BE06-BE04	0.5764	0.3294	1.43	0.4297	0.4297	0.4179	SuperCrit flow, Inlet end controls
BE13-BE06	0.3688	0.2108	0.63	0.2831	0.2831	0.2401	SuperCrit flow, Inlet end controls
DM21-DM19	0.2583	0.2066	0.30	0.2107	0.2107	0.1430	SuperCrit flow, Inlet end controls
BD13-OL01	0.8345	0.2782	2.97	0.7084	0.5369	0.7084	Outlet Control M1 Backwater
BW18-DM10	0.1943	0.1295	0.17	0.1510	0.1510	0.1240	SuperCrit flow, Inlet end controls

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to groundDM-11 to ground: No flow to route											

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.30
DM-11 Dummy	DM-11 to infiltration	0.0000	--na--	--na--	--na--	0.0000	103.0000

Flow does not exceed capacity in any pipes.

The peak flow to the river is at 6.35 cfs (out the Butler outfall).

6 Month Event – 1.26 inches of Rain

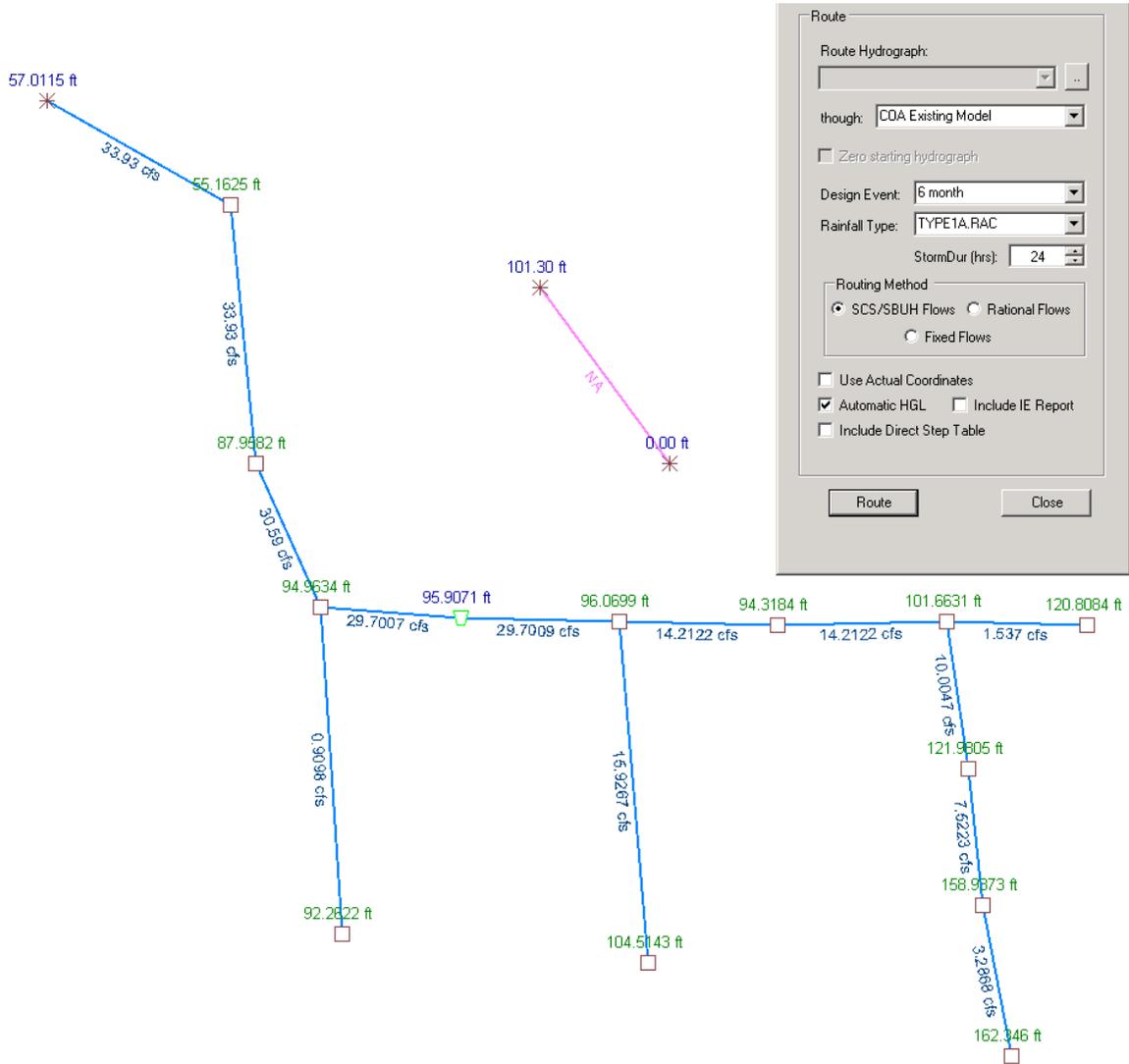


Figure D-3 – Peak Flow Rate Results from 6 Month Event Model

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ROUTEHYD [] THRU [COA Existing Model] USING [6 month] AND [TYPE1A.RAC] NOTZERO RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	3.2868	15.7042	0.2093	0.5435	0.3106	21 in Diam	5.1618	6.529	0.00	BE-13
BE06-BE04	57.50	7.5223	11.4736	0.6556	1.0333	0.5905	21 in Diam	5.0887	4.7701	0.00	BE-06
BE04-DM19	76.80	10.0047	23.0196	0.4346	0.8072	0.4613	21 in Diam	9.2286	9.5704	0.00	BE-04
DM21-DM19	8.90	1.537	10.7468	0.143	0.3188	0.2551	15 in Diam	6.2276	8.7572	0.00	DM-21b
DM19-DM14	101.80	14.2122	31.3855	0.4528	0.9441	0.472	24 in Diam	9.741	9.9903	0.00	DM-21a
DM14-OL01	101.80	14.2122	35.2153	0.4036	1.3273	0.4424	36 in Diam	4.7102	4.9819	0.00	
BD13-OL01	122.30	15.9267	24.3008	0.6554	1.771	0.5903	36 in Diam	3.6672	3.4379	0.00	BD-13
OL01-DM11	224.10	29.7009	37.2525	0.7973	2.025	0.675	36 in Diam	5.8506	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
6 month	29.7009	29.7007	1.5704	19.7401	0.0005	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	29.7007	45.7863	0.6487	1.7594	0.5865	36 in Diam	6.8936	6.4774	0.00	
BW18-DM10	5.70	0.9098	12.2362	0.0744	0.2765	0.1844	18 in Diam	4.0641	6.9243	0.00	BW-18

DM10-DM08	5.70	30.59	60.5086	0.5055	1.5102	0.5034	36 in Diam	8.5807	8.5602	0.00	
DM08-DM02	25.60	33.93	232.00	0.1463	1.289	0.2578	60 in Diam	8.465	11.8157	0.00	DM-08
DM2-DM1	25.60	33.93	86.5813	0.3919	1.3056	0.4352	36 in Diam	11.4913	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							57.0115
No approach losses at node DM-08 because inverts and/or crowns are offset.							
DM-02	DM-01	54.5617	-----	0.6008	-----	55.1625	65.4400
DM-08	DM-02	87.8448	-----	0.1133	-----	87.9582	98.8200
DM-10	DM-08	94.4584	-----	0.4862	0.0188	94.9634	103.2100
DM-11	DM-10	95.9071	--na--	--na--	--na--	95.9071	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	95.9283	0.2088	0.2435	0.1070	96.0699	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	95.7750	1.4734	0.0167	-----	94.3184	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	99.9962	-----	1.5078	0.1591	101.6631	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	121.9762	-----	0.0043	-----	121.9805	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	159.3429	0.4137	0.0082	-----	158.9373	170.7200
BE-13	BE-06	162.3460	-----	-----	-----	162.3460	193.8900
DM-21	DM-19	120.8084	-----	-----	-----	120.8084	139.6700
BD-13	OL-01	104.5143	-----	-----	-----	104.5143	114.5000
BW-18	DM-10	92.2622	-----	-----	-----	92.2622	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	2.8641	0.9547	33.93	1.8915	1.8915	1.3056	SuperCrit flow, Inlet end controls
DM08-DM02	2.1648	0.4330	33.93	1.6203	1.6203	1.2890	SuperCrit flow, Inlet end controls
DM10-DM08	2.6684	0.8895	30.59	2.2802	1.7930	1.5102	SuperCrit flow, Inlet end controls
DM11-DM10	4.1243	1.3748	29.70	3.1716	1.7658	1.7594	Outlet Control
OL01-DM11	2.8223	0.9408	29.70	2.0250	1.7658	2.0250	Outlet Control M1 Backwater
DM14-OL01	1.8440	0.6147	14.21	1.3699	1.2014	1.3273	Outlet Control M1 Backwater
DM19-DM14	2.0967	1.0483	14.21	1.3585	1.3585	0.9441	SuperCrit flow, Inlet end controls
BE04-DM19	1.8092	1.0338	10.00	1.1784	1.1784	0.8072	SuperCrit flow, Inlet end controls
BE06-BE04	1.5159	0.8662	7.52	1.0333	1.0164	1.0333	Outlet Control M2 Backwater
BE13-BE06	0.9050	0.5171	3.29	0.6593	0.6593	0.5435	SuperCrit flow, Inlet end controls
DM21-DM19	0.6604	0.5284	1.54	0.4912	0.4912	0.3188	SuperCrit flow, Inlet end controls
BD13-OL01	2.0843	0.6948	15.93	1.7710	1.2743	1.7710	Outlet Control M2 Backwater
BW18-DM10	0.4742	0.3161	0.91	0.3553	0.3553	0.2765	SuperCrit flow, Inlet end controls

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to groundDM-11 to ground: No flow to route											

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.30
DM-11 Dummy	DM-11 to infiltration	0.0000	--na--	--na--	--na--	0.0000	103.0000

Flow does not exceed capacity in any pipes.

The peak flow to the river is at 29.35 cfs (out the Butler outfall).

2 Year Event – 1.80 inches of Rain

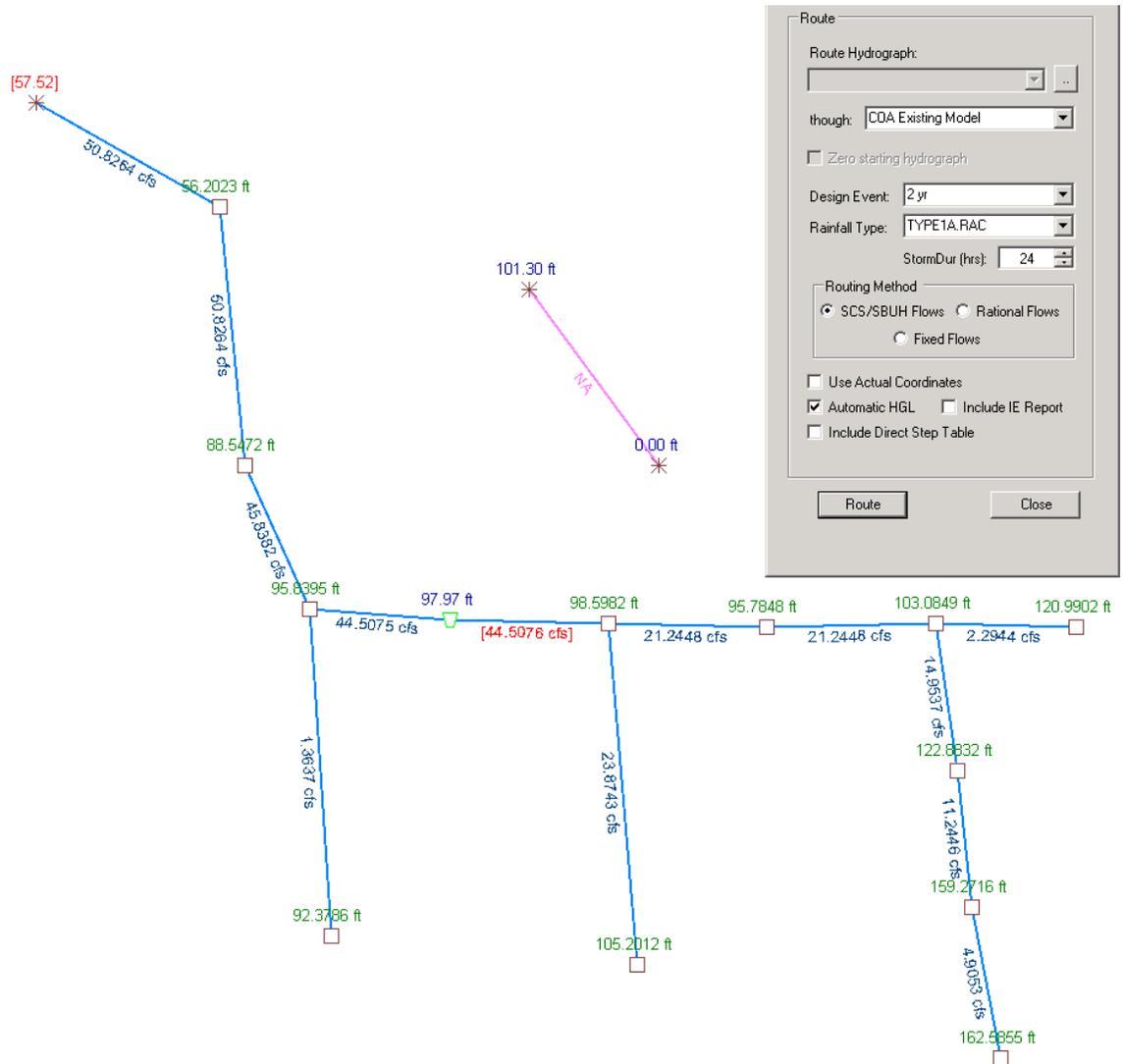


Figure D-4 – Peak Flow Rate Results from 2 Year Event Model

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ROUTEHYD [] THRU [COA Existing Model] USING [2 yr] AND [TYPE1A.RAC] NOTZERO RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	4.9053	15.7042	0.3124	0.67	0.3829	21 in Diam	5.79	6.529	0.00	BE-13
BE06-BE04	57.50	11.2446	11.4736	0.98	1.4046	0.8026	21 in Diam	5.4343	4.7701	0.00	BE-06
BE04-DM19	76.80	14.9537	23.0196	0.6496	1.0272	0.587	21 in Diam	10.1886	9.5704	0.00	BE-04
DM21-DM19	8.90	2.2944	10.7468	0.2135	0.3923	0.3139	15 in Diam	6.9609	8.7572	0.00	DM-21b
DM19-DM14	101.80	21.2448	31.3855	0.6769	1.2066	0.6033	24 in Diam	10.7242	9.9903	0.00	DM-21a
DM14-OL01	101.80	21.2448	35.2153	0.6033	1.6813	0.5604	36 in Diam	5.2111	4.9819	0.00	
BD13-OL01	122.30	23.8743	24.3008	0.9824	2.4139	0.8046	36 in Diam	3.9168	3.4379	0.00	BD-13
OL01-DM11	224.10	44.5076	37.2525	1.1948	-----	na	36 in Diam	6.2965	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
2 yr	44.5076	44.5075	2.0566	25.8517	0.0006	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	44.5075	45.7863	0.9721	2.3877	0.7959	36 in Diam	7.378	6.4774	0.00	
BW18-DM10	5.70	1.3637	12.2362	0.1115	0.3383	0.2255	18 in Diam	4.567	6.9243	0.00	BW-18

DM10-DM08	5.70	45.8382	60.5086	0.7575	1.9519	0.6506	36 in Diam	9.4137	8.5602	0.00	
DM08-DM02	25.60	50.8264	232.00	0.2191	1.591	0.3182	60 in Diam	9.4562	11.8157	0.00	DM-08
DM2-DM1	25.60	50.8264	86.5813	0.587	1.6527	0.5509	36 in Diam	12.7337	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							57.4399
No approach losses at node DM-08 because inverts and/or crowns are offset.							
DM-02	DM-01	55.4525	-----	0.7498	-----	56.2023	65.4400
DM-08	DM-02	88.4107	-----	0.1364	-----	88.5472	98.8200
DM-10	DM-08	95.4182	-----	0.4056	0.0157	95.8395	103.2100
DM-11	DM-10	97.9699	--na--	--na--	--na--	97.9699	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	98.4369	0.2382	0.2777	0.1218	98.5982	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	97.5504	1.7858	0.0203	-----	95.7848	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	101.0533	-----	1.8378	0.1937	103.0849	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	122.8783	-----	0.0049	-----	122.8832	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	159.7818	0.5206	0.0103	-----	159.2716	170.7200
BE-13	BE-06	162.5855	-----	-----	-----	162.5855	193.8900
DM-21	DM-19	120.9902	-----	-----	-----	120.9902	139.6700
BD-13	OL-01	105.2012	-----	-----	-----	105.2012	114.5000
BW-18	DM-10	92.3786	-----	-----	-----	92.3786	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	3.7549	1.2516	50.83	2.3199	2.3199	1.6527	SuperCrit flow, Inlet end controls
DM08-DM02	2.7307	0.5461	50.83	1.9995	1.9995	1.5910	SuperCrit flow, Inlet end controls
DM10-DM08	3.6282	1.2094	45.84	2.8692	2.2066	1.9519	SuperCrit flow, Inlet end controls
DM11-DM10	6.1872	2.0624	44.51	4.0477	2.1749	2.3877	Outlet Control
OL01-DM11	6.1915	2.0638	44.51	4.0399	2.1749	>D	Outlet Control
DM14-OL01	4.3884	1.4628	21.24	3.8982	1.4815	1.6813	Outlet Control
DM19-DM14	3.1538	1.5769	21.24	1.6503	1.6503	1.2066	SuperCrit flow, Inlet end controls
BE04-DM19	2.7113	1.5493	14.95	1.4331	1.4331	1.0272	SuperCrit flow, Inlet end controls
BE06-BE04	1.9548	1.1171	11.24	1.4046	1.2504	1.4046	Outlet Control M1 Backwater
BE13-BE06	1.1445	0.6540	4.91	0.8121	0.8121	0.6700	SuperCrit flow, Inlet end controls
DM21-DM19	0.8422	0.6737	2.29	0.6054	0.6054	0.3923	SuperCrit flow, Inlet end controls
BD13-OL01	2.7712	0.9237	23.87	2.4139	1.5750	2.4139	Outlet Control M1 Backwater
BW18-DM10	0.5906	0.3937	1.36	0.4376	0.4376	0.3383	SuperCrit flow, Inlet end controls

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to groundDM-11 to ground: No flow to route											

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.30
DM-11 Dummy	DM-11 to infiltration	0.0000	--na--	--na--	--na--	0.0000	103.0000

Flow does exceed capacity in one of the upstream pipes; however, that pipe is an existing condition which is already identified by the City of Arlington as a possible future improvement.

The peak flow to the river is at 50.82 cfs (out the Butler outfall).

10 Year Event – 2.75 inches of Rain

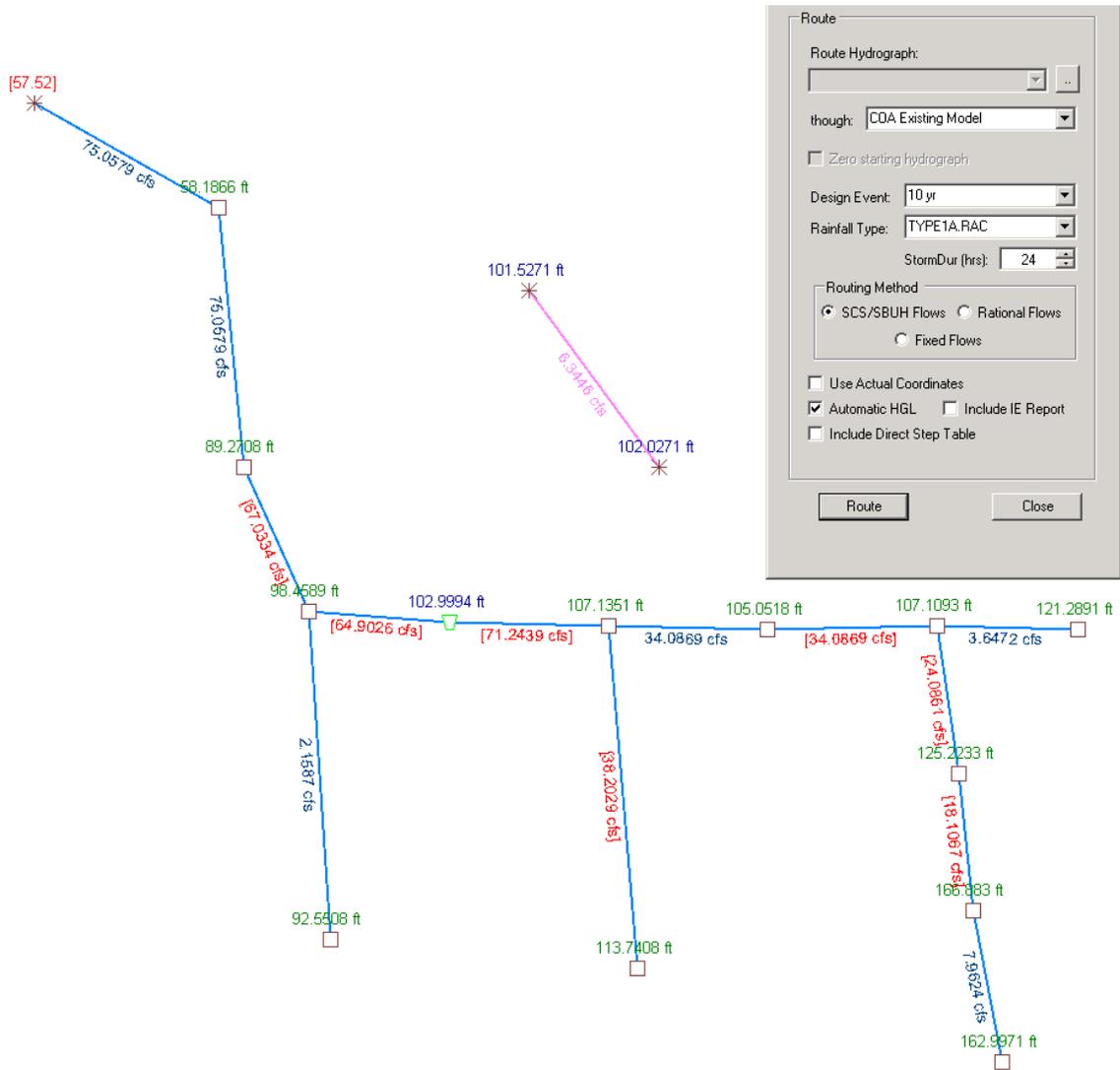


Figure D-5 – Peak Flow Rate Results from 10 Year Event Model

Appended on: Friday, November 13, 2009 3:23:50 PM

ROUTEHYD [] THRU [COA Existing Model] USING [10 yr] AND [TYPE1A.RAC] NOTZERO RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	7.9624	15.7042	0.507	0.8825	0.5043	21 in Diam	6.5492	6.529	0.00	BE-13
BE06-BE04	57.50	18.1067	11.4736	1.5781	-----	na	21 in Diam	7.5279	4.7701	0.00	BE-06
BE04-DM19	76.80	24.0861	23.0196	1.0463	1.5205	0.8689	21 in Diam	10.8536	9.5704	0.00	BE-04
DM21-DM19	8.90	3.6472	10.7468	0.3394	0.5019	0.4016	15 in Diam	7.9154	8.7572	0.00	DM-21b
DM19-DM14	101.80	34.0869	31.3855	1.0861	-----	na	24 in Diam	10.8502	9.9903	0.00	DM-21a
DM14-OL01	101.80	34.0869	35.2153	0.968	2.3773	0.7924	36 in Diam	5.6743	4.9819	0.00	
BD13-OL01	122.30	38.2029	24.3008	1.5721	-----	na	36 in Diam	5.4046	3.4379	0.00	BD-13
OL01-DM11	224.10	71.2439	37.2525	1.9125	-----	na	36 in Diam	10.0789	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
10 yr	71.2439	71.2473	3.5178	44.2192	0.001	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	64.9026	45.7863	1.4175	-----	na	36 in Diam	9.1818	6.4774	0.00	
BW18-DM10	5.70	2.1587	12.2362	0.1764	0.4259	0.284	18 in Diam	5.226	6.9243	0.00	BW-18

DM10-DM08	5.70	67.0334	60.5086	1.1078	-----	na	36 in Diam	9.4833	8.5602	0.00	
DM08-DM02	25.60	75.0579	232.00	0.3235	1.9411	0.3882	60 in Diam	10.6522	11.8157	0.00	DM-08
DM2-DM1	25.60	75.0579	86.5813	0.8669	2.16	0.72	36 in Diam	13.776	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							57.84
No approach losses at node DM-08 because inverts and/or crowns are offset.							
DM-02	DM-01	57.2352	-----	0.9514	-----	58.1866	65.4400
DM-08	DM-02	89.1324	-----	0.1384	-----	89.2708	98.8200
DM-10	DM-08	98.8693	1.3091	0.8625	0.0363	98.4589	103.2100
DM-11	DM-10	102.9994	--na--	--na--	--na--	102.9994	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	106.8276	0.4536	0.5288	0.2323	107.1351	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	106.8591	1.8281	0.0208	-----	105.0518	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	106.6358	1.8292	2.0856	0.2172	107.1093	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	125.2139	-----	0.0094	-----	125.2233	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	167.5358	0.6660	0.0132	-----	166.8830	170.7200
BE-13	BE-06	162.9971	-----	-----	-----	162.9971	193.8900
DM-21	DM-19	121.2891	-----	-----	-----	121.2891	139.6700
BD-13	OL-01	113.7408	-----	-----	-----	113.7408	114.5000
BW-18	DM-10	92.5508	-----	-----	-----	92.5508	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
DM2-DM1	5.5376	1.8459	75.06	2.7200	2.7200	2.1600	SuperCrit flow, Inlet end controls				
DM08-DM02	3.4524	0.6905	75.06	3.0506	2.4501	1.9411	SuperCrit flow, Inlet end controls				
DM10-DM08	13.2111	4.4037	67.03	3.5928	2.6166	>D	Outlet Control				
DM11-DM10	11.2166	3.7389	64.90	6.6671	2.5841	>D	Outlet Control				
OL01-DM11	14.5822	4.8607	71.24	9.0694	2.6742	>D	Outlet Control				
DM14-OL01	13.6971	4.5657	34.09	12.4351	1.8961	2.3773	Outlet Control				
DM19-DM14	31.0199	15.5099	34.09	2.0000	1.9117	>D	Outlet Control				
BE04-DM19	5.0469	2.8840	24.09	1.7500	1.6689	1.5205	SuperCrit flow, Inlet end controls				
BE06-BE04	14.3422	8.1955	18.11	1.7500	1.5482	>D	Outlet Control				
BE13-BE06	1.5561	0.8892	7.96	1.0468	1.0468	0.8825	SuperCrit flow, Inlet end controls				
DM21-DM19	1.1411	0.9129	3.65	0.7724	0.7724	0.5019	SuperCrit flow, Inlet end controls				
BD13-OL01	16.4988	5.4996	38.20	3.0000	2.0123	>D	Outlet Control				
BW18-DM10	0.7628	0.5085	2.16	0.5549	0.5549	0.4259	SuperCrit flow, Inlet end controls				

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to ground											
DM-11 to ground	0.00	6.3446	----	0.00	0.103	----	Ditch	5.9736	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.5271
DM-11 Dummy	DM-11 to infiltration	102.0271	--na--	--na--	--na--	102.0271	103.0000

Flow does exceed capacity in some of the upstream conveyance system; however, that upstream conveyance system is an existing condition which is already identified by the City of Arlington as a future improvement.

The peak flow to the river is at 75.06 cfs (out the Butler outfall).

100 Year Event – 3.75 inches of Rain

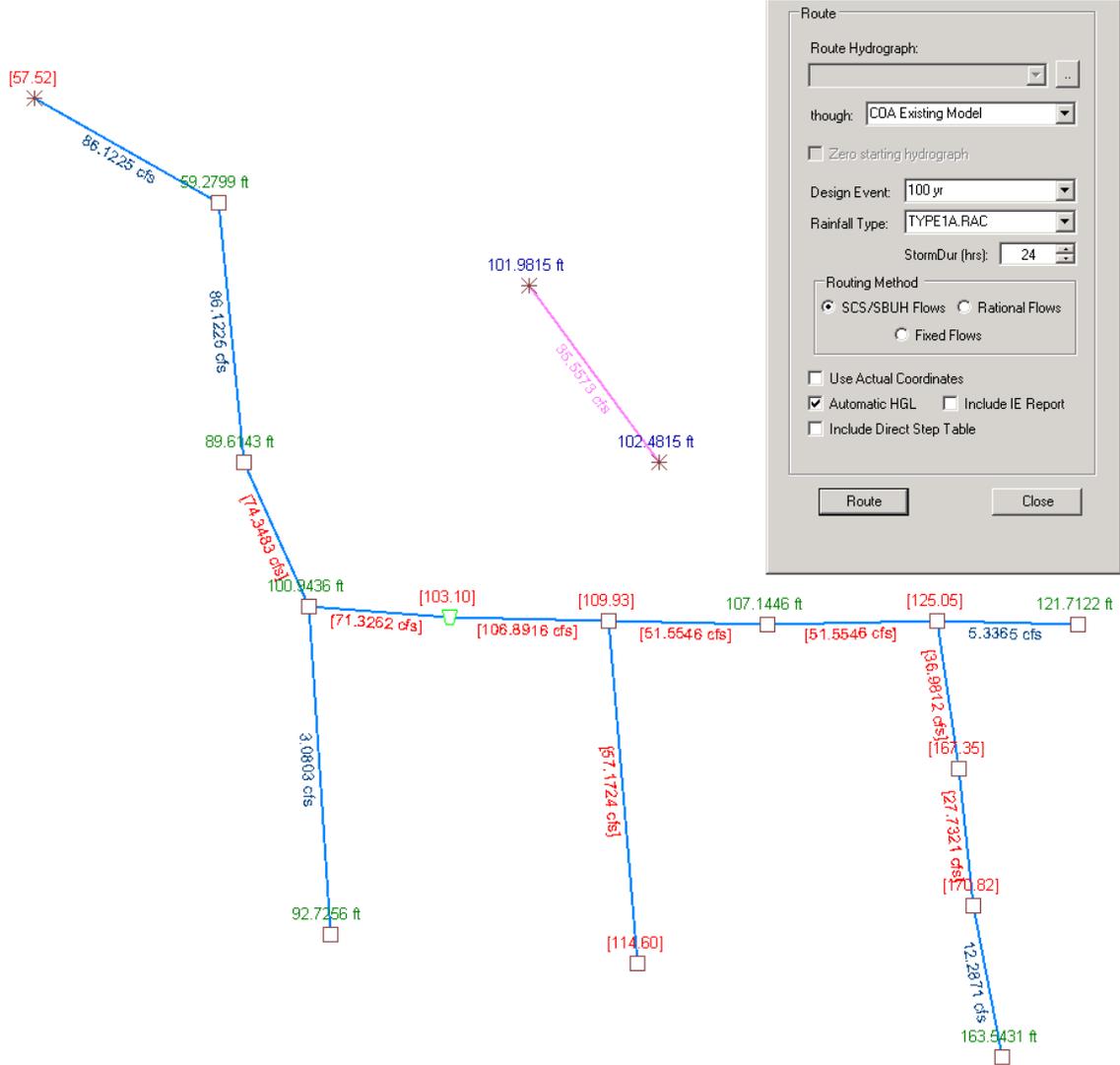


Figure D-6 – Peak Flow Rate Results from 100 Year Event Model

Appended on: Friday, November 13, 2009 3:33:07 PM

ROUTEHYD [] THRU [COA Existing Model] USING [100 yr] AND [TYPE1A.RAC] NOTZERO RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	12.2871	15.7042	0.7824	1.1655	0.666	21 in Diam	7.2216	6.529	0.00	BE-13
BE06-BE04	57.50	27.7321	11.4736	2.417	-----	na	21 in Diam	11.5297	4.7701	0.00	BE-06
BE04-DM19	76.80	36.9812	23.0196	1.6065	-----	na	21 in Diam	15.375	9.5704	0.00	BE-04
DM21-DM19	8.90	5.3365	10.7468	0.4966	0.6226	0.4981	15 in Diam	8.7403	8.7572	0.00	DM-21b
DM19-DM14	101.80	51.5545	31.3855	1.6426	-----	na	24 in Diam	16.4103	9.9903	0.00	DM-21a
DM14-OL01	101.80	51.5545	35.2153	1.464	-----	na	36 in Diam	7.2935	4.9819	0.00	
BD13-OL01	122.30	57.1724	24.3008	2.3527	-----	na	36 in Diam	8.0882	3.4379	0.00	BD-13
OL01-DM11	224.10	106.89	37.2525	2.8694	-----	na	36 in Diam	15.1221	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
100 yr	106.8917	106.8834	4.2487	53.4056	0.0012	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	71.3262	45.7863	1.5578	-----	na	36 in Diam	10.0906	6.4774	0.00	
BW18-DM10	5.70	3.0803	12.2362	0.2517	0.5129	0.342	18 in Diam	5.7684	6.9243	0.00	BW-18

DM10-DM08	5.70	74.3483	60.5086	1.2287	-----	na	36 in Diam	10.5181	8.5602	0.00	
DM08-DM02	25.60	86.1225	232.00	0.3712	2.1102	0.422	60 in Diam	10.934	11.8157	0.00	DM-08
DM2-DM1	25.60	86.1225	86.5813	0.9947	2.4453	0.8151	36 in Diam	13.9599	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							57.9406
No approach losses at node DM-08 because inverts and/or crowns are offset.							
DM-02	DM-01	58.2774	-----	1.0025	-----	59.2799	65.4400
DM-08	DM-02	89.4440	-----	0.1703	-----	89.6143	98.8200
DM-10	DM-08	101.4264	1.5811	1.0417	0.0566	100.9436	103.2100
DM-11	DM-10	106.4291	--na--	--na--	--na--	106.4291	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	117.1544	1.0158	1.1843	0.5240	109.9300	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	111.2788	4.1817	0.0475	-----	107.1446	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	143.9984	3.6707	4.1852	0.4168	125.0500	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	185.4115	2.0642	0.0220	-----	167.3500	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	188.6221	0.8098	0.0160	-----	170.8200	170.7200
BE-13	BE-06	163.5431	-----	-----	-----	163.5431	193.8900
DM-21	DM-19	121.7122	-----	-----	-----	121.7122	139.6700
BD-13	OL-01	130.4745	-----	-----	-----	114.6000	114.5000
BW-18	DM-10	92.7256	-----	-----	-----	92.7256	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	6.5798	2.1933	86.12	2.8206	2.8206	2.4453	SuperCrit flow, Inlet end controls
DM08-DM02	3.7640	0.7528	86.12	4.1439	2.6331	2.1102	SuperCrit flow, Inlet end controls
DM10-DM08	15.7682	5.2561	74.35	3.9363	2.7120	>D	Outlet Control
DM11-DM10	14.6464	4.8821	71.33	9.1518	2.6753	>D	Outlet Control
OL01-DM11	24.9090	8.3030	106.89	12.4991	2.9177	>D	Outlet Control
DM14-OL01	18.1168	6.0389	51.55	15.2300	2.3358	>D	Outlet Control
DM19-DM14	68.3825	34.1913	51.55	2.0000	>D	>D	Outlet Control
BE04-DM19	101.9417	58.2524	36.98	1.7500	>D	>D	Outlet Control
BE06-BE04	35.4284	20.2448	27.73	5.8900	1.7017	>D	Outlet Control
BE13-BE06	2.1021	1.2012	12.29	1.3068	1.3068	1.1655	SuperCrit flow, Inlet end controls
DM21-DM19	1.5642	1.2513	5.34	0.9368	0.9368	0.6226	SuperCrit flow, Inlet end controls
BD13-OL01	33.2325	11.0775	57.17	3.0000	2.4497	>D	Outlet Control
BW18-DM10	0.9376	0.6251	3.08	0.6677	0.6677	0.5129	SuperCrit flow, Inlet end controls

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to ground											
DM-11 to ground	0.00	35.5573	----	0.00	0.2866	----	Ditch	11.426	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.9815
DM-11 Dummy	DM-11 to infiltration	102.4815	--na--	--na--	--na--	102.4815	103.0000

Flow does exceed capacity in some of the upstream conveyance system; however, that upstream conveyance system is an existing condition which is already identified by the City of Arlington as a future improvement.

The peak flow to the river is at 86.12 cfs (out the Butler outfall).

Layout Report: COA Existing Model

Event	Precip (in)
weekly	0.38
6 month	1.26
2 yr	1.80
10 yr	2.75
25 yr	3.20
100 yr	3.75

Reach Records

Record Id: BD13-OL01

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	OL-01	UpNode	BD-13
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	2594.00 ft	Slope	0.20%
Up Invert	102.43 ft	Dn Invert	107.67 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BE04-DM19

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-19	UpNode	BE-04
Material	unspecified	Size	21 in Diam
Ent Losses	Headwall		
Length	1154.00 ft	Slope	3.18%
Up Invert	120.167 ft	Dn Invert	156.83 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BE06-BE04

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	BE-04	UpNode	BE-06
Material	unspecified	Size	21 in Diam
Ent Losses	Groove End Projecting		
Length	586.50 ft	Slope	0.79%
Up Invert	157.827 ft	Dn Invert	161.46 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BE13-BE06

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	BE-06	UpNode	BE-13
Material	unspecified	Size	21 in Diam
Ent Losses	Groove End w/Headwall		
Length	1838.20 ft	Slope	1.48%
Up Invert	161.441 ft	Dn Invert	188.61 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BW18-DM10

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-10	UpNode	BW-18
Material	unspecified	Size	18 in Diam
Ent Losses	Headwall		
Length	1297.40 ft	Slope	1.15%
Up Invert	91.788 ft	Dn Invert	106.76 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM08-DM02

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.024
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-02	UpNode	DM-08
Material	unspecified	Size	60 in Diam
Ent Losses	Headwall		
Length	1134.20 ft	Slope	2.69%
Up Invert	85.68 ft	Dn Invert	55.136 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM10-DM08

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-08	UpNode	DM-10
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	494.50 ft	Slope	1.24%
Up Invert	91.79 ft	Dn Invert	85.678 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM-11 to ground

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift		DM-11 overflow
DnNode	DM-11 to infiltration	UpNode	DM-11 Dummy
Length	10.00 ft	Slope	5.00%
Bottom Width	10.00 ft	Top of Bank	3.00 ft
SS1	3.00v:1h	SS2	3.00v:1h
Up Invert	101.80 ft	Dn Invert	101.30 ft

Record Id: DM11-DM10

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift		DM-11 outlet
DnNode	DM-10	UpNode	DM-11
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	181.30 ft	Slope	0.71%
Up Invert	93.07 ft	Dn Invert	91.7918 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM14-OL01

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	OL-01	UpNode	DM-14
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	183.10 ft	Slope	0.42%
Up Invert	93.931 ft	Dn Invert	94.70 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM19-DM14

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-14	UpNode	DM-19
Material	unspecified	Size	24 in Diam
Ent Losses	Headwall		
Length	768.40 ft	Slope	2.90%
Up Invert	97.8995 ft	Dn Invert	120.16 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM21-DM19

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-19	UpNode	DM-21
Material	unspecified	Size	15 in Diam
Ent Losses	Headwall		
Length	268.50 ft	Slope	4.17%
Up Invert	120.148 ft	Dn Invert	131.35 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM2-DM1

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.011
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-01	UpNode	DM-02
Material	unspecified	Size	36 in Diam
Ent Losses	Groove End w/Headwall		
Length	285.20 ft	Slope	1.20%
Up Invert	51.6976 ft	Dn Invert	55.12 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: OL01-DM11

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-11	UpNode	OL-01
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	183.10 ft	Slope	0.47%
Up Invert	93.106 ft	Dn Invert	93.93 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Node Records

Record Id: BD-13

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	107.67 ft	Max El.	114.50 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BE-04

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	156.83 ft	Max El.	167.25 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BE-06

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	161.46 ft	Max El.	170.72 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BE-13

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	188.61 ft	Max El.	193.89 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BW-18

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	106.76 ft	Max El.	110.77 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: DM-01

Descrip:	Butler Outfall	Increment	0.10 ft
Start El.	51.70 ft	Max El.	57.42 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: DM-02

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	55.12 ft	Max El.	65.44 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: DM-08

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	85.68 ft	Max El.	98.82 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 2-60
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	19.634 sf
MH/CB Type Node			

Record Id: DM-10

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	91.79 ft	Max El.	103.21 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 2-60
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	19.634 sf
MH/CB Type Node			

Record Id: DM-11

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
Void Ratio	100.00		
Storage Node	DM-11 Combo	Discharge Structure	DM-11 Storage
Detention Pond Type Node			

Record Id: DM-11 Storage

Descrip:	pipe end	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
Void Ratio	100.00		
Stage (ft)		Area (sf)	
0.00		12.57	
4.93		12.57	
8.23		12.57	
8.43		80.00	
9.73		160.00	
9.93		180.00	
Stage-Storage Type Node			

Record Id: DM-11 Dummy

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	101.80 ft	Max El.	103.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: DM-11 to infiltration

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	101.30 ft	Max El.	104.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: DM-14

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	94.70 ft	Max El.	111.08 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: DM-19

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	120.16 ft	Max El.	124.95 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: DM-21

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	131.35 ft	Max El.	139.67 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: OL-01

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	93.93 ft	Max El.	109.83 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Discharge Records

Record Id: DM-11 Combo

Combination Discharge Structure			
Descrip:	combined pipe outlet and curb overflow	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
List of Controls		DM-11 outlet DM-11 overflow	

Record Id: DM-11 outlet

Vertical Orifice			
Descrip:	combine infiltration and weir	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
Weir Area	7.069 sf	Weir Coeff	0.61

Record Id: DM-11 overflow

Broad Crested Weir			
Descrip:	combine infiltration and weir	Increment	0.10 ft
Start El.	96.25 ft	Max El.	103.00 ft
Length	10.00 ft	cd	3.216

Contributing Drainage Areas

Record Id: BD-13

Design Method	SCS	Rainfall type	TYPE1A.RAC
Hyd Intv	15.00 min	Peaking Factor	484.00
Storm Duration	11.00 hrs	Abstraction Coeff	0.20
Pervious Area	57.00 ac	DCIA	65.30 ac
Pervious CN	68.00	DC CN	98.00
Pervious TC	40.0494 min	DC TC	25.0767 min

Pervious CN Calc		
Description	SubArea	Sub cn
LAWN - SCS A	57.00 ac	68.00
Pervious Compositied CN (AMC 2)		68.00

Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.2%	0.41	0.00 in	16.1453 min
Shallow	gutter flow	200.00 ft	2.2%	0.0118		0.8261 min
Sheet	pipe conveyance	2670.00 ft	2.2%	0.012	1.80 in	23.078 min
Pervious TC						40.0494 min

DCI - CN Calc		
Description	SubArea	Sub cn
Pavement and rooftops	65.30 ac	98.00
DC Compositied CN (AMC 2)		98.00

DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	Smooth Surfaces.	50.00 ft	2.2%	0.011	1.80 in	0.8932 min
Shallow	Paved	200.00 ft	2.2%	0.01		1.1055 min
Sheet	pipe system	2670.00 ft	2.2%	0.012	1.80 in	23.078 min
Pervious TC						25.0767 min

Record Id: BE-04

Design Method	SCS	Rainfall type	TYPE1A.RAC
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Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	11.00 hrs	Abstraction Coeff	0.20			
Pervious Area	9.90 ac	DCIA	9.40 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	23.9905 min	DC TC	11.0219 min			
Pervious CN Calc						
Description		SubArea	Sub cn			
LAWN - SCS A		9.90 ac	68.00			
Pervious Compositied CN (AMC 2)			68.00			
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	3.3%	0.41	0.00 in	13.7281 min
Shallow	gutter flow	200.00 ft	3.3%	0.01189		0.6796 min
Sheet	pipe conveyance	1090.00 ft	3.3%	0.012	1.80 in	9.5828 min
Pervious TC						23.9905 min
DCI - CN Calc						
Description		SubArea	Sub cn			
Pavement and rooftops		9.40 ac	98.00			
DC Compositied CN (AMC 2)			98.00			
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	3.3%	0.011	0.00 in	0.7594 min
Shallow	gutter flow	200.00 ft	3.3%	0.01189		0.6796 min
Sheet	pipe system	1090.00 ft	3.3%	0.012	1.80 in	9.5828 min
Pervious TC						11.0219 min

Record Id: BE-06

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	11.00 hrs	Abstraction Coeff	0.20			
Pervious Area	17.00 ac	DCIA	16.30 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	31.3508 min	DC TC	15.1775 min			
Pervious CN Calc						
Description		SubArea		Sub cn		
LAWN - SCS A		17.00 ac		68.00		
Pervious Compositd CN (AMC 2)				68.00		
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	1.9%	0.41	0.00 in	17.1204 min
Shallow	gutter flow	200.00 ft	1.9%	0.01189		0.8957 min
Sheet	pipe conveyance	1250.00 ft	1.9%	0.012	1.80 in	13.3347 min
Pervious TC						31.3508 min
DCI - CN Calc						
Description		SubArea		Sub cn		
Pavement and rooftops		16.30 ac		98.00		
DC Compositd CN (AMC 2)				98.00		
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	1.9%	0.011	0.00 in	0.9471 min
Shallow	gutter flow	200.00 ft	1.9%	0.01189		0.8957 min
Sheet	pipe system	1250.00 ft	1.9%	0.012	1.80 in	13.3347 min
Pervious TC						15.1775 min

Record Id: BE-13

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			

Storm Duration	11.00 hrs	Abstraction Coeff	0.20			
Pervious Area	11.80 ac	DCIA	12.40 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	18.5304 min	DC TC	9.0904 min			
Pervious CN Calc						
Description	SubArea	Sub cn				
LAWN - SCS A	11.80 ac	68.00				
Pervious Compositd CN (AMC 2)		68.00				
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	7.3%	0.41	0.00 in	9.9928 min
Shallow	gutter flow	200.00 ft	7.3%	0.01189		0.4569 min
Sheet	pipe conveyance	1310.00 ft	7.3%	0.012	1.80 in	8.0806 min
Pervious TC						18.5304 min
DCI - CN Calc						
Description	SubArea	Sub cn				
Pavement and rooftops	12.40 ac	98.00				
DC Compositd CN (AMC 2)		98.00				
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	7.3%	0.011	0.00 in	0.5528 min
Shallow	gutter flow	200.00 ft	7.3%	0.01189		0.4569 min
Sheet	pipe system	1310.00 ft	7.3%	0.012	1.80 in	8.0806 min
Pervious TC						9.0904 min

Record Id: BW-18

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	24.00 hrs	Abstraction Coeff	0.20			
Pervious Area	2.00 ac	DCIA	3.70 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	54.1477 min	DC TC	23.9846 min			
Pervious CN Calc						
Description		SubArea		Sub cn		
LAWN - SCS A		2.00 ac		68.00		
Pervious Compositd CN (AMC 2)				68.00		
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	0.4%	0.41	0.00 in	31.9295 min
Shallow	gutter flow	200.00 ft	0.4%	0.01189		2.5927 min
Sheet	pipe conveyance	890.00 ft	0.4%	0.012	1.80 in	18.9516 min
Pervious TC						53.4738 min
DCI - CN Calc						
Description		SubArea		Sub cn		
Pavement and rooftops		3.70 ac		98.00		
DC Compositd CN (AMC 2)				98.00		
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	0.4%	0.011	0.00 in	1.7664 min
Shallow	gutter flow	200.00 ft	0.4%	0.01189		2.5927 min
Sheet	pipe system	890.00 ft	0.4%	0.012	1.80 in	18.9516 min
Pervious TC						23.3107 min

Record Id: DM-08

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			

Storm Duration	11.00 hrs	Abstraction Coeff	0.20			
Pervious Area	7.00 ac	DCIA	12.90 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	24.7356 min	DC TC	11.0791 min			
Pervious CN Calc						
Description		SubArea			Sub cn	
LAWN - SCS A		7.00 ac			68.00	
Pervious Compositd CN (AMC 2)					68.00	
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.9%	0.41	0.00 in	14.4563 min
Shallow	gutter flow	200.00 ft	2.9%	0.01189		0.725 min
Sheet	pipe conveyance	1010.00 ft	2.9%	0.0121	0.00 in	2.3535 min
Pervious TC						17.5348 min
DCI - CN Calc						
Description		SubArea			Sub cn	
Pavement and rooftops		12.90 ac			98.00	
DC Compositd CN (AMC 2)					98.00	
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	2.9%	0.011	0.00 in	0.7997 min
Shallow	gutter flow	200.00 ft	2.9%	0.01189		0.725 min
Sheet	pipe system	1010.00 ft	2.9%	0.0121	0.00 in	2.3535 min
Pervious TC						3.8782 min

Record Id: DM-21a

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	11.00 hrs	Abstraction Coeff	0.20			
Pervious Area	5.70 ac	DCIA	10.40 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	32.6143 min	DC TC	17.0757 min			
Pervious CN Calc						
Description		SubArea		Sub cn		
LAWN - SCS A		5.70 ac		68.00		
Pervious Compositd CN (AMC 2)				68.00		
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.1%	0.41	0.00 in	16.4486 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe conveyance	1550.00 ft	2.1%	0.0121	0.00 in	4.2445 min
Pervious TC						21.545 min
DCI - CN Calc						
Description		SubArea		Sub cn		
Pavement and rooftops		10.40 ac		98.00		
DC Compositd CN (AMC 2)				98.00		
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	2.1%	0.011	0.00 in	0.9099 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe system	1550.00 ft	2.1%	0.0121	0.00 in	4.2445 min
Pervious TC						6.0063 min

Record Id: DM-21b

Design Method	SCS	Rainfall type	TYPE1A.RAC
Hyd Intv	15.00 min	Peaking Factor	484.00

Storm Duration	11.00 hrs	Abstraction Coeff	0.20
Pervious Area	3.10 ac	DCIA	5.80 ac
Pervious CN	68.00	DC CN	98.00
Pervious TC	24.658 min	DC TC	9.1194 min

Pervious CN Calc

Description	SubArea	Sub cn
LAWN - SCS A	3.10 ac	68.00
Pervious Compositd CN (AMC 2)		68.00

Pervious TC Calc

Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.1%	0.41	0.00 in	16.4486 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe conveyance	620.00 ft	2.1%	0.0121	0.00 in	1.6978 min
Pervious TC						18.9983 min

DCI - CN Calc

Description	SubArea	Sub cn
Pavement and rooftops	5.80 ac	98.00
DC Compositd CN (AMC 2)		98.00

DCI - TC Calc

Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	2.1%	0.011	0.00 in	0.9099 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe system	620.00 ft	2.1%	0.0121	0.00 in	1.6978 min
Pervious TC						3.4597 min

Hydraulic Model: Post-Project Conditions

**APPENDIX E
DEVELOPED STORMWATER MODEL RESULTS FOR ARLINGTON
CONSTRUCTED WETLAND**

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Developed Stormwater Model

Below is a diagram of the stormwater model showing the labels of various pipes, catch basins, and wetland cells. Note that DM-01 PD is the Butler outfall and that DM-02 PD is the proposed Catch Basin 6A. This note applies to Figures E-2 though E-6.

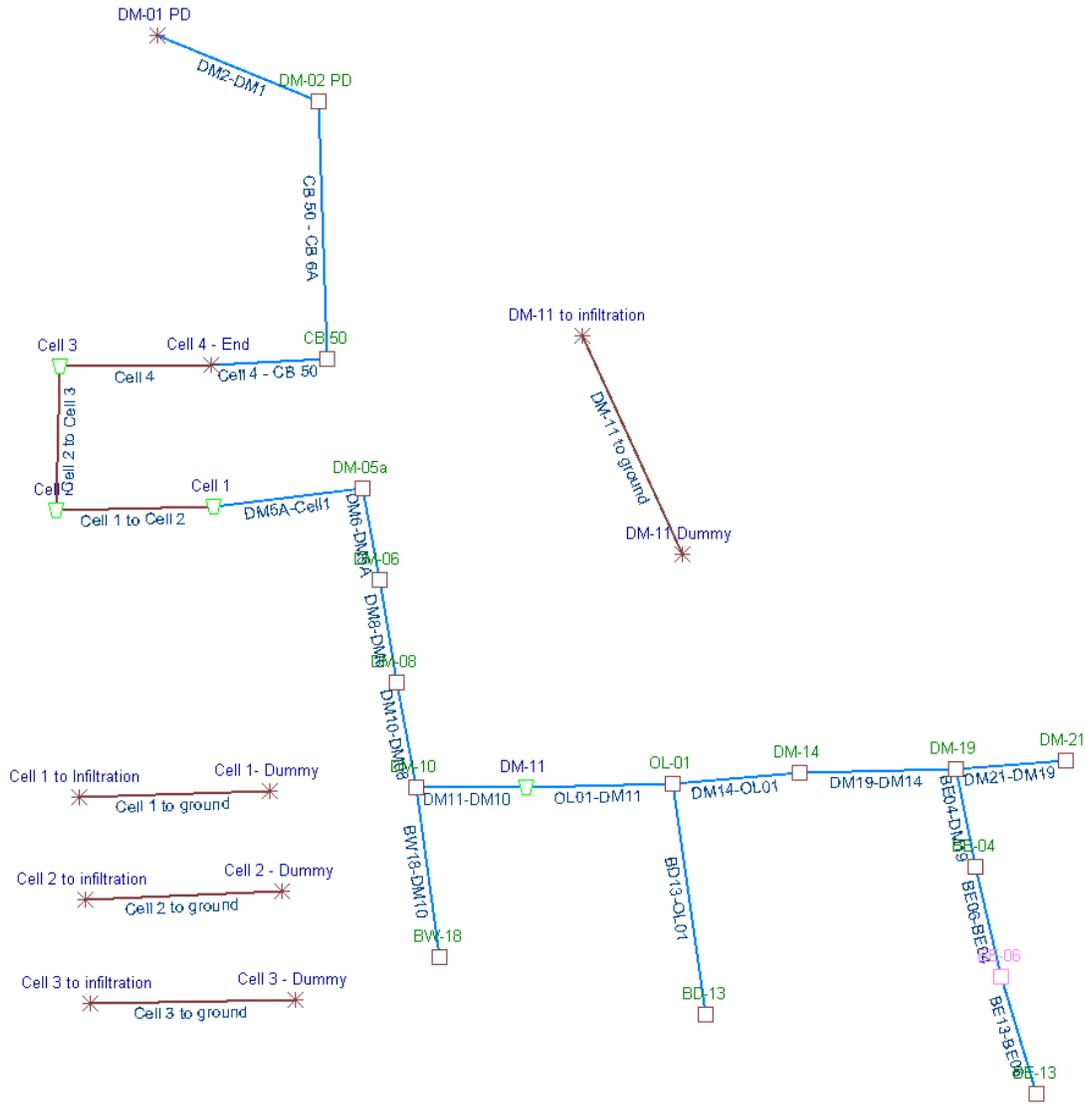


Figure E-1. Stormwater Model

Developed Weekly Event – 0.38 inches of Rain

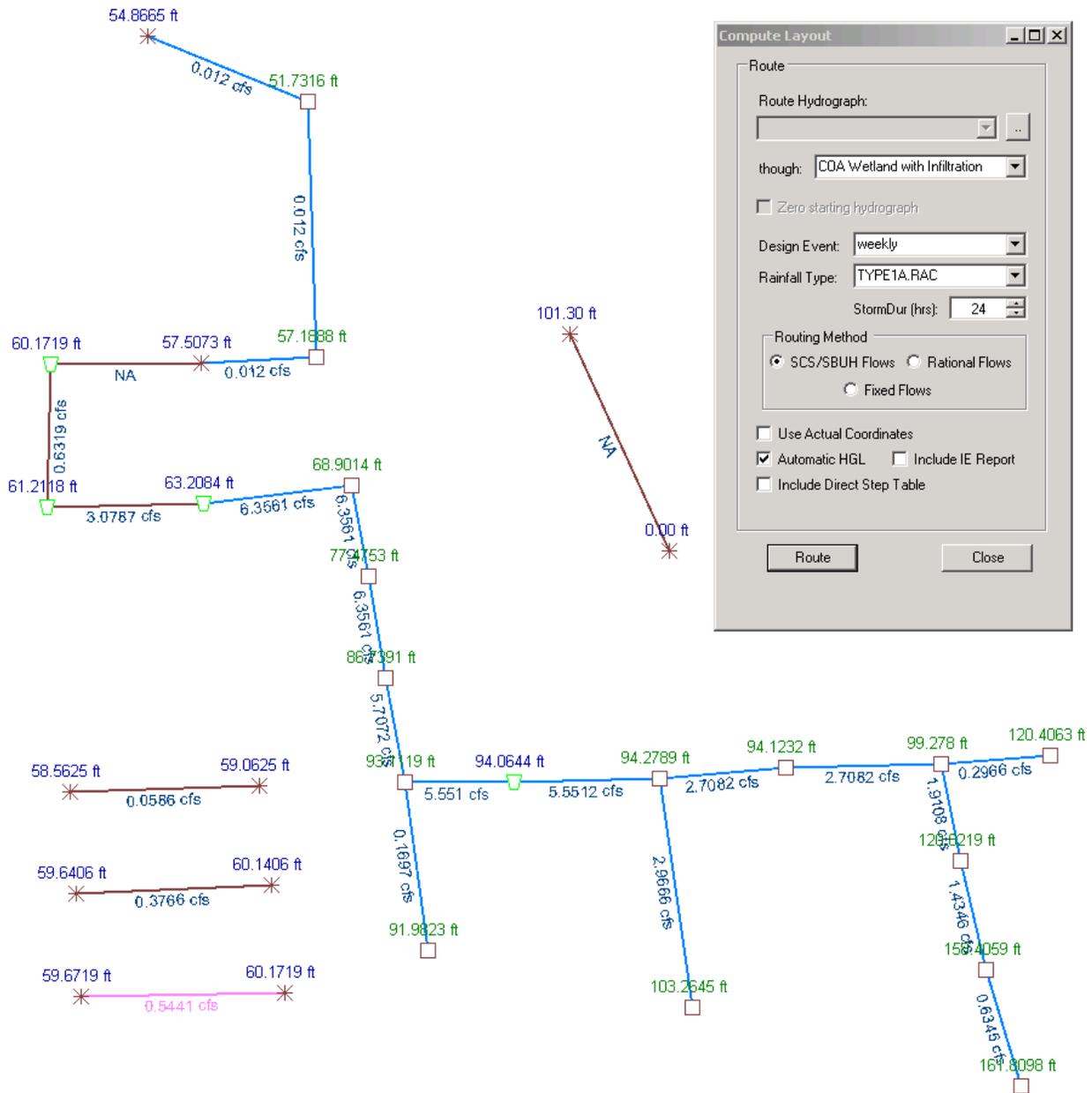


Figure E-2. Peak Flow Rate Results from Weekly Event Model

Appended on: Monday, November 16, 2009 1:58:33 PM

ROUTEHYD [] THRU [COA Wetland with Infiltration] USING [weekly] AND [TYPE1A.RAC] NOTZERO
RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13- BE06	24.20	0.6345	15.7042	0.0404	0.2401	0.1372	21 in Diam	3.193	6.529	0.00	BE-13
BE06- BE04	57.50	1.4346	11.4736	0.125	0.4179	0.2388	21 in Diam	3.2553	4.7701	0.00	BE-06
BE04- DM19	76.80	1.9108	23.0196	0.083	0.3404	0.1945	21 in Diam	5.8066	9.5704	0.00	BE-04
DM21- DM19	8.90	0.2966	10.7468	0.0276	0.143	0.1144	15 in Diam	3.8137	8.7572	0.00	DM-21b
DM19- DM14	101.80	2.7082	31.3855	0.0863	0.3969	0.1984	24 in Diam	6.1228	9.9903	0.00	DM-21a
DM14- OL01	101.80	2.7082	35.2153	0.0769	0.5619	0.1873	36 in Diam	2.9564	4.9819	0.00	
BD13- OL01	122.30	2.9666	24.3008	0.1221	0.7084	0.2361	36 in Diam	2.3269	3.4379	0.00	BD-13
OL01- DM11	224.10	5.5512	37.2525	0.149	0.7804	0.2601	36 in Diam	3.7984	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
weekly	5.5512	5.551	0.5129	6.4475	0.0001	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11- DM10	0.00	5.551	45.7863	0.1212	0.7062	0.2354	36 in Diam	4.3736	6.4774	0.00	
BW18- DM10	5.70	0.1697	12.2362	0.0139	0.124	0.0827	18 in Diam	2.4405	6.9243	0.00	BW-18
DM10-	5.70	5.7072	60.5086	0.0943	0.6225	0.2075	36 in	5.3804	8.5602	0.00	

DM08							Diam				
DM8-DM6	25.60	6.3561	45.8221	0.1387	0.7543	0.2514	36 in Diam	4.5623	6.4825	0.00	DM-08
DM6-DM5A	25.60	6.3561	134.57	0.0472	0.4432	0.1477	36 in Diam	9.7745	19.038	0.00	
DM5A-Cell1	25.60	6.3561	108.10	0.0588	0.6578	0.1645	48 in Diam	4.7067	8.6025	0.00	
LPOOLCOMPUTE [Cell 1] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
weekly	6.3686	3.1372	4.2084	25479.3152	0.5849	188.9544					
Routing split hyd [Cell 1- weir] through Cell 1 to Cell 2											
Cell 1 to Cell 2	0.00	3.0787	----	0.00	0.0327	----	Ditch	15.4319	-----	0.00	Cell 1
LPOOLCOMPUTE [Cell 2] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
weekly	3.1407	1.0085	1.2118	43070.6923	0.9888	0.0059					
Routing split hyd [Cell 2 - weir; Cell 2 -weir - lower] through Cell 2 to Cell 3											
Cell 2 to Cell 3	0.00	0.6319	----	0.00	0.0142	----	Ditch	7.3852	-----	0.00	Cell 2
LPOOLCOMPUTE [Cell 3] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
weekly	0.6488	0.5441	0.1294	5965.838	0.137	0.0059					
Routing split hyd [Cell 3 - weir] through Cell 4Cell 4: No flow to route											
Cell 4 - CB 50	0.41	0.012	109.29	0.0001	0.041	0.0117	42 in Diam	0.5819	11.3591	0.00	Cell 4
CB 50 - CB 6A	0.41	0.012	113.57	0.0001	0.0408	0.0116	42 in Diam	0.5867	11.8047	0.00	
DM2- DM1	0.41	0.012	86.5813	0.0001	0.0365	0.0122	36 in Diam	0.7487	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							54.8665
No approach losses at node CB 50 because inverts and/or crowns are offset.							
DM-02 PD	DM-01 PD	51.7329	0.0053	0.0040	-----	51.7316	67.8500
CB 50	DM-02 PD	57.1872	0.0053	0.0068	-----	57.1888	66.0000
Cell 4 - End	CB 50	57.5073	--na--	--na--	--na--	57.5073	63.0000
Cell 3	Cell 4 - End	0.0000	--na--	--na--	--na--	60.1719	63.5000
Cell 2	Cell 3	60.0684	--na--	--na--	--na--	61.2118	63.5000
Cell 1	Cell 2	63.1948	--na--	--na--	--na--	63.2084	65.5000
DM-05a	Cell 1	67.0718	-----	1.8295	-----	68.9014	77.0000
DM-06	DM-05a	77.4731	-----	0.0022	-----	77.4753	96.2200
DM-08	DM-06	86.7361	-----	0.0029	-----	86.7391	98.8200
DM-10	DM-08	92.7913	-----	0.3130	0.0076	93.1119	103.2100
DM-11	DM-10	94.0644	--na--	--na--	--na--	94.0644	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	94.2225	0.0841	0.0967	0.0437	94.2789	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	94.6965	0.5821	0.0089	-----	94.1232	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	98.6553	-----	0.5591	0.0636	99.2780	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	120.8204	-----	0.0015	-----	120.8219	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	158.4034	-----	0.0025	-----	158.4059	170.7200
BE-13	BE-06	161.8098	-----	-----	-----	161.8098	193.8900
DM-21	DM-19	120.4063	-----	-----	-----	120.4063	139.6700
BD-13	OL-01	103.2645	-----	-----	-----	103.2645	114.5000
BW-18	DM-10	91.9823	-----	-----	-----	91.9823	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
DM2-DM1	0.0469	0.0156	0.01	0.0365	0.0336	0.0365	Outlet Control M1 Backwater				
CB 50 - CB 6A	0.0472	0.0135	0.01	0.0408	0.0000	0.0408	Outlet Control M2 Backwater				
Cell 4 - CB 50	0.0473	0.0135	0.01	0.0488	0.0000	0.0410	Outlet Control M1 Backwater				
Cell 2 to Cell 3	0.0684	na	0.6319	0.0684	0.0684	0.0142	Supercritical flow, dc at up node				
Cell 1 to Cell 2	0.1948	na	3.0787	0.1948	0.1948	0.0327	Supercritical flow, dc at up node				
DM5A-Cell1	0.9718	0.2430	6.36	0.7313	0.7313	0.6578	SuperCrit flow, Inlet end controls				
DM6-DM5A	0.8731	0.2910	6.36	2.6514	0.7920	0.4432	SuperCrit flow, Inlet end controls				
DM8-DM6	1.0561	0.3520	6.36	0.8785	0.7920	0.7543	SuperCrit flow, Inlet end controls				
DM10-DM08	1.0013	0.3338	5.71	1.0611	0.7495	0.6225	SuperCrit flow, Inlet end controls				
DM11-DM10	0.9944	0.3315	5.55	1.3201	0.7387	0.7062	SuperCrit flow, Inlet end controls				
OL01-DM11	1.1165	0.3722	5.55	0.7804	0.7387	0.7804	Outlet Control M1 Backwater				
DM14-OL01	0.7655	0.2552	2.71	0.5619	0.5119	0.5619	Outlet Control M2 Backwater				
DM19-DM14	0.7558	0.3779	2.71	0.5736	0.5736	0.3969	SuperCrit flow, Inlet end controls				
BE04-DM19	0.6534	0.3734	1.91	0.4980	0.4980	0.3404	SuperCrit flow, Inlet end controls				
BE06-BE04	0.5764	0.3294	1.43	0.4297	0.4297	0.4179	SuperCrit flow, Inlet end controls				
BE13-BE06	0.3688	0.2108	0.63	0.2831	0.2831	0.2401	SuperCrit flow, Inlet end controls				
DM21-DM19	0.2583	0.2066	0.30	0.2107	0.2107	0.1430	SuperCrit flow, Inlet end controls				
BD13-OL01	0.8345	0.2782	2.97	0.7084	0.5369	0.7084	Outlet Control M1 Backwater				
BW18-DM10	0.1943	0.1295	0.17	0.1510	0.1510	0.1240	SuperCrit flow, Inlet end controls				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to groundDM-11 to ground: No flow to route											

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.30
DM-11 Dummy	DM-11 to infiltration	0.0000	--na--	--na--	--na--	0.0000	103.0000

Conduit Notes

Reach	HW Depth (ft)		HW/D ratio		Q (cfs)		TW Depth (ft)		Dc (ft)	Dn (ft)	Comment
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 1 - infiltration] through Cell 1 to ground											
Cell 1 to ground	0.00	0.0586	----	0.00	0.0244	----	Ditch	2.2353	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							58.5625
Cell 1- Dummy	Cell 1 to Infiltration	59.0625	--na--	--na--	--na--	59.0625	65.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
Cell 1 to ground	0.0625	na	0.0586	0.0625	0.0625	0.0244	Supercritical flow, dc at up node

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 2 - infiltration] through Cell 2 to ground											
Cell 2 to ground	0.00	0.3766	----	0.00	0.0728	----	Ditch	4.2495	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.6406
Cell 2 - Dummy	Cell 2 to infiltration	60.1406	--na--	--na--	--na--	60.1406	63.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
Cell 2 to ground	0.1406	na	0.3766	0.1406	0.1406	0.0728	Supercritical flow, dc at up node

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 3 - infiltration] through Cell 3 to ground											
Cell 3 to ground	0.00	0.5441	----	0.00	0.1479	----	Ditch	2.5469	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.6719
Cell 3 - Dummy	Cell 3 to infiltration	60.1719	--na--	--na--	--na--	60.1719	62.0000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
Cell 3 to ground	0.1719	na	0.5441	0.1719	0.1719	0.1479	Supercritical flow, dc at up node

Flow does not exceed capacity in any pipes (any pipe in a surcharge state would have its flow contained in brackets on the graphic).

The depth of the peak water storage in Cell 1 is 0.21 feet above the weir notch (63.21). The depth of the peak water storage in Cell 2 is 0.21 feet above the lower weir notch (61.21). This equates to 1.21 feet of water storage in the channel areas and 1.71 feet of water storage in the pool areas. The depth of the water storage in Cell 3 is 1 inch below the weir notch (2 inches deep). The peak flow to the river is reduced from 6.36 cfs (coming into the wetland) to 0.12 cfs (out the Butler outfall). The water discharged out the Butler outfall is as a result of the rainwater which falls on Cell 4.

Developed 6 Month Event – 1.26 inches of Rain

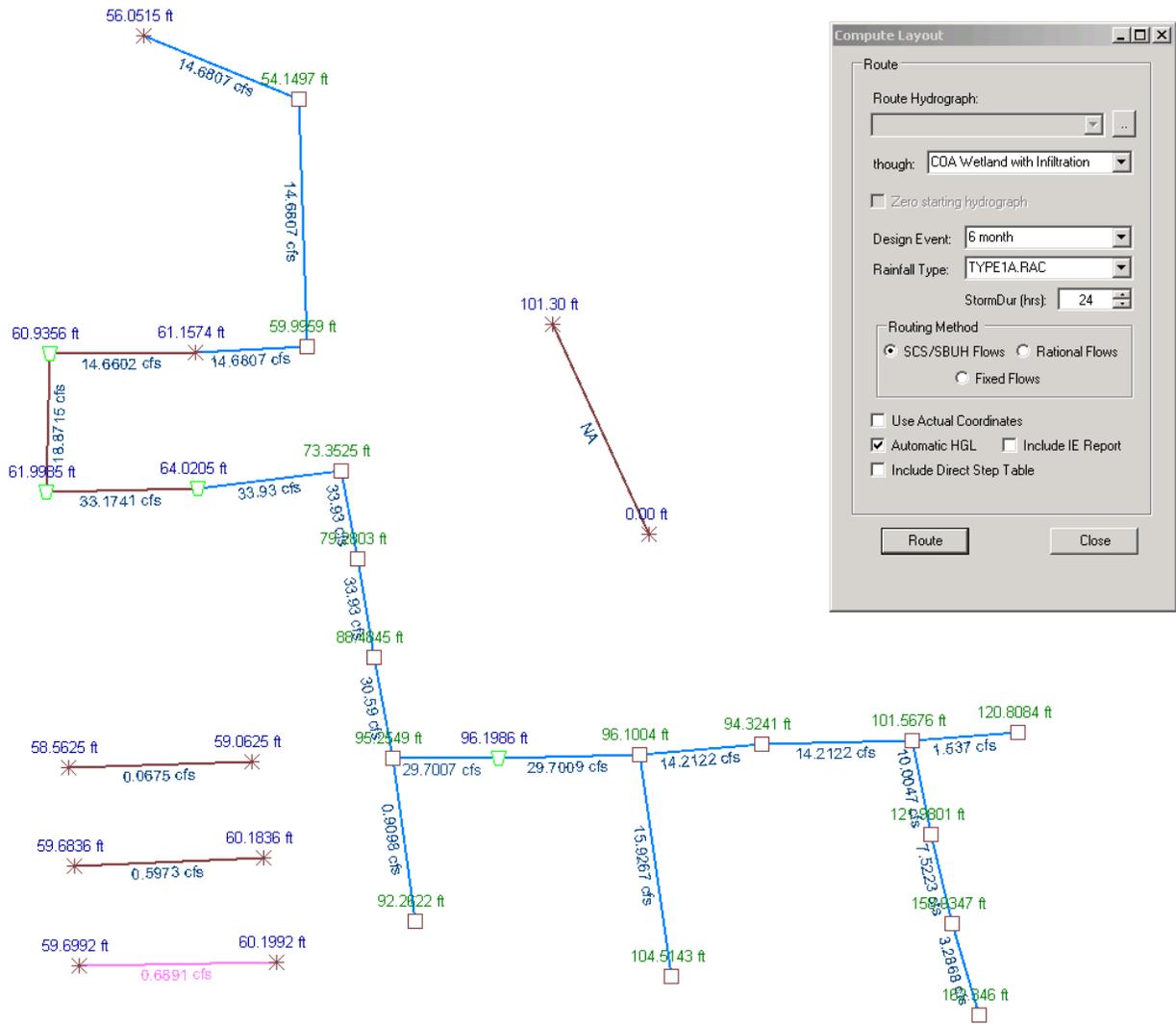


Figure E-3 – Peak Flow Rate Results from 6 Month Event Model

Appended on: Friday, November 20, 2009 11:01:32 AM

ROUTEHYD [] THRU [COA Wetland with Infiltration] USING [6 month] AND [TYPE1A.RAC] NOTZERO
RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	3.2868	15.7042	0.2093	0.5435	0.3106	21 in Diam	5.1618	6.529	0.00	BE-13
BE06-BE04	57.50	7.5223	11.4736	0.6556	1.0333	0.5905	21 in Diam	5.0887	4.7701	0.00	BE-06
BE04-DM19	76.80	10.0047	23.0196	0.4346	0.8072	0.4613	21 in Diam	9.2286	9.5704	0.00	BE-04
DM21-DM19	8.90	1.537	10.7468	0.143	0.3188	0.2551	15 in Diam	6.2276	8.7572	0.00	DM-21b
DM19-DM14	101.80	14.2122	31.3855	0.4528	0.9441	0.472	24 in Diam	9.741	9.9903	0.00	DM-21a
DM14-OL01	101.80	14.2122	35.2153	0.4036	1.3273	0.4424	36 in Diam	4.7102	4.9819	0.00	
BD13-OL01	122.30	15.9267	24.3008	0.6554	1.771	0.5903	36 in Diam	3.6672	3.4379	0.00	BD-13
OL01-DM11	224.10	29.7009	37.2525	0.7973	2.025	0.675	36 in Diam	5.8506	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
6 month	29.7009	29.7007	1.5704	19.7401	0.0005	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	29.7007	45.7863	0.6487	1.7594	0.5865	36 in Diam	6.8936	6.4774	0.00	
BW18-DM10	5.70	0.9098	12.2362	0.0744	0.2765	0.1844	18 in Diam	4.0641	6.9243	0.00	BW-18
DM10-	5.70	30.59	60.5086	0.5055	1.5102	0.5034	36 in	8.5807	8.5602	0.00	

DM08							Diam				
DM8-DM6	25.60	33.93	45.8221	0.7405	1.9232	0.6411	36 in Diam	7.0882	6.4825	0.00	DM-08
DM6-DM5A	25.60	33.93	134.57	0.2521	1.0267	0.3422	36 in Diam	15.8671	19.038	0.00	
DM5A-Cell1	25.60	33.93	108.10	0.3139	1.5343	0.3836	48 in Diam	7.6463	8.6025	0.00	
LPOOLCOMPUTE [Cell 1] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
6 month	34.0107	33.2416	5.0205	34322.5243	0.7879	187.6782					
Routing split hyd [Cell 1- weir] through Cell 1 to Cell 2											
Cell 1 to Cell 2	0.00	33.1741	----	0.00	0.1352	----	Ditch	38.3078	-----	0.00	Cell 1
LPOOLCOMPUTE [Cell 2] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
6 month	33.7305	19.4688	1.9985	114971.9152	2.6394	0.0059					
Routing split hyd [Cell 2 - weir; Cell 2 -weir - lower] through Cell 2 to Cell 3											
Cell 2 to Cell 3	0.00	18.8715	----	0.00	0.1089	----	Ditch	27.378	-----	0.00	Cell 2
LPOOLCOMPUTE [Cell 3] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
6 month	19.1558	15.3214	0.9356	48940.4635	1.1235	0.0059					
Routing split hyd [Cell 3 - weir] through Cell 4											
Cell 4	0.00	14.6601	----	0.00	0.5811	----	Ditch	2.7538	-----	-56059.99	Cell 3
Cell 4 - CB 50	0.41	14.6806	109.29	0.1343	0.8662	0.2475	42 in Diam	7.9172	11.3591	0.00	Cell 4
CB 50 - CB 6A	0.41	14.6806	113.57	0.1293	0.8494	0.2427	42 in Diam	8.1388	11.8047	0.00	
DM2- DM1	0.41	14.6806	86.5813	0.1696	0.8342	0.2781	36 in Diam	9.1489	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							56.0515
No approach losses at node CB 50 because inverts and/or crowns are offset.							
DM-02 PD	DM-01 PD	53.3810	-----	0.7687	-----	54.1497	67.8500
CB 50	DM-02 PD	58.7306	-----	1.2653	-----	59.9959	66.0000
Cell 4 - End	CB 50	61.1574	--na--	--na--	--na--	61.1574	63.0000
Cell 3	Cell 4 - End	59.7911	--na--	--na--	--na--	60.9356	63.5000
Cell 2	Cell 3	60.6064	--na--	--na--	--na--	61.9985	63.5000
Cell 1	Cell 2	63.8469	--na--	--na--	--na--	64.0205	65.5000
DM-05a	Cell 1	68.5313	-----	4.8212	-----	73.3525	77.0000
DM-06	DM-05a	79.2751	-----	0.0052	-----	79.2803	96.2200
DM-08	DM-06	88.7734	0.2908	0.0019	-----	88.4845	98.8200
DM-10	DM-08	94.4584	-----	0.7776	0.0188	95.2549	103.2100
DM-11	DM-10	96.1985	--na--	--na--	--na--	96.1985	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	95.9620	0.2088	0.2403	0.1070	96.1004	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	95.7750	1.4734	0.0225	-----	94.3241	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	99.9962	-----	1.4123	0.1591	101.5676	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	121.9762	-----	0.0039	-----	121.9801	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	159.3429	0.4137	0.0056	-----	158.9347	170.7200
BE-13	BE-06	162.3460	-----	-----	-----	162.3460	193.8900
DM-21	DM-19	120.8084	-----	-----	-----	120.8084	139.6700
BD-13	OL-01	104.5143	-----	-----	-----	104.5143	114.5000
BW-18	DM-10	92.2622	-----	-----	-----	92.2622	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	1.6950	0.5650	14.68	1.2215	1.2215	0.8342	SuperCrit flow, Inlet end controls
CB 50 - CB 6A	1.5906	0.4544	14.68	1.1668	1.1668	0.8494	SuperCrit flow, Inlet end controls
Cell 4 - CB 50	3.6974	1.0564	14.68	2.8559	1.1668	0.8662	Outlet Control M1 Backwater
Cell 4	0.5811	na	14.6602	3.6953	0.4526	0.5811	M1 curve, step from twdepth down to dnDirect Step Backwater Calc
Cell 2 to Cell 3	0.6064	na	18.8715	0.6064	0.6064	0.1089	Supercritical flow, dc at up node
Cell 1 to Cell 2	0.8469	na	33.1741	0.8469	0.8469	0.1352	Supercritical flow, dc at up node
DM5A-Cell1	2.4313	0.6078	33.93	1.7318	1.7318	1.5343	SuperCrit flow, Inlet end controls
DM6-DM5A	2.6751	0.8917	33.93	7.1025	1.8915	1.0267	SuperCrit flow, Inlet end controls
DM8-DM6	3.0934	1.0311	33.93	2.6835	1.8915	1.9232	Outlet Control M1 Backwater
DM10-DM08	2.6684	0.8895	30.59	2.8065	1.7930	1.5102	SuperCrit flow, Inlet end controls
DM11-DM10	4.4158	1.4719	29.70	3.4631	1.7658	1.7594	Outlet Control
OL01-DM11	2.8560	0.9520	29.70	2.2685	1.7658	2.0250	Outlet Control M1 Backwater
DM14-OL01	1.8440	0.6147	14.21	1.4004	1.2014	1.3273	Outlet Control M1 Backwater
DM19-DM14	2.0967	1.0483	14.21	1.3585	1.3585	0.9441	SuperCrit flow, Inlet end controls
BE04-DM19	1.8092	1.0338	10.00	1.1784	1.1784	0.8072	SuperCrit flow, Inlet end controls
BE06-BE04	1.5159	0.8662	7.52	1.0333	1.0164	1.0333	Outlet Control M2 Backwater
BE13-BE06	0.9050	0.5171	3.29	0.6593	0.6593	0.5435	SuperCrit flow, Inlet end controls

DM21-DM19	0.6604	0.5284	1.54	0.4912	0.4912	0.3188	SuperCrit flow, Inlet end controls				
BD13-OL01	2.0843	0.6948	15.93	1.7710	1.2743	1.7710	Outlet Control M2 Backwater				
BW18-DM10	0.4742	0.3161	0.91	0.3553	0.3553	0.2765	SuperCrit flow, Inlet end controls				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to groundDM-11 to ground: No flow to route											

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.30
DM-11 Dummy	DM-11 to infiltration	0.0000	--na--	--na--	--na--	0.0000	103.0000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 1 - infiltration] through Cell 1 to ground											
Cell 1 to ground	0.00	0.0675	----	0.00	0.0269	----	Ditch	2.3252	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							58.5625
Cell 1- Dummy	Cell 1 to Infiltration	59.0625	--na--	--na--	--na--	59.0625	65.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 1 to ground	0.0625	na	0.0675	0.0625	0.0625	0.0269	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 2 - infiltration] through Cell 2 to ground											
Cell 2 to ground	0.00	0.5973	----	0.00	0.0942	----	Ditch	4.9412	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.6836
Cell 2 - Dummy	Cell 2 to infiltration	60.1836	--na--	--na--	--na--	60.1836	63.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 2 to ground	0.1836	na	0.5973	0.1836	0.1836	0.0942	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 3 - infiltration] through Cell 3 to ground											
Cell 3 to ground	0.00	0.6891	----	0.00	0.1675	----	Ditch	2.7386	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.6992

Cell 3 - Dummy	Cell 3 to infiltration	60.1992	--na--	--na--	--na--	60.1992	62.0000
----------------	------------------------	---------	--------	--------	--------	---------	---------

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
Cell 3 to ground	0.1992	na	0.6891	0.1992	0.1992	0.1675	Supercritical flow, dc at up node

Flow does not exceed capacity in any pipes.

The depth of the peak water storage in Cell 1 is 1.02 feet above the weir notch (64.02). The depth of the peak water storage in Cell 2 is 1.0 feet above the lower weir notch and 0.5 feet above the upper weir notch (62.00). This peak storage elevation equates to 2.0 feet of water storage in the channel areas and 2.5 feet of water storage in the pool areas. The depth of the water storage in Cell 3 is 0.17 feet (2 inches). The peak flow to the river is reduced from 33.93 cfs (coming into the wetland) to 14.68 cfs (out the Butler outfall).

Developed 2 Year Event – 1.80 inches of Rain

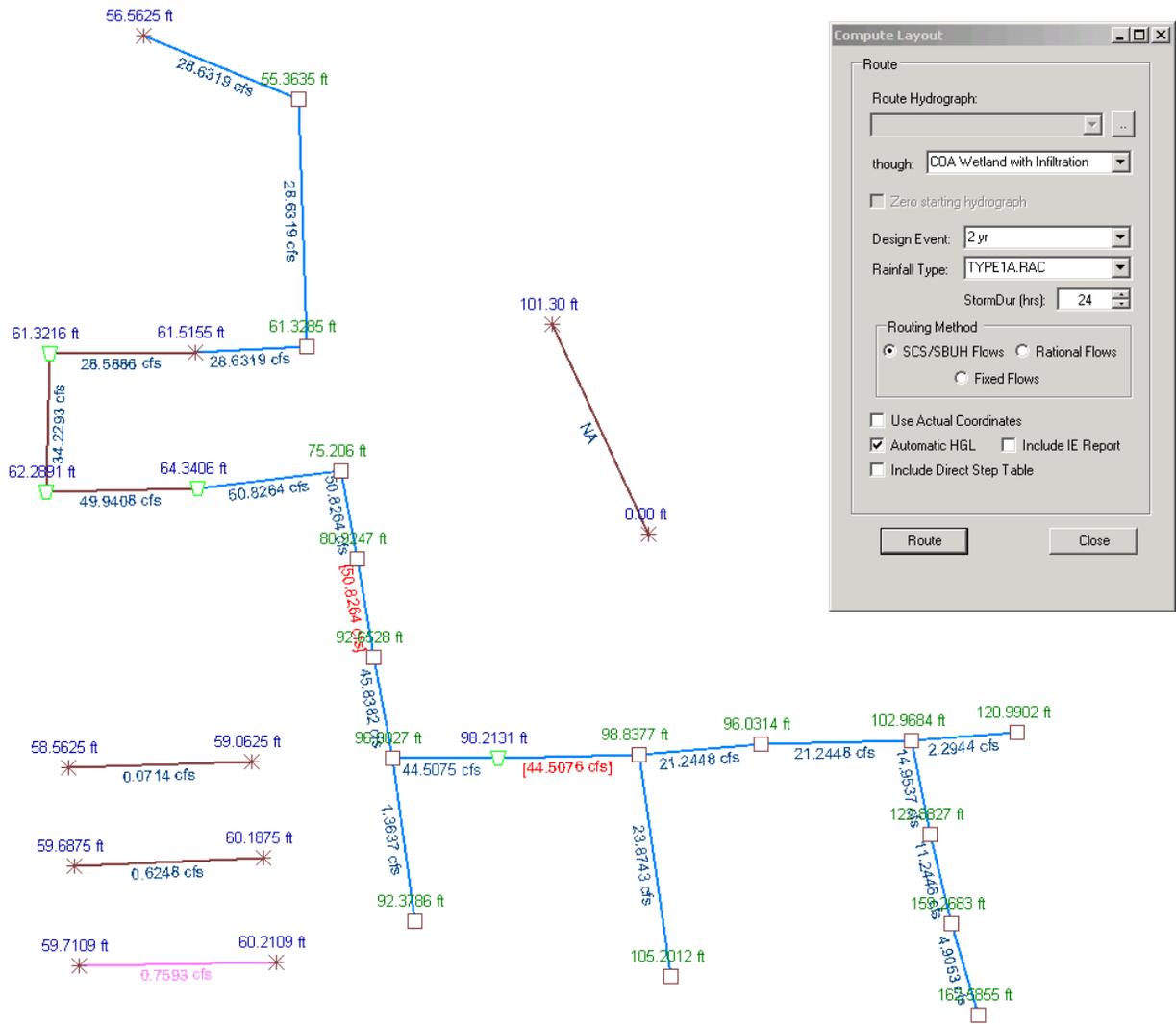


Figure E-4 – Peak Flow Rate Results from 2 Year Event Model

Appended on: Friday, November 20, 2009 11:33:27 AM

ROUTEHYD [] THRU [COA Wetland with Infiltration] USING [2 yr] AND [TYPE1A.RAC] NOTZERO
RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	4.9053	15.7042	0.3124	0.67	0.3829	21 in Diam	5.79	6.529	0.00	BE-13
BE06-BE04	57.50	11.2446	11.4736	0.98	1.4046	0.8026	21 in Diam	5.4343	4.7701	0.00	BE-06
BE04-DM19	76.80	14.9537	23.0196	0.6496	1.0272	0.587	21 in Diam	10.1886	9.5704	0.00	BE-04
DM21-DM19	8.90	2.2944	10.7468	0.2135	0.3923	0.3139	15 in Diam	6.9609	8.7572	0.00	DM-21b
DM19-DM14	101.80	21.2448	31.3855	0.6769	1.2066	0.6033	24 in Diam	10.7242	9.9903	0.00	DM-21a
DM14-OL01	101.80	21.2448	35.2153	0.6033	1.6813	0.5604	36 in Diam	5.2111	4.9819	0.00	
BD13-OL01	122.30	23.8743	24.3008	0.9824	2.4139	0.8046	36 in Diam	3.9168	3.4379	0.00	BD-13
OL01-DM11	224.10	44.5076	37.2525	1.1948	-----	na	36 in Diam	6.2965	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
2 yr	44.5076	44.5075	2.0566	25.8517	0.0006	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	44.5075	45.7863	0.9721	2.3877	0.7959	36 in Diam	7.378	6.4774	0.00	
BW18-DM10	5.70	1.3637	12.2362	0.1115	0.3383	0.2255	18 in Diam	4.567	6.9243	0.00	BW-18
DM10-	5.70	45.8382	60.5086	0.7575	1.9519	0.6506	36 in	9.4137	8.5602	0.00	

DM08							Diam				
DM8-DM6	25.60	50.8264	45.8221	1.1092	-----	na	36 in Diam	7.1905	6.4825	0.00	DM-08
DM6-DM5A	25.60	50.8264	134.57	0.3777	1.2785	0.4262	36 in Diam	17.696	19.038	0.00	
DM5A-Cell1	25.60	50.8264	108.10	0.4702	1.9301	0.4825	48 in Diam	8.4656	8.6025	0.00	
LPOOLCOMPUTE [Cell 1] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
2 yr	50.9666	50.0122	5.3405	38164.0644	0.8761	187.8495					
Routing split hyd [Cell 1- weir] through Cell 1 to Cell 2											
Cell 1 to Cell 2	0.00	49.9408	----	0.00	0.1724	----	Ditch	44.4417	-----	0.00	Cell 1
LPOOLCOMPUTE [Cell 2] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
2 yr	50.8731	34.8541	2.2891	145665.0711	3.344	0.0059					
Routing split hyd [Cell 2 - weir; Cell 2 -weir - lower] through Cell 2 to Cell 3											
Cell 2 to Cell 3	0.00	34.2293	----	0.00	0.1553	----	Ditch	34.0796	-----	0.00	Cell 2
LPOOLCOMPUTE [Cell 3] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
2 yr	34.756	29.3168	1.3216	73086.7271	1.6778	0.0059					
Routing split hyd [Cell 3 - weir] through Cell 4											
Cell 4	0.00	28.5886	----	0.00	0.8548	----	Ditch	3.4445	-----	-63011.23	Cell 3
Cell 4 - CB 50	0.41	28.6319	109.29	0.262	1.2223	0.3492	42 in Diam	9.5697	11.3591	0.00	Cell 4
CB 50 - CB 6A	0.41	28.6319	113.57	0.2521	1.1978	0.3422	42 in Diam	9.8382	11.8047	0.00	
DM2- DM1	0.41	28.6319	86.5813	0.3307	1.1807	0.3936	36 in Diam	11.0815	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							56.5625
No approach losses at node CB 50 because inverts and/or crowns are offset.							
DM-02 PD	DM-01 PD	54.2402	-----	1.1233	-----	55.3635	67.8500
CB 50	DM-02 PD	59.4799	-----	1.8486	-----	61.3285	66.0000
Cell 4 - End	CB 50	61.5155	--na--	--na--	--na--	61.5155	63.0000
Cell 3	Cell 4 - End	60.0648	--na--	--na--	--na--	61.3216	63.5000
Cell 2	Cell 3	60.8627	--na--	--na--	--na--	62.2891	63.5000
Cell 1	Cell 2	64.0715	--na--	--na--	--na--	64.3406	65.5000
DM-05a	Cell 1	69.2093	-----	5.9966	-----	75.2060	77.0000
DM-06	DM-05a	80.9193	-----	0.0054	-----	80.9247	96.2200
DM-08	DM-06	93.3015	0.6530	0.0043	-----	92.6528	98.8200
DM-10	DM-08	95.4182	-----	0.6487	0.0157	96.0827	103.2100
DM-11	DM-10	98.2131	--na--	--na--	--na--	98.2131	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	98.6800	0.2382	0.2741	0.1218	98.8377	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	97.7899	1.7858	0.0273	-----	96.0314	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	101.0533	-----	1.7214	0.1937	102.9684	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	122.8783	-----	0.0044	-----	122.8827	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	159.7818	0.5206	0.0070	-----	159.2683	170.7200
BE-13	BE-06	162.5855	-----	-----	-----	162.5855	193.8900
DM-21	DM-19	120.9902	-----	-----	-----	120.9902	139.6700
BD-13	OL-01	105.2012	-----	-----	-----	105.2012	114.5000
BW-18	DM-10	92.3786	-----	-----	-----	92.3786	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	2.5542	0.8514	28.63	1.7325	1.7325	1.1807	SuperCrit flow, Inlet end controls
CB 50 - CB 6A	2.3399	0.6685	28.63	1.6512	1.6512	1.1978	SuperCrit flow, Inlet end controls
Cell 4 - CB 50	4.3755	1.2501	28.63	4.1885	1.6512	1.2223	Outlet Control
Cell 4	0.8548	na	28.5886	4.0534	0.6918	0.8548	M1 curve, step from twdepth down to dnDirect Step Backwater Calc
Cell 2 to Cell 3	0.8627	na	34.2293	0.8627	0.8627	0.1553	Supercritical flow, dc at up node
Cell 1 to Cell 2	1.0715	na	49.9408	1.0715	1.0715	0.1724	Supercritical flow, dc at up node
DM5A-Cell1	3.1093	0.7773	50.83	2.1404	2.1404	1.9301	SuperCrit flow, Inlet end controls
DM6-DM5A	4.3193	1.4398	50.83	8.9560	2.3199	1.2785	SuperCrit flow, Inlet end controls
DM8-DM6	16.7047	5.5682	50.83	4.3279	2.3199	>D	Outlet Control
DM10-DM08	3.6282	1.2094	45.84	6.9748	2.2066	1.9519	SuperCrit flow, Inlet end controls
DM11-DM10	6.4303	2.1434	44.51	4.2909	2.1749	2.3877	Outlet Control
OL01-DM11	6.4346	2.1449	44.51	4.2831	2.1749	>D	Outlet Control
DM14-OL01	4.6279	1.5426	21.24	4.1377	1.4815	1.6813	Outlet Control
DM19-DM14	3.1538	1.5769	21.24	1.6503	1.6503	1.2066	SuperCrit flow, Inlet end controls
BE04-DM19	2.7113	1.5493	14.95	1.4331	1.4331	1.0272	SuperCrit flow, Inlet end controls
BE06-BE04	1.9548	1.1171	11.24	1.4046	1.2504	1.4046	Outlet Control M1 Backwater
BE13-BE06	1.1445	0.6540	4.91	0.8121	0.8121	0.6700	SuperCrit flow, Inlet end controls

DM21-DM19	0.8422	0.6737	2.29	0.6054	0.6054	0.3923	SuperCrit flow, Inlet end controls				
BD13-OL01	2.7712	0.9237	23.87	2.4139	1.5750	2.4139	Outlet Control M1 Backwater				
BW18-DM10	0.5906	0.3937	1.36	0.4376	0.4376	0.3383	SuperCrit flow, Inlet end controls				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to ground											DM-11 to ground: No flow to route

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.30
DM-11 Dummy	DM-11 to infiltration	0.0000	--na--	--na--	--na--	0.0000	103.0000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 1 - infiltration] through Cell 1 to ground											
Cell 1 to ground	0.00	0.0714	----	0.00	0.0273	----	Ditch	2.4142	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							58.5625
Cell 1- Dummy	Cell 1 to Infiltration	59.0625	--na--	--na--	--na--	59.0625	65.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 1 to ground	0.0625	na	0.0714	0.0625	0.0625	0.0273	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 2 - infiltration] through Cell 2 to ground											
Cell 2 to ground	0.00	0.6248	----	0.00	0.0962	----	Ditch	5.0405	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.6875
Cell 2 - Dummy	Cell 2 to infiltration	60.1875	--na--	--na--	--na--	60.1875	63.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 2 to ground	0.1875	na	0.6248	0.1875	0.1875	0.0962	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 3 - infiltration] through Cell 3 to ground											
Cell 3 to ground	0.00	0.7593	----	0.00	0.1763	----	Ditch	2.8176	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.7109

Cell 3 - Dummy	Cell 3 to infiltration	60.2109	--na--	--na--	--na--	60.2109	62.0000
----------------	------------------------	---------	--------	--------	--------	---------	---------

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
Cell 3 to ground	0.2109	na	0.7593	0.2109	0.2109	0.1763	Supercritical flow, dc at up node

Flow does exceed capacity in some of the upstream basin pipes. These are known deficiencies that the City of Arlington has previously identified for future repair/upgrade. All the proposed project piping is sufficient. A small backwater effect is present from the inlet to the piping draining to the wetland to the weir from Cell 3. The backwater rises to a peak of 0.25' above the weir notch 60. As a result, water levels in Cell 3 may rise to a level slightly higher than modeled but no higher than the peak backwater elevation (60.52'). For a brief time during the peak flows, the two cells (3 and 4) may act like one pond (they would maintain the same elevation), until such time as the backwater effect decreased and regular flow through weir 3 resumes.

The depth of the peak water storage in Cell 1 is 1.34 feet above the weir notch (64.34). The depth of the peak water storage in Cell 2 is 1.29 feet above the lower weir notch and 0.79 feet above the upper weir notch (62.29). This peak storage elevation equates to 2.29 feet of water storage in the channel areas and 2.79 feet of water storage in the pool areas. The peak depth of the water storage in Cell 3 is 1.32 to 1.52' feet. The peak flow to the river is reduced from 50.83 cfs (coming into the wetland) to 28.63 cfs (out the Butler outfall).

Developed 10 Year Event – 2.75 inches of Rain

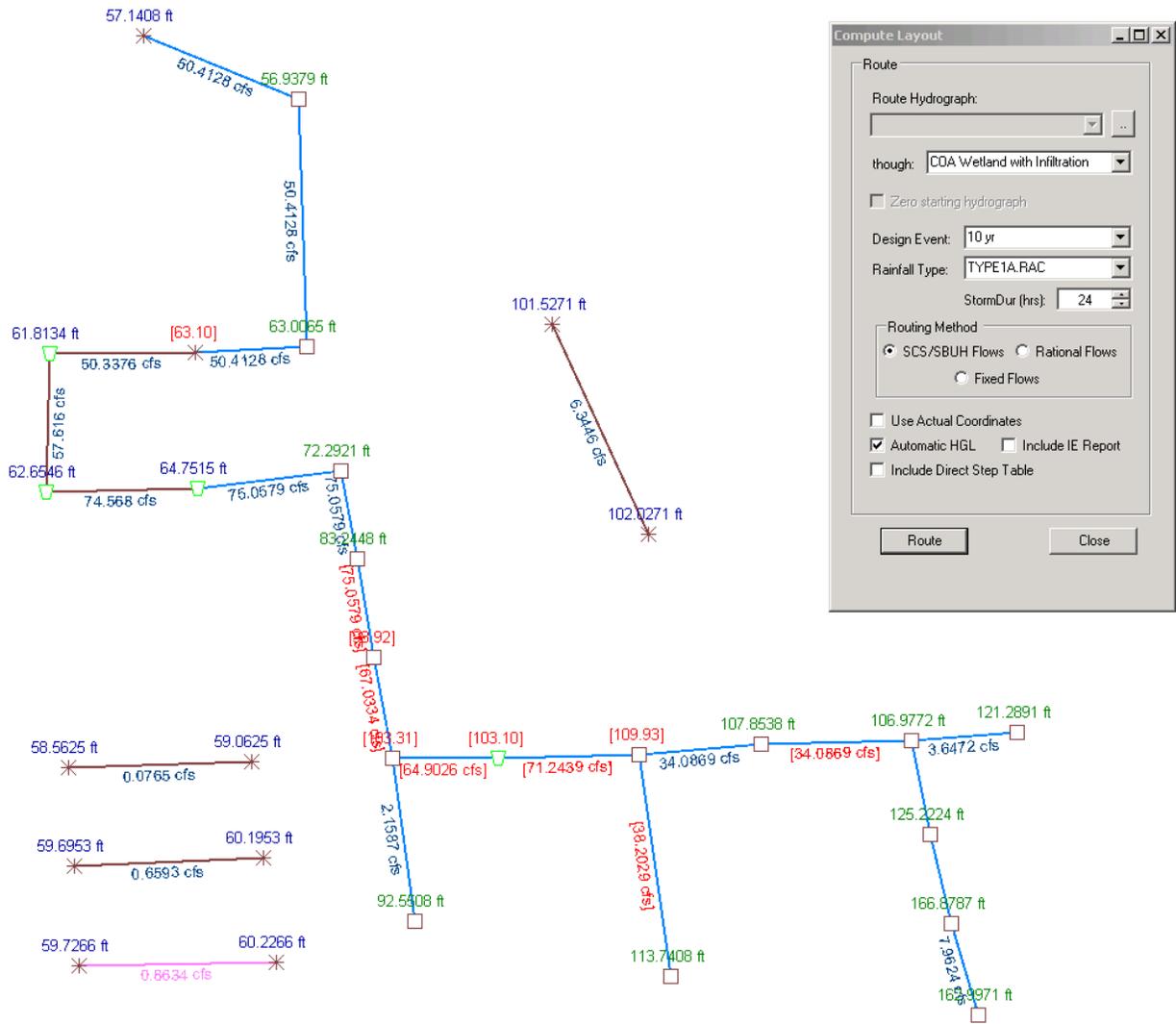


Figure E-5 – Peak Flow Rate Results from 10 Year Event Model

Appended on: Friday, November 20, 2009 11:56:31 AM

ROUTEHYD [] THRU [COA Wetland with Infiltration] USING [10 yr] AND [TYPE1A.RAC] NOTZERO
RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	7.9624	15.7042	0.507	0.8825	0.5043	21 in Diam	6.5492	6.529	0.00	BE-13
BE06-BE04	57.50	18.1067	11.4736	1.5781	-----	na	21 in Diam	7.5279	4.7701	0.00	BE-06
BE04-DM19	76.80	24.0861	23.0196	1.0463	1.5205	0.8689	21 in Diam	10.8536	9.5704	0.00	BE-04
DM21-DM19	8.90	3.6472	10.7468	0.3394	0.5019	0.4016	15 in Diam	7.9154	8.7572	0.00	DM-21b
DM19-DM14	101.80	34.0869	31.3855	1.0861	-----	na	24 in Diam	10.8502	9.9903	0.00	DM-21a
DM14-OL01	101.80	34.0869	35.2153	0.968	2.3773	0.7924	36 in Diam	5.6743	4.9819	0.00	
BD13-OL01	122.30	38.2029	24.3008	1.5721	-----	na	36 in Diam	5.4046	3.4379	0.00	BD-13
OL01-DM11	224.10	71.2439	37.2525	1.9125	-----	na	36 in Diam	10.0789	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
10 yr	71.2439	71.2473	3.5178	44.2192	0.001	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	64.9026	45.7863	1.4175	-----	na	36 in Diam	9.1818	6.4774	0.00	
BW18-DM10	5.70	2.1587	12.2362	0.1764	0.4259	0.284	18 in Diam	5.226	6.9243	0.00	BW-18
DM10-	5.70	67.0334	60.5086	1.1078	-----	na	36 in	9.4833	8.5602	0.00	

DM08							Diam				
DM8-DM6	25.60	75.0579	45.8221	1.638	-----	na	36 in Diam	10.6185	6.4825	0.00	DM-08
DM6-DM5A	25.60	75.0579	134.57	0.5578	1.6023	0.5341	36 in Diam	19.541	19.038	0.00	
DM5A-Cell1	25.60	75.0579	108.10	0.6943	2.4537	0.6134	48 in Diam	9.2869	8.6025	0.00	
LPOOLCOMPUTE [Cell 1] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
10 yr	75.3004	74.6445	5.7515	43614.4512	1.0013	188.0268					
Routing split hyd [Cell 1- weir] through Cell 1 to Cell 2											
Cell 1 to Cell 2	0.00	74.568	----	0.00	0.2182	----	Ditch	51.354	-----	0.00	Cell 1
LPOOLCOMPUTE [Cell 2] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
10 yr	76.2708	58.2753	2.6546	186228.8399	4.2752	0.0059					
Routing split hyd [Cell 2 - weir; Cell 2 -weir - lower] through Cell 2 to Cell 3											
Cell 2 to Cell 3	0.00	57.616	----	0.00	0.2109	----	Ditch	41.1935	-----	0.00	Cell 2
LPOOLCOMPUTE [Cell 3] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
10 yr	58.5072	51.1662	1.8134	107447.4306	2.4667	0.0059					
Routing split hyd [Cell 3 - weir] through Cell 4											
Cell 4	0.00	50.3376	----	0.00	1.1789	----	Ditch	4.1224	-----	-72668.93	Cell 3
Cell 4 - CB 50	0.41	50.4128	109.29	0.4613	1.6702	0.4772	42 in Diam	11.1251	11.3591	0.00	Cell 4
CB 50 - CB 6A	0.41	50.4128	113.57	0.4439	1.6334	0.4667	42 in Diam	11.4499	11.8047	0.00	
DM2- DM1	0.41	50.4128	86.5813	0.5823	1.6442	0.5481	36 in Diam	12.7102	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							57.1408
No approach losses at node CB 50 because inverts and/or crowns are offset.							
DM-02 PD	DM-01 PD	55.4164	-----	1.5215	-----	56.9379	67.8500
CB 50	DM-02 PD	60.5081	-----	2.4984	-----	63.0064	66.0000
Cell 4 - End	CB 50	63.5861	--na--	--na--	--na--	63.1000	63.0000
Cell 3	Cell 4 - End	60.3889	--na--	--na--	--na--	61.8134	63.5000
Cell 2	Cell 3	61.1618	--na--	--na--	--na--	62.6546	63.5000
Cell 1	Cell 2	64.3406	--na--	--na--	--na--	64.7515	65.5000
DM-05a	Cell 1	70.1330	-----	2.1592	-----	72.2921	77.0000
DM-06	DM-05a	83.2331	-----	0.0117	-----	83.2448	96.2200
DM-08	DM-06	110.2360	1.3965	0.0092	-----	98.9200	98.8200
DM-10	DM-08	108.5184	1.3091	1.3795	0.0363	103.3100	103.2100
DM-11	DM-10	107.8504	--na--	--na--	--na--	107.8504	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	111.6787	0.4536	0.5219	0.2323	109.9300	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	109.6540	1.8281	0.0279	-----	107.8538	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	106.6358	1.8292	1.9535	0.2172	106.9772	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	125.2139	-----	0.0085	-----	125.2224	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	167.5358	0.6660	0.0089	-----	166.8787	170.7200
BE-13	BE-06	162.9971	-----	-----	-----	162.9971	193.8900
DM-21	DM-19	121.2891	-----	-----	-----	121.2891	139.6700
BD-13	OL-01	113.7408	-----	-----	-----	113.7408	114.5000
BW-18	DM-10	92.5508	-----	-----	-----	92.5508	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	3.7304	1.2435	50.41	2.3108	2.3108	1.6442	SuperCrit flow, Inlet end controls
CB 50 - CB 6A	3.3681	0.9623	50.41	2.2192	2.2192	1.6334	SuperCrit flow, Inlet end controls
Cell 4 - CB 50	6.4461	1.8417	50.41	5.8664	2.2192	1.6702	Outlet Control
Cell 4	1.1789	na	50.3376	5.6379	0.9824	1.1789	M1 curve, step from twdepth down to dnDirect Step Backwater Calc
Cell 2 to Cell 3	1.1618	na	57.616	1.1618	1.1618	0.2109	Supercritical flow, dc at up node
Cell 1 to Cell 2	1.3406	na	74.568	1.3406	1.3406	0.2182	Supercritical flow, dc at up node
DM5A-Cell1	4.0330	1.0082	75.06	2.6221	2.6221	2.4537	SuperCrit flow, Inlet end controls
DM6-DM5A	6.6331	2.2110	75.06	6.0421	2.7200	1.6023	SuperCrit flow, Inlet end controls
DM8-DM6	33.6392	11.2131	75.06	6.6480	2.7200	>D	Outlet Control
DM10-DM08	22.8602	7.6201	67.03	13.2420	2.6166	>D	Outlet Control
DM11-DM10	16.0677	5.3559	64.90	11.5182	2.5841	>D	Outlet Control
OL01-DM11	19.4332	6.4777	71.24	13.9204	2.6742	>D	Outlet Control
DM14-OL01	16.4920	5.4973	34.09	15.2300	1.8961	2.3773	Outlet Control
DM19-DM14	31.0199	15.5099	34.09	2.0000	1.9117	>D	Outlet Control
BE04-DM19	5.0469	2.8840	24.09	1.7500	1.6689	1.5205	SuperCrit flow, Inlet end controls
BE06-BE04	14.3422	8.1955	18.11	1.7500	1.5482	>D	Outlet Control

BE13- BE06	1.5561	0.8892	7.96	1.0468	1.0468	0.8825	SuperCrit flow, Inlet end controls				
DM21- DM19	1.1411	0.9129	3.65	0.7724	0.7724	0.5019	SuperCrit flow, Inlet end controls				
BD13- OL01	16.4988	5.4996	38.20	3.0000	2.0123	>D	Outlet Control				
BW18- DM10	0.7628	0.5085	2.16	0.5549	0.5549	0.4259	SuperCrit flow, Inlet end controls				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to ground											
DM-11 to ground	0.00	6.3446	----	0.00	0.103	----	Ditch	5.9736	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.5271
DM-11 Dummy	DM-11 to infiltration	102.0271	--na--	--na--	--na--	102.0271	103.0000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
DM-11 to ground	0.2271	na	6.3446	0.2271	0.2271	0.103	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 1 - infiltration] through Cell 1 to ground											
Cell 1 to ground	0.00	0.0765	----	0.00	0.0288	----	Ditch	2.4443	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							58.5625
Cell 1-Dummy	Cell 1 to Infiltration	59.0625	--na--	--na--	--na--	59.0625	65.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 1 to ground	0.0625	na	0.0765	0.0625	0.0625	0.0288	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 2 - infiltration] through Cell 2 to ground											
Cell 2 to ground	0.00	0.6593	----	0.00	0.0991	----	Ditch	5.1269	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.6953
Cell 2 - Dummy	Cell 2 to infiltration	60.1953	--na--	--na--	--na--	60.1953	63.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 2 to ground	0.1953	na	0.6593	0.1953	0.1953	0.0991	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 3 - infiltration] through Cell 3 to ground											

Cell 3 to ground	0.00	0.8634	----	0.00	0.189	----	Ditch	2.9162	-----	0.00	
------------------	------	--------	------	------	-------	------	-------	--------	-------	------	--

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.7266
Cell 3 - Dummy	Cell 3 to infiltration	60.2266	--na--	--na--	--na--	60.2266	62.0000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
Cell 3 to ground	0.2266	na	0.8634	0.2266	0.2266	0.189	Supercritical flow, dc at up node

Flow does exceed capacity in some of the upstream basin pipes. These are known deficiencies that the City of Arlington has previously identified for future repair/upgrade. All the proposed project piping passes the peak storm flows. A backwater effect is present from the piping draining the wetland to the river. The software limitations prevent modeling the backwater situation accurately beyond the outfall piping. See section 3.1.6 for a full discussion of how backwater from the river outfall occurs.

The actual depths of water stored in the Cells will depend largely on how fast the river rises and falls in relation to the storm event. However, for large storm events, it is more likely than not the backwater will occur from the wetland discharge piping to the river, and that discharge to the WSDOT wetland through the overflow spillway will occur. This is similar to what occurs in the existing condition, when stormwater flows exceed the piping capacity of the Butler outfall or the river is up, and the stormwater flow coming from the downtown basin is largely discharged onto the surface of the subject property from the WSDOT bioswale and CB 6. The stormwater then flows over the surface of the property south to the WSDOT wetland. Allowing the wetland to backwater and flood to the WSDOT wetland when necessary mimics the existing condition to a certain extent. Since the creation of the wetland adds 9 acre feet of additional water storage to the property, it takes more water to create flow to the WSDOT wetland than in the existing condition.

The model gives the following results; however, they do not reflect the backwater from the outfall as discussed above. It is likely that during peak discharge Cells 2, 3, and 4 will all fill to an elevation slightly above 62.5 and that stormwater will be discharged to the WSDOT wetland. The model gives depth of the peak water storage in Cell 1 is 1.75 feet above the weir notch (64.75).

This cell is unaffected by the backwater effects. The depth of the peak water storage in Cell 2 is modeled at 1.65 feet above the lower weir notch and 1.15 feet above the upper weir notch (62.65). This peak storage elevation equates to 2.65 feet of water storage in the channel areas and 3.15 feet of water storage in the pool areas. At an elevation of 62.5, discharge will begin into the WSDOT wetland. The peak depth of the water storage in Cell 3 is 1.81 to 3 feet. The peak flow to the river is modeled as being reduced from 75.05 cfs (coming into the wetland) to 50.41 cfs (out the Butler outfall). Actual peak discharge flows to the river are likely to be lower due to backwater effects. Some of the peak discharge reduction is additional storage in the wetlands and some is discharge to the WSDOT wetland.

Developed 100 Year Event – 3.75 inches of Rain

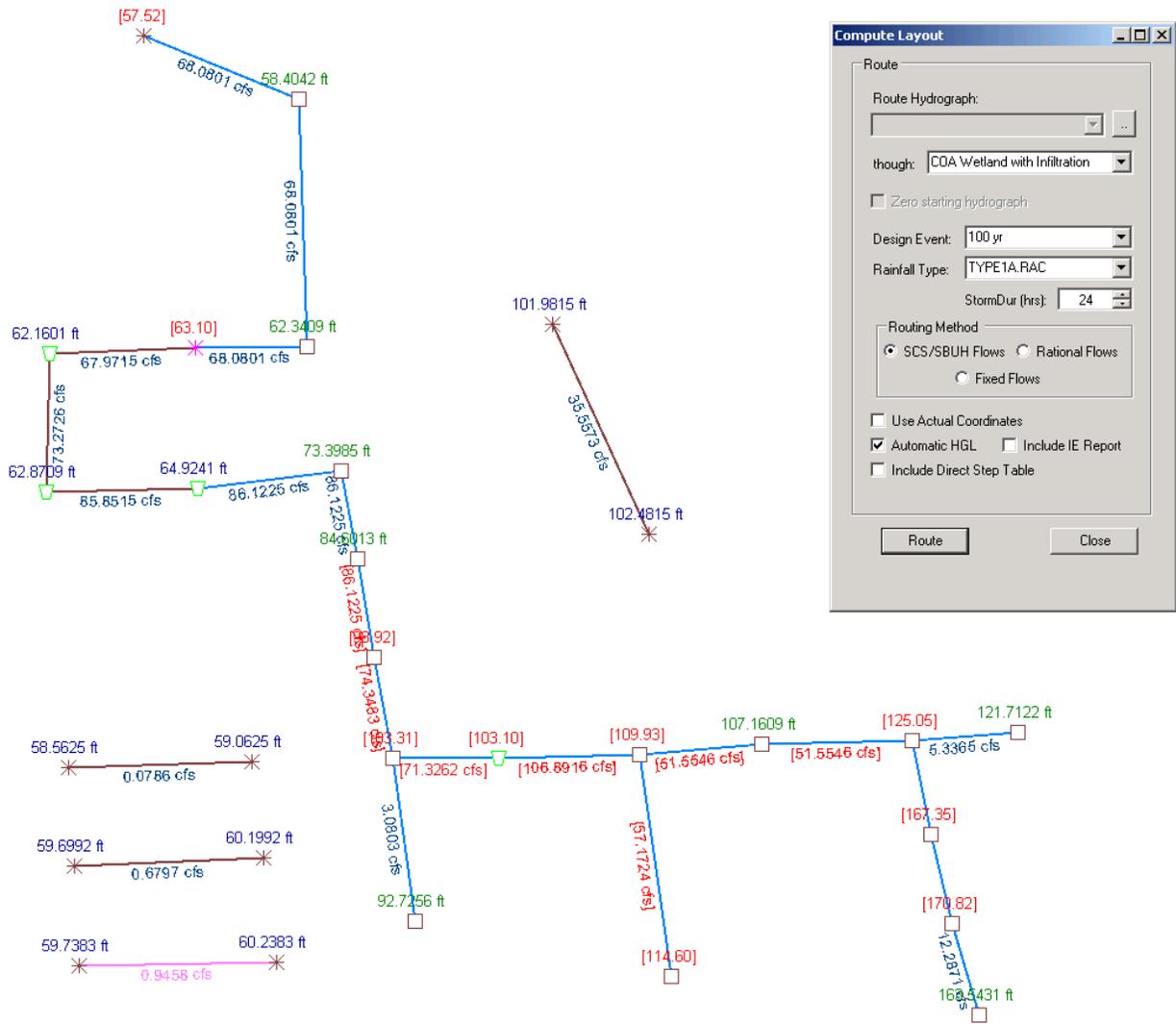


Figure E-6 – Peak Flow Rate Results from 100 Year Event Model

Appended on: Friday, November 20, 2009 2:40:22 PM

ROUTEHYD [] THRU [COA Wetland with Infiltration] USING [100 yr] AND [TYPE1A.RAC] NOTZERO
RELATIVE SCS/SBUH

Gravity Analysis using 24 hr duration storm

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
BE13-BE06	24.20	12.2871	15.7042	0.7824	1.1655	0.666	21 in Diam	7.2216	6.529	0.00	BE-13
BE06-BE04	57.50	27.7321	11.4736	2.417	-----	na	21 in Diam	11.5297	4.7701	0.00	BE-06
BE04-DM19	76.80	36.9812	23.0196	1.6065	-----	na	21 in Diam	15.375	9.5704	0.00	BE-04
DM21-DM19	8.90	5.3365	10.7468	0.4966	0.6226	0.4981	15 in Diam	8.7403	8.7572	0.00	DM-21b
DM19-DM14	101.80	51.5545	31.3855	1.6426	-----	na	24 in Diam	16.4103	9.9903	0.00	DM-21a
DM14-OL01	101.80	51.5545	35.2153	1.464	-----	na	36 in Diam	7.2935	4.9819	0.00	
BD13-OL01	122.30	57.1724	24.3008	2.3527	-----	na	36 in Diam	8.0882	3.4379	0.00	BD-13
OL01-DM11	224.10	106.89	37.2525	2.8694	-----	na	36 in Diam	15.1221	5.2701	0.00	
LPOOLCOMPUTE [DM-11] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
100 yr	106.8917	106.8834	4.2487	53.4056	0.0012	0.0107					
Routing split hyd [DM-11 outlet] through DM11-DM10											
DM11-DM10	0.00	71.3262	45.7863	1.5578	-----	na	36 in Diam	10.0906	6.4774	0.00	
BW18-DM10	5.70	3.0803	12.2362	0.2517	0.5129	0.342	18 in Diam	5.7684	6.9243	0.00	BW-18
DM10-	5.70	74.3483	60.5086	1.2287	-----	na	36 in	10.5181	8.5602	0.00	

DM08							Diam				
DM8-DM6	25.60	86.1225	45.8221	1.8795	-----	na	36 in Diam	12.1838	6.4825	0.00	DM-08
DM6-DM5A	25.60	86.1225	134.57	0.64	1.7443	0.5814	36 in Diam	20.1981	19.038	0.00	
DM5A-Cell1	25.60	86.1225	108.10	0.7967	2.6985	0.6746	48 in Diam	9.5487	8.6025	0.00	
LPOOLCOMPUTE [Cell 1] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
100 yr	86.4777	85.9302	5.9241	45988.5482	1.0558	188.1391					
Routing split hyd [Cell 1- weir] through Cell 1 to Cell 2											
Cell 1 to Cell 2	0.00	85.8515	----	0.00	0.2365	----	Ditch	54.1008	-----	0.00	Cell 1
LPOOLCOMPUTE [Cell 2] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
100 yr	88.5068	73.9523	2.8709	211250.301	4.8496	0.0059					
Routing split hyd [Cell 2 - weir; Cell 2 -weir - lower] through Cell 2 to Cell 3											
Cell 2 to Cell 3	0.00	73.2726	----	0.00	0.2432	----	Ditch	44.7651	-----	0.00	Cell 2
LPOOLCOMPUTE [Cell 3] SUMMARY using Puls, 24 hr Storm Event											
Event	Match Q (cfs)	Peak Q (cfs)	Max Depth (ft)	Vol (cf)	Vol (acft)	Time to Empty					
100 yr	74.5313	68.8799	2.1601	134531.1823	3.0884	0.0059					
Routing split hyd [Cell 3 - weir] through Cell 4											
Cell 4	0.00	67.9715	----	0.00	1.3937	----	Ditch	4.5209	-----	-80169.01	Cell 3
Cell 4 - CB 50	0.41	68.08	109.29	0.6229	2.0006	0.5716	42 in Diam	11.9761	11.3591	0.00	Cell 4
CB 50 - CB 6A	0.41	68.08	113.57	0.5994	1.9536	0.5582	42 in Diam	12.3297	11.8047	0.00	
DM2-DM1	0.41	68.08	86.5813	0.7863	2.0048	0.6683	36 in Diam	13.5628	12.2487	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							57.4624
No approach losses at node CB 50 because inverts and/or crowns are offset.							
DM-02 PD	DM-01 PD	56.6399	-----	1.7643	-----	58.4042	67.8500
CB 50	DM-02 PD	61.2777	-----	1.0632	-----	62.3409	66.0000
Cell 4 - End	CB 50	63.3980	--na--	--na--	--na--	63.1000	63.0000
Cell 3	Cell 4 - End	60.6037	--na--	--na--	--na--	62.1601	63.5000
Cell 2	Cell 3	61.3278	--na--	--na--	--na--	62.8709	63.5000
Cell 1	Cell 2	64.4481	--na--	--na--	--na--	64.9241	65.5000
DM-05a	Cell 1	70.5559	-----	2.8427	-----	73.3985	77.0000
DM-06	DM-05a	84.5859	-----	0.0154	-----	84.6013	96.2200
DM-08	DM-06	120.1369	1.7179	0.0113	-----	98.9200	98.8200
DM-10	DM-08	110.7321	1.5811	1.6661	0.0566	103.3100	103.2100
DM-11	DM-10	108.7955	--na--	--na--	--na--	108.7955	103.0000
No approach losses at node BD-13 because inverts and/or crowns are offset.							
OL-01	DM-11	119.5208	1.0158	1.1689	0.5240	109.9300	109.8300
No approach losses at node DM-19 because inverts and/or crowns are offset.							
DM-14	OL-01	111.2788	4.1817	0.0638	-----	107.1609	111.0800
No approach losses at node BE-04 because inverts and/or crowns are offset.							
DM-19	DM-14	143.9984	3.6707	3.9200	0.4168	125.0500	124.9500
No approach losses at node BE-06 because inverts and/or crowns are offset.							
BE-04	DM-19	185.4115	2.0642	0.0198	-----	167.3500	167.2500
No approach losses at node BE-13 because inverts and/or crowns are offset.							
BE-06	BE-04	188.6221	0.8098	0.0109	-----	170.8200	170.7200
BE-13	BE-06	163.5431	-----	-----	-----	163.5431	193.8900
DM-21	DM-19	121.7122	-----	-----	-----	121.7122	139.6700
BD-13	OL-01	130.4745	-----	-----	-----	114.6000	114.5000
BW-18	DM-10	92.7256	-----	-----	-----	92.7256	110.7700

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
DM2-DM1	4.9539	1.6513	68.08	2.6324	2.6324	2.0048	SuperCrit flow, Inlet end controls
CB 50 - CB 6A	4.1377	1.1822	68.08	3.5754	2.5869	1.9536	SuperCrit flow, Inlet end controls
Cell 4 - CB 50	6.2580	1.7880	68.08	5.2009	2.5869	2.0006	Outlet Control
Cell 4	1.3937	na	67.9715	5.6379	1.1795	1.3937	M1 curve, step from twdepth down to dnDirect Step Backwater Calc
Cell 2 to Cell 3	1.3278	na	73.2726	1.3278	1.3278	0.2432	Supercritical flow, dc at up node
Cell 1 to Cell 2	1.4481	na	85.8515	1.4481	1.4481	0.2365	Supercritical flow, dc at up node
DM5A-Cell1	4.4559	1.1140	86.12	2.8143	2.8143	2.6985	SuperCrit flow, Inlet end controls
DM6-DM5A	7.9859	2.6620	86.12	7.1485	2.8206	1.7443	SuperCrit flow, Inlet end controls
DM8-DM6	43.5401	14.5134	86.12	8.0045	2.8206	>D	Outlet Control
DM10-DM08	25.0739	8.3580	74.35	13.2420	2.7120	>D	Outlet Control
DM11-DM10	17.0128	5.6709	71.33	11.5182	2.6753	>D	Outlet Control
OL01-DM11	27.2754	9.0918	106.89	14.8655	2.9177	>D	Outlet Control
DM14-OL01	18.1168	6.0389	51.55	15.2300	2.3358	>D	Outlet Control
DM19-DM14	68.3825	34.1913	51.55	2.0000	>D	>D	Outlet Control
BE04-DM19	101.9417	58.2524	36.98	1.7500	>D	>D	Outlet Control
BE06-BE04	35.4284	20.2448	27.73	5.8900	1.7017	>D	Outlet Control

BE13- BE06	2.1021	1.2012	12.29	1.3068	1.3068	1.1655	SuperCrit flow, Inlet end controls				
DM21- DM19	1.5642	1.2513	5.34	0.9368	0.9368	0.6226	SuperCrit flow, Inlet end controls				
BD13- OL01	33.2325	11.0775	57.17	3.0000	2.4497	>D	Outlet Control				
BW18- DM10	0.9376	0.6251	3.08	0.6677	0.6677	0.5129	SuperCrit flow, Inlet end controls				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [DM-11 overflow] through DM-11 to ground											
DM-11 to ground	0.00	35.5573	----	0.00	0.2866	----	Ditch	11.426	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							101.9815
DM-11 Dummy	DM-11 to infiltration	102.4815	--na--	--na--	--na--	102.4815	103.0000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
DM-11 to ground	0.6815	na	35.5573	0.6815	0.6815	0.2866	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 1 - infiltration] through Cell 1 to ground											
Cell 1 to ground	0.00	0.0786	----	0.00	0.0293	----	Ditch	2.4672	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							58.5625
Cell 1-Dummy	Cell 1 to Infiltration	59.0625	--na--	--na--	--na--	59.0625	65.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment
Cell 1 to ground	0.0625	na	0.0786	0.0625	0.0625	0.0293	Supercritical flow, dc at up node

Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 2 - infiltration] through Cell 2 to ground											
Cell 2 to ground	0.00	0.6797	----	0.00	0.1011	----	Ditch	5.1603	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.6992
Cell 2 - Dummy	Cell 2 to infiltration	60.1992	--na--	--na--	--na--	60.1992	63.5000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 2 to ground	0.1992	na	0.6797	0.1992	0.1992	0.1011	Supercritical flow, dc at up node				
Reach ID	Area (ac)	Flow (cfs)	Full Q (cfs)	Full ratio	nDepth (ft)	Depth ratio	Size	nVel (ft/s)	fVel (ft/s)	Infil Vol (cf)	CBasin / Hyd
Routing split hyd [Cell 3 - infiltration] through Cell 3 to ground											
Cell 3 to ground	0.00	0.9458	----	0.00	0.1978	----	Ditch	3.0019	-----	0.00	

HGL Analysis

From Node	To Node	HG El (ft)	App (ft)	Bend (ft)	Junct Loss (ft)	Adjusted HG El (ft)	Max El (ft)
							59.7383
Cell 3 - Dummy	Cell 3 to infiltration	60.2383	--na--	--na--	--na--	60.2383	62.0000

Conduit Notes

Reach	HW Depth (ft)	HW/D ratio	Q (cfs)	TW Depth (ft)	Dc (ft)	Dn (ft)	Comment				
Cell 3 to ground	0.2383	na	0.9458	0.2383	0.2383	0.1978	Supercritical flow, dc at up node				

Flow does exceed capacity in some of the upstream basin pipes. These are known deficiencies that the City of Arlington has previously identified for future repair/upgrade. All the proposed project piping passes the peak storm flows. The bore pipe passes the 100 year flow with a headwater depth of 73.4' which is 3.7' below the proposed rim elevation. A backwater effect is present from the piping draining the wetland to the river. The software limitations prevent modeling the backwater situation accurately beyond the outfall piping. See Section 3.1.6 for a full discussion of how backwater from the river outfall occurs.

The actual depths of water stored in the Cells will depend largely on how fast the river rises and falls in relation to the storm event. However, for large storm events, it is more likely than not the

backwater will occur from the wetland discharge piping to the river, and that discharge to the WSDOT wetland through the overflow spillway will occur. This is similar to what occurs in the existing condition, when stormwater flows exceed the piping capacity of the Butler outfall or the river is up, and the stormwater flow coming from the downtown basin is largely discharged onto the surface of the subject property from the WSDOT bioswale and CB 6. The stormwater then flows over the surface of the property south to the WSDOT wetland. Allowing the wetland to backwater and flood to the WSDOT wetland when necessary mimics the existing condition to a certain extent. Since the creation of the wetland adds 9 acre feet of additional water storage to the property, it takes more water to create flow to the WSDOT wetland than in the existing condition.

The model gives the following results; however, they do not reflect the backwater from the outfall as discussed above. It is likely that during peak discharge Cells 2, 3, and 4 will all fill to an elevation slightly above 62.5 and that stormwater will be discharged to the WSDOT wetland. It is possible during the 100 year event for stormwater to fill the WSDOT wetland and for stormwater to exit the north side of the wetland and flow west over dike road. The model gives depth of the peak water storage in Cell 1 as 1.92 feet above the weir notch (64.91). This cell is unaffected by the backwater effects. The depth of the peak water storage in Cell 2 is modeled at 1.87 feet above the lower weir notch and 1.37 feet above the upper weir notch (62.87). This peak storage elevation equates to 2.87 feet of water storage in the channel areas and 3.37 feet of water storage in the pool areas. At a water elevation of 62.5, discharge will begin into the WSDOT wetland. The peak depth of the water storage in Cell 3 is 1.81 to 3 feet. The peak flow to the river is modeled as being reduced from 86.12 cfs (coming into the wetland) to 68.08 cfs (out the Butler outfall). Actual peak discharge flows to the river are likely to be lower due to backwater effects. Some of the peak discharge reduction is additional storage in the wetlands and some is discharge to the WSDOT wetland.

Appended on: Friday, November 20, 2009 2:56:02 PM

Layout Report: COA Wetland with Infiltration

Event	Precip (in)
weekly	0.38
6 month	1.26
2 yr	1.80
10 yr	2.75
25 yr	3.20
100 yr	3.75

Reach Records

Record Id: BD13-OL01

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	OL-01	UpNode	BD-13
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	2594.00 ft	Slope	0.20%
Up Invert	102.43 ft	Dn Invert	107.67 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BE04-DM19

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-19	UpNode	BE-04
Material	unspecified	Size	21 in Diam
Ent Losses	Headwall		
Length	1154.00 ft	Slope	3.18%
Up Invert	120.167 ft	Dn Invert	156.83 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BE06-BE04

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	BE-04	UpNode	BE-06
Material	unspecified	Size	21 in Diam
Ent Losses	Groove End Projecting		
Length	586.50 ft	Slope	0.79%
Up Invert	157.827 ft	Dn Invert	161.46 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BE13-BE06

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	BE-06	UpNode	BE-13
Material	unspecified	Size	21 in Diam
Ent Losses	Groove End w/Headwall		
Length	1838.20 ft	Slope	1.48%
Up Invert	161.441 ft	Dn Invert	188.61 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: BW18-DM10

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-10	UpNode	BW-18
Material	unspecified	Size	18 in Diam
Ent Losses	Headwall		
Length	1297.40 ft	Slope	1.15%
Up Invert	91.788 ft	Dn Invert	106.76 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: CB 50 - CB 6A

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-02 PD	UpNode	CB 50
Material	unspecified	Size	42 in Diam
Ent Losses	Groove End w/Headwall		
Length	214.00 ft	Slope	1.08%
Up Invert	57.14 ft	Dn Invert	54.8288 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: Cell 1 to Cell 2

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift		Cell 1- weir
DnNode	Cell 2	UpNode	Cell 1
Length	1.00 ft	Slope	150.00%
Bottom Width	6.00 ft	Top of Bank	3.00 ft
SS1	3.00v:1h	SS2	3.00v:1h
Up Invert	63.00 ft	Dn Invert	61.50 ft

Record Id: Cell 1 to ground

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift		Cell 1 - infiltration
DnNode	Cell 1 to Infiltration	UpNode	Cell 1- Dummy
Length	10.00 ft	Slope	5.00%
Bottom Width	1.00 ft	Top of Bank	1.00 ft
SS1	3.00v:1h	SS2	3.00v:1h
Up Invert	59.00 ft	Dn Invert	58.50 ft

Record Id: Cell 2 to Cell 3

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift		Cell 2 - weir; Cell 2 -weir - lower
DnNode	Cell 3	UpNode	Cell 2
Length	1.00 ft	Slope	100.00%
Bottom Width	6.00 ft	Top of Bank	3.00 ft
SS1	3.00v:1h	SS2	3.00v:1h
Up Invert	60.00 ft	Dn Invert	61.00 ft

Record Id: Cell 2 to ground

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift		Cell 2 - infiltration
DnNode	Cell 2 to infiltration	UpNode	Cell 2 - Dummy
Length	10.00 ft	Slope	5.00%
Bottom Width	1.00 ft	Top of Bank	1.00 ft
SS1	3.00v:1h	SS2	3.00v:1h
Up Invert	60.00 ft	Dn Invert	59.50 ft

Record Id: Cell 3 to ground

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.03
Routing Method:	Travel Time Shift		Cell 3 - infiltration
DnNode	Cell 3 to infiltration	UpNode	Cell 3 - Dummy
Length	10.00 ft	Slope	5.00%
Bottom Width	1.00 ft	Top of Bank	1.00 ft
SS1	3.00v:1h	SS2	3.00v:1h
Up Invert	60.00 ft	Dn Invert	59.50 ft

Record Id: Cell 4

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.03
Routing Method:	Travel Time Shift		Cell 3 - weir
DnNode	Cell 4 - End	UpNode	Cell 3
Length	227.00 ft	Slope	0.77%
Bottom Width	8.00 ft	Top of Bank	5.9167 ft
SS1	2.00v:1h	SS2	2.00v:1h
Up Invert	59.21 ft	Dn Invert	57.4621 ft

Record Id: Cell 4 - CB 50

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	CB 50	UpNode	Cell 4 - End
Material	unspecified	Size	42 in Diam
Ent Losses	Beveled ring, 33.7 deg bevels		
Length	32.00 ft	Slope	1.00%
Up Invert	57.46 ft	Dn Invert	57.14 ft

Conduit Constraints				
Min Vel	Max Vel	Min Slope	Max Slope	Min Cover
2.00 ft/s	15.00 ft/s	0.50%	2.00%	3.00 ft
Drop across MH		0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.				

Record Id: DM10-DM08

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-08	UpNode	DM-10
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	494.50 ft	Slope	1.24%
Up Invert	91.79 ft	Dn Invert	85.678 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
Drop across MH		0.00 ft	Ex/Infil Rate
Match inverts.			

Record Id: DM-11 to ground

Section Shape:	Ditch		
Uniform Flow Method:	Manning's	Coefficient:	0.012
Routing Method:	Travel Time Shift		DM-11 overflow
DnNode	DM-11 to infiltration	UpNode	DM-11 Dummy
Length	10.00 ft	Slope	5.00%
Bottom Width	10.00 ft	Top of Bank	3.00 ft
SS1	3.00v:1h	SS2	3.00v:1h
Up Invert	101.80 ft	Dn Invert	101.30 ft

Record Id: DM11-DM10

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift		DM-11 outlet
DnNode	DM-10	UpNode	DM-11
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	181.30 ft	Slope	0.71%
Up Invert	93.07 ft	Dn Invert	91.7918 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM14-OL01

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	OL-01	UpNode	DM-14
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	183.10 ft	Slope	0.42%
Up Invert	93.931 ft	Dn Invert	94.70 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM19-DM14

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-14	UpNode	DM-19
Material	unspecified	Size	24 in Diam
Ent Losses	Headwall		
Length	768.40 ft	Slope	2.90%
Up Invert	97.8995 ft	Dn Invert	120.16 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM21-DM19

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-19	UpNode	DM-21
Material	unspecified	Size	15 in Diam
Ent Losses	Headwall		
Length	268.50 ft	Slope	4.17%
Up Invert	120.148 ft	Dn Invert	131.35 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM2-DM1

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.011
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-01 PD	UpNode	DM-02 PD
Material	unspecified	Size	36 in Diam
Ent Losses	Groove End w/Headwall		
Length	262.00 ft	Slope	1.20%
Up Invert	51.686 ft	Dn Invert	54.83 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM5A-Cell1

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.015
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	Cell 1	UpNode	DM-05a
Material	unspecified	Size	48 in Diam
Ent Losses	Headwall		
Length	172.00 ft	Slope	0.75%
Up Invert	64.81 ft	Dn Invert	66.10 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
1.00 ft/s	15.00 ft/s	0.25%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM6-DM5A

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.024
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-05a	UpNode	DM-06
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	75.00 ft	Slope	13.80%
Up Invert	76.60 ft	Dn Invert	66.25 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: DM8-DM6

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.024
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-06	UpNode	DM-08
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	567.70 ft	Slope	1.60%
Up Invert	85.68 ft	Dn Invert	76.5968 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Record Id: OL01-DM11

Section Shape:	Circular		
Uniform Flow Method:	Manning's	Coefficient:	0.016
Routing Method:	Travel Time Shift	Contributing Hyd	
DnNode	DM-11	UpNode	OL-01
Material	unspecified	Size	36 in Diam
Ent Losses	Headwall		
Length	183.10 ft	Slope	0.47%
Up Invert	93.106 ft	Dn Invert	93.93 ft
Conduit Constraints			
Min Vel	Max Vel	Min Slope	Max Slope
2.00 ft/s	15.00 ft/s	0.50%	2.00%
			Min Cover
			3.00 ft
Drop across MH	0.00 ft	Ex/Infil Rate	0.00 in/hr
Match inverts.			

Node Records

Record Id: BD-13

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	107.67 ft	Max El.	114.50 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BE-04

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	156.83 ft	Max El.	167.25 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BE-06

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	161.46 ft	Max El.	170.72 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BE-13

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	188.61 ft	Max El.	193.89 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: BW-18

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	106.76 ft	Max El.	110.77 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: CB 50

Descrip:	type 2 - 72	Increment	0.10 ft
Start El.	57.15 ft	Max El.	66.00 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 2--72
Ent Ke		Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	28.274 sf
MH/CB Type Node			

Record Id: Cell 1

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	59.00 ft	Max El.	65.50 ft
Void Ratio	100.00		
Storage Node	Cell 1 - combo	Discharge Structure	Cell 1 - Storage
Detention Pond Type Node			

Record Id: Cell 1 - Storage

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	59.00 ft	Max El.	64.50 ft
Void Ratio	100.00		
Stage (ft)		Area (sf)	
0.00		2547.00	
0.50		2619.00	
1.00		4169.00	
2.00		5860.00	
3.00		7614.00	
4.00		9727.00	
5.00		11616.00	
5.50		12683.00	
Stage-Storage Type Node			

Record Id: Cell 1- Dummy

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	60.00 ft	Max El.	65.50 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: Cell 1 to Infiltration

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	59.50 ft	Max El.	65.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: Cell 2

Descrip:	wetland cell	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
Void Ratio	100.00		
Storage Node	Cell 2 - combo	Discharge Structure	Cell 2 - Storage
Detention Pond Type Node			

Record Id: Cell 2 - Storage

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
Void Ratio	100.00		
Stage (ft)		Area (sf)	
0.00		12782.00	
0.50		34486.00	
1.00		44088.00	
1.50		93666.00	
2.00		103242.00	
3.00		119563.00	
3.50		140434.00	
Stage-Storage Type Node			

Record Id: Cell 2 - Dummy

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: Cell 2 to infiltration

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	59.50 ft	Max El.	63.50 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: Cell 3

Descrip:	wetland cell	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
Void Ratio	100.00		
Storage Node	Cell 3 - combo	Discharge Structure	Cell 3 - Storage
Detention Pond Type Node			

Record Id: Cell 3 - Storage

Descrip:	cell 3	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.00 ft
Void Ratio	100.00		
Stage (ft)		Area (sf)	
0.00		44976.00	
0.50		52823.00	
1.00		60531.00	
1.50		68415.00	
2.00		78284.00	
3.00		99741.00	
Stage-Storage Type Node			

Record Id: Cell 3 - Dummy

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	60.00 ft	Max El.	62.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: Cell 3 to infiltration

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	59.50 ft	Max El.	62.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: Cell 4 - End

Descrip:	inlet of pipe system	Increment	0.10 ft
Start El.	57.46 ft	Max El.	63.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: DM-01 PD

Descrip:	Butler Outfall	Increment	0.10 ft
Start El.	51.70 ft	Max El.	57.42 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: DM-02 PD

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	54.83 ft	Max El.	67.85 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 2--72
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	28.274 sf
MH/CB Type Node			

Record Id: DM-05a

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	66.10 ft	Max El.	77.00 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 3-72
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	28.2743 sf
MH/CB Type Node			

Record Id: DM-06

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	76.60 ft	Max El.	96.22 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 2-60
Ent Ke		Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	19.634 sf
MH/CB Type Node			

Record Id: DM-08

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	85.68 ft	Max El.	98.82 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 2-60
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	19.634 sf
MH/CB Type Node			

Record Id: DM-10

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	91.79 ft	Max El.	103.21 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 2-60
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	19.634 sf
MH/CB Type Node			

Record Id: DM-11

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
Void Ratio	100.00		
Storage Node	DM-11 Combo	Discharge Structure	DM-11 Storage
Detention Pond Type Node			

Record Id: DM-11 Storage

Descrip:	pipe end	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
Void Ratio	100.00		
Stage (ft)		Area (sf)	
0.00		12.57	
4.93		12.57	
8.23		12.57	
8.43		80.00	
9.73		160.00	
9.93		180.00	
Stage-Storage Type Node			

Record Id: DM-11 Dummy

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	101.80 ft	Max El.	103.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: DM-11 to infiltration

Descrip:	Settling Pool	Increment	0.10 ft
Start El.	101.30 ft	Max El.	104.00 ft
Void Ratio	100.00		
Dummy Type Node			

Record Id: DM-14

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	94.70 ft	Max El.	111.08 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: DM-19

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	120.16 ft	Max El.	124.95 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: DM-21

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	131.35 ft	Max El.	139.67 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge;.ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Record Id: OL-01

Descrip:	Manhole structure	Increment	0.10 ft
Start El.	93.93 ft	Max El.	109.83 ft
Void Ratio	100.00		
Condition	Existing	Structure Type	CB-TYPE 1-48
Ent Ke	CMP: Headwall or Headwall & Wingwall sq edge; ke=0.5	Channelization	No Special Shape
Catch	0.00 ft	Bottom Area	12.5664 sf
MH/CB Type Node			

Discharge Records

Record Id: DM-11 Combo

Combination Discharge Structure			
Descrip:	combined pipe outlet and curb overflow	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
List of Controls		DM-11 outlet DM-11 overflow	

Record Id: DM-11 outlet

Vertical Orifice			
Descrip:	combine infiltration and weir	Increment	0.10 ft
Start El.	93.07 ft	Max El.	103.00 ft
Weir Area	7.069 sf	Weir Coeff	0.61

Record Id: DM-11 overflow

Broad Crested Weir			
Descrip:	combine infiltration and weir	Increment	0.10 ft
Start El.	96.25 ft	Max El.	103.00 ft
Length	10.00 ft	cd	3.216

Record Id: Cell 1 - combo

Combination Discharge Structure			
Descrip:	combine infiltration and weir	Increment	0.10 ft
Start El.	60.00 ft	Max El.	65.50 ft
List of Controls		Cell 1 - infiltration Cell 1- weir	

Record Id: Cell 1 - infiltration

Infiltration			
Descrip:	infiltration out of cell 1	Increment	0.10 ft
Start El.	60.00 ft	Max El.	65.50 ft
Infiltration rate	0.25 in/hr	WP Multiplier	0.50

Record Id: Cell 1- weir

Broad Crested Weir			
Descrip:	Broad Crested Weir	Increment	0.10 ft
Start El.	63.00 ft	Max El.	65.50 ft
Length	10.00 ft	cd	3.216

Record Id: Cell 2 - combo

Combination Discharge Structure			
Descrip:	combine infiltration and weir	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
List of Controls		Cell 2 - infiltration Cell 2 - weir Cell 2 -weir - lower	

Record Id: Cell 2 - infiltration

Infiltration			
Descrip:	infiltration out of cell 2	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
Infiltration rate	0.25 in/hr	WP Multiplier	1.00

Record Id: Cell 2 - weir

Broad Crested Weir			
Descrip:	Broad Crested Weir	Increment	0.10 ft
Start El.	61.50 ft	Max El.	63.50 ft
Length	11.00 ft	cd	3.216

Record Id: Cell 2 -weir - lower

Broad Crested Weir			
Descrip:	Broad Crested Weir	Increment	0.10 ft
Start El.	61.00 ft	Max El.	63.50 ft
Length	2.00 ft	cd	3.216

Record Id: Cell 3 - combo

Combination Discharge Structure			
Descrip:	combine infiltration and weir	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
List of Controls		Cell 3 - infiltration Cell 3 - weir	

Record Id: Cell 3 - infiltration

Infiltration			
Descrip:	infiltration out of cell 3	Increment	0.10 ft
Start El.	60.00 ft	Max El.	63.50 ft
Infiltration rate	0.50 in/hr	WP Multiplier	1.00

Record Id: Cell 3 - weir

Broad Crested Weir			
Descrip:	Broad Crested Weir	Increment	0.10 ft
Start El.	60.25 ft	Max El.	62.00 ft
Length	8.00 ft	cd	3.216

Contributing Drainage Areas

Record Id: BD-13

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	24.00 hrs	Abstraction Coeff	0.20			
Pervious Area	57.00 ac	DCIA	65.30 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	40.0494 min	DC TC	25.0767 min			
Pervious CN Calc						
Description		SubArea		Sub cn		
LAWN - SCS A		57.00 ac		68.00		
Pervious Compositd CN (AMC 2)				68.00		
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.2%	0.41	1.80 in	16.1453 min
Shallow	gutter flow	200.00 ft	2.2%	0.0118		0.8261 min
Sheet	pipe conveyance	2670.00 ft	2.2%	0.012	1.80 in	23.078 min
Pervious TC						40.0494 min
DCI - CN Calc						
Description		SubArea		Sub cn		
Pavement and rooftops		65.30 ac		98.00		
DC Compositd CN (AMC 2)				98.00		
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	Smooth Surfaces.	50.00 ft	2.2%	0.011	1.80 in	0.8932 min
Shallow	Paved	200.00 ft	2.2%	0.01		1.1055 min
Sheet	pipe system	2670.00 ft	2.2%	0.012	1.80 in	23.078 min
Pervious TC						25.0767 min

Record Id: BE-04

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	24.00 hrs	Abstraction Coeff	0.20			
Pervious Area	9.90 ac	DCIA	9.40 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	23.9905 min	DC TC	11.0219 min			
Pervious CN Calc						
Description		SubArea			Sub cn	
LAWN - SCS A		9.90 ac			68.00	
Pervious Compositied CN (AMC 2)					68.00	
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	3.3%	0.41	0.00 in	13.7281 min
Shallow	gutter flow	200.00 ft	3.3%	0.01189		0.6796 min
Sheet	pipe conveyance	1090.00 ft	3.3%	0.012	1.80 in	9.5828 min
Pervious TC						23.9905 min
DCI - CN Calc						
Description		SubArea			Sub cn	
Pavement and rooftops		9.40 ac			98.00	
DC Compositied CN (AMC 2)					98.00	
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	3.3%	0.011	0.00 in	0.7594 min
Shallow	gutter flow	200.00 ft	3.3%	0.01189		0.6796 min
Sheet	pipe system	1090.00 ft	3.3%	0.012	1.80 in	9.5828 min
Pervious TC						11.0219 min

Record Id: BE-06

Design Method	SCS	Rainfall type	TYPE1A.RAC			
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Hyd Intv	15.00 min	Peaking Factor	484.00
Storm Duration	24.00 hrs	Abstraction Coeff	0.20
Pervious Area	17.00 ac	DCIA	16.30 ac
Pervious CN	68.00	DC CN	98.00
Pervious TC	31.3508 min	DC TC	15.1775 min

Pervious CN Calc

Description	SubArea	Sub cn
LAWN - SCS A	17.00 ac	68.00
Pervious Compositied CN (AMC 2)		68.00

Pervious TC Calc

Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	1.9%	0.41	0.00 in	17.1204 min
Shallow	gutter flow	200.00 ft	1.9%	0.01189		0.8957 min
Sheet	pipe conveyance	1250.00 ft	1.9%	0.012	1.80 in	13.3347 min
Pervious TC						31.3508 min

DCI - CN Calc

Description	SubArea	Sub cn
Pavement and rooftops	16.30 ac	98.00
DC Compositied CN (AMC 2)		98.00

DCI - TC Calc

Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	1.9%	0.011	0.00 in	0.9471 min
Shallow	gutter flow	200.00 ft	1.9%	0.01189		0.8957 min
Sheet	pipe system	1250.00 ft	1.9%	0.012	1.80 in	13.3347 min
Pervious TC						15.1775 min

Record Id: BE-13

Design Method	SCS	Rainfall type	TYPE1A.RAC
Hyd Intv	15.00 min	Peaking Factor	484.00
Storm Duration	24.00 hrs	Abstraction Coeff	0.20

Pervious Area	11.80 ac	DCIA	12.40 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	18.5304 min	DC TC	9.0904 min			
Pervious CN Calc						
Description		SubArea	Sub cn			
LAWN - SCS A		11.80 ac	68.00			
Pervious Compositied CN (AMC 2)			68.00			
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	7.3%	0.41	0.00 in	9.9928 min
Shallow	gutter flow	200.00 ft	7.3%	0.01189		0.4569 min
Sheet	pipe conveyance	1310.00 ft	7.3%	0.012	1.80 in	8.0806 min
Pervious TC						18.5304 min
DCI - CN Calc						
Description		SubArea	Sub cn			
Pavement and rooftops		12.40 ac	98.00			
DC Compositied CN (AMC 2)			98.00			
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	7.3%	0.011	0.00 in	0.5528 min
Shallow	gutter flow	200.00 ft	7.3%	0.01189		0.4569 min
Sheet	pipe system	1310.00 ft	7.3%	0.012	1.80 in	8.0806 min
Pervious TC						9.0904 min

Record Id: BW-18

Design Method	SCS	Rainfall type	TYPE1A.RAC
Hyd Intv	15.00 min	Peaking Factor	484.00
Storm Duration	24.00 hrs	Abstraction Coeff	0.20
Pervious Area	2.00 ac	DCIA	3.70 ac
Pervious CN	68.00	DC CN	98.00

Pervious TC	54.1477 min	DC TC	23.9846 min			
Pervious CN Calc						
Description	SubArea	Sub cn				
LAWN - SCS A	2.00 ac	68.00				
Pervious Compositated CN (AMC 2)		68.00				
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	0.4%	0.41	0.00 in	31.9295 min
Shallow	gutter flow	200.00 ft	0.4%	0.01189		2.5927 min
Sheet	pipe conveyance	890.00 ft	0.4%	0.012	1.80 in	18.9516 min
Pervious TC						53.4738 min
DCI - CN Calc						
Description	SubArea	Sub cn				
Pavement and rooftops	3.70 ac	98.00				
DC Compositated CN (AMC 2)		98.00				
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	0.4%	0.011	0.00 in	1.7664 min
Shallow	gutter flow	200.00 ft	0.4%	0.01189		2.5927 min
Sheet	pipe system	890.00 ft	0.4%	0.012	1.80 in	18.9516 min
Pervious TC						23.3107 min

Record Id: Cell 1

Design Method	SCS	Rainfall type	TYPE1A.RAC
Hyd Intv	15.00 min	Peaking Factor	484.00
Storm Duration	24.00 hrs	Abstraction Coeff	0.20
Pervious Area	0.19 ac	DCIA	0.33 ac
Pervious CN	85.00	DC CN	98.00
Pervious TC	16.1453 min	DC TC	5.00 min
Pervious CN Calc			

Description	SubArea	Sub cn
WETLAND SLOPES	0.19 ac	85.00
Pervious Compositied CN (AMC 2)		85.00

Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.2%	0.41	0.00 in	16.1453 min
Pervious TC						16.1453 min

DCI - CN Calc		
Description	SubArea	Sub cn
WETLAND POND	0.33 ac	98.00
DC Compositied CN (AMC 2)		98.00

DCI - TC Calc							
Type	Description	Length	Slope	Coeff	Misc	TT	
Shallow	Nearly bare ground (n=0.025)	20.00 ft	2.2%	0.025		0.175 min	
Pervious TC						0.175 min	

Record Id: Cell 2

Design Method	SCS	Rainfall type	TYPE1A.RAC				
Hyd Intv	15.00 min	Peaking Factor	484.00				
Storm Duration	24.00 hrs	Abstraction Coeff	0.20				
Pervious Area	1.10 ac	DCIA	3.04 ac				
Pervious CN	85.00	DC CN	98.00				
Pervious TC	16.1453 min	DC TC	5.00 min				
Pervious CN Calc							
Description		SubArea	Sub cn				
WETLAND SLOPES		1.10 ac	85.00				
Pervious Compositied CN (AMC 2)			85.00				
Pervious TC Calc							
Type	Description	Length	Slope	Coeff	Misc	TT	
Sheet	off lawn	50.00 ft	2.2%	0.41	0.00 in	16.1453 min	
	Pervious TC					16.1453 min	
DCI - CN Calc							
Description		SubArea	Sub cn				
WETLAND POND		3.04 ac	98.00				
DC Compositied CN (AMC 2)			98.00				
DCI - TC Calc							
Type	Description	Length	Slope	Coeff	Misc	TT	
Shallow	Nearly bare ground (n=0.025)	20.00 ft	2.2%	0.025		0.175 min	
	Pervious TC					0.175 min	

Record Id: Cell 3

Design Method	SCS	Rainfall type	TYPE1A.RAC				
Hyd Intv	15.00 min	Peaking Factor	484.00				
Storm Duration	24.00 hrs	Abstraction Coeff	0.20				
Pervious Area	1.10 ac	DCIA	2.30 ac				
Pervious CN	85.00	DC CN	98.00				
Pervious TC	16.1453 min	DC TC	5.00 min				
Pervious CN Calc							
Description		SubArea	Sub cn				
WETLAND SLOPES		1.10 ac	85.00				
Pervious Compositied CN (AMC 2)			85.00				
Pervious TC Calc							
Type	Description	Length	Slope	Coeff	Misc	TT	
Sheet	off slopes	50.00 ft	2.2%	0.41	1.80 in	16.1453 min	
	Pervious TC					16.1453 min	
DCI - CN Calc							
Description		SubArea	Sub cn				
WETLAND POND		2.30 ac	98.00				
DC Compositied CN (AMC 2)			98.00				
DCI - TC Calc							
Type	Description	Length	Slope	Coeff	Misc	TT	
Shallow	Nearly bare ground (n=0.025)	20.00 ft	2.2%	0.025		0.175 min	
	Pervious TC					0.175 min	

Record Id: Cell 4

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	24.00 hrs	Abstraction Coeff	0.20			
Pervious Area	0.18 ac	DCIA	0.23 ac			
Pervious CN	85.00	DC CN	98.00			
Pervious TC	16.1453 min	DC TC	5.00 min			
Pervious CN Calc						
Description		SubArea		Sub cn		
WETLAND SLOPES		0.18 ac		85.00		
Pervious Compositied CN (AMC 2)				85.00		
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.2%	0.41	0.00 in	16.1453 min
	Pervious TC					16.1453 min
DCI - CN Calc						
Description		SubArea		Sub cn		
WETLAND POND		0.23 ac		98.00		
DC Compositied CN (AMC 2)				98.00		
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Shallow	Short grass, pasture and lawns (n=0.030)	20.00 ft	2.2%	0.03		0.21 min
	Pervious TC					0.21 min

Record Id: DM-08

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	24.00 hrs	Abstraction Coeff	0.20			
Pervious Area	7.00 ac	DCIA	12.90 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	24.7356 min	DC TC	11.0791 min			
Pervious CN Calc						
Description		SubArea		Sub cn		
LAWN - SCS A		7.00 ac		68.00		
Pervious Compositied CN (AMC 2)				68.00		
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.9%	0.41	0.00 in	14.4563 min
Shallow	gutter flow	200.00 ft	2.9%	0.01189		0.725 min
Sheet	pipe conveyance	1010.00 ft	2.9%	0.0121	0.00 in	2.3535 min
Pervious TC						17.5348 min
DCI - CN Calc						
Description		SubArea		Sub cn		
Pavement and rooftops		12.90 ac		98.00		
DC Compositied CN (AMC 2)				98.00		
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	2.9%	0.011	0.00 in	0.7997 min
Shallow	gutter flow	200.00 ft	2.9%	0.01189		0.725 min
Sheet	pipe system	1010.00 ft	2.9%	0.0121	0.00 in	2.3535 min
Pervious TC						3.8782 min

Record Id: DM-21a

Design Method	SCS	Rainfall type	TYPE1A.RAC			
Hyd Intv	15.00 min	Peaking Factor	484.00			
Storm Duration	24.00 hrs	Abstraction Coeff	0.20			
Pervious Area	5.70 ac	DCIA	10.40 ac			
Pervious CN	68.00	DC CN	98.00			
Pervious TC	32.6143 min	DC TC	17.0757 min			
Pervious CN Calc						
Description		SubArea		Sub cn		
LAWN - SCS A		5.70 ac		68.00		
Pervious Compositd CN (AMC 2)				68.00		
Pervious TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.1%	0.41	0.00 in	16.4486 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe conveyance	1550.00 ft	2.1%	0.0121	0.00 in	4.2445 min
Pervious TC						21.545 min
DCI - CN Calc						
Description		SubArea		Sub cn		
Pavement and rooftops		10.40 ac		98.00		
DC Compositd CN (AMC 2)				98.00		
DCI - TC Calc						
Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	2.1%	0.011	0.00 in	0.9099 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe system	1550.00 ft	2.1%	0.0121	0.00 in	4.2445 min
Pervious TC						6.0063 min

Record Id: DM-21b

Design Method	SCS	Rainfall type	TYPE1A.RAC			
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Hyd Intv	15.00 min	Peaking Factor	484.00
Storm Duration	24.00 hrs	Abstraction Coeff	0.20
Pervious Area	3.10 ac	DCIA	5.80 ac
Pervious CN	68.00	DC CN	98.00
Pervious TC	24.658 min	DC TC	9.1194 min

Pervious CN Calc

Description	SubArea	Sub cn
LAWN - SCS A	3.10 ac	68.00
Pervious Composited CN (AMC 2)		68.00

Pervious TC Calc

Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	off lawn	50.00 ft	2.1%	0.41	0.00 in	16.4486 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe conveyance	620.00 ft	2.1%	0.0121	0.00 in	1.6978 min
Pervious TC						18.9983 min

DCI - CN Calc

Description	SubArea	Sub cn
Pavement and rooftops	5.80 ac	98.00
DC Composited CN (AMC 2)		98.00

DCI - TC Calc

Type	Description	Length	Slope	Coeff	Misc	TT
Sheet	pavement flow	50.00 ft	2.1%	0.011	0.00 in	0.9099 min
Shallow	gutter flow	200.00 ft	2.1%	0.01189		0.8519 min
Sheet	pipe system	620.00 ft	2.1%	0.0121	0.00 in	1.6978 min
Pervious TC						3.4597 min